Draft

Defense Intelligence Agency (DIA) Headquarters (HQ) Annex Environmental Assessment

Fort Belvoir, Virginia June 2021







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Proposed DIA HQ Annex

Fort Belvoir, Virginia

ENVIRONMENTAL ASSESSMENT

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1.0 INTRODUCTION

1.1 PROJECT BACKGROUND

Pursuant to the National Environmental Policy Act (NEPA) of 1969 (Title 42, U.S. Code [USC], 4321-4370f), as amended, regulations of the Council on Environmental Quality (CEQ) (40 Code of Federal Regulations [CFR] 1500-1508), and 32 CFR Part 651 (Army Analysis of Environmental Actions), Fort Belvoir has prepared an Environmental Assessment (EA) to evaluate potential environmental effects associated with construction of the Defense Intelligence Agency Headquarters (DIA HQ) Annex Building at Fort Belvoir, Virginia.

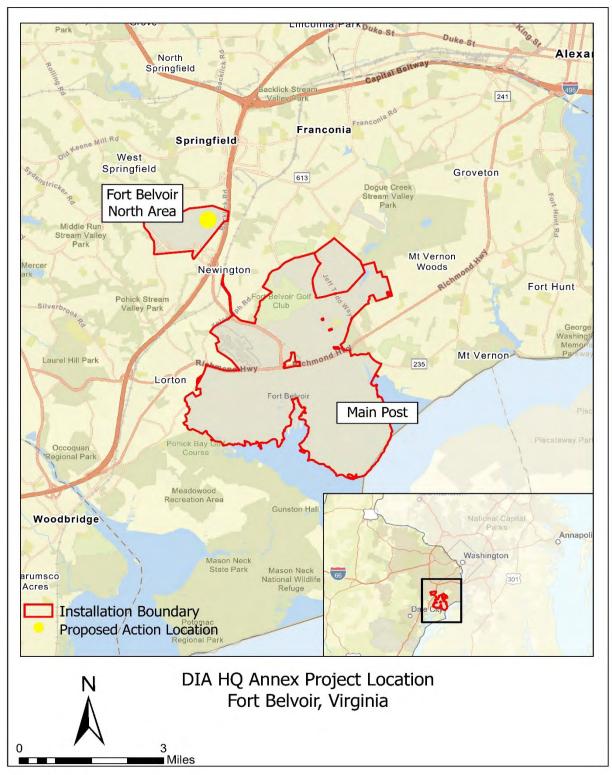
Fort Belvoir is located approximately 18 miles southwest of Washington, DC, and 17 miles south of the Pentagon, on the Potomac River in Fairfax County, Virginia (Figure 1-1). As a strategic sustaining base for America's Army in the National Capital Region (NCR), Fort Belvoir provides logistical, intelligence, and administrative support to a diverse group of more than 140 Army and Department of Defense (DoD) organizations. Fort Belvoir contributes to the nation's defense primarily by providing a secure operating environment for regional and worldwide DoD missions and functions (U.S. Army, 2015).

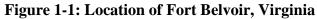
The Defense Intelligence Agency (DIA) was established in the 1960s at Arlington Hall Station and over the past four decades has provided vital intelligence in support of key moments in major conflicts. Its first offices were established on Bolling Air Force Base in 1984 (<u>https://www.dia.mil/</u>). The Defense Intelligence Analysis Center (DIAC) became operational in 1984 and allowed the consolidation and centralization of personnel and missions formerly scattered in a number of locations across the NCR. The DIAC was subsequently expanded in 2005 and renamed as DIA HQ in 2012. In 2010, a new Joint Use Intelligence Analysis Facility was opened in Rivanna Station, and, in 2011, the Russell-Knox Building, housing elements of five military investigative agencies including DIA's Counterintelligence and Human Intelligence Center, opened at Marine Corps Base Quantico, Virginia.

1.2 PURPOSE AND NEED

The purpose of this project is to build and operate an approximately 77,000 net square foot (NSF) / 116,080 gross square foot (GSF) administrative building with an associated parking structure at Fort Belvoir to consolidate administrative facilities for approximately 650 personnel from DIA HQ to address safety, security, and operational concerns specific to the administrative functions of the agency.

The need for the facility is to alleviate the current space constraints of existing leased facilities that pose sustained and increased safety and security concerns. The approximately 650 personnel proposed to be consolidated in an administrative facility on Fort Belvoir represent the authorized civilian and military strength and require quality work environment improvements to mitigate the lack of safety, security, and efficiency. The action would also provide for compliance with





Office of Management and Budget (OMB) guidance identifying "good stewardship of taxpayer resources" and increasing joint site usage efficiencies.

1.3 SCOPE OF THE ENVIRONMENTAL ASSESSMENT

In accordance with CEQ NEPA implementation regulations and 32 CFR Part 651, either an Environmental Impact Statement (EIS) or an EA must be prepared for any federal action, unless the action is determined to be exempt by law, an emergency, or categorically excluded. The EA results in either a Finding of No Significant Impact (FNSI) or a Notice of Intent (NOI) to prepare an EIS.

This EA informs decision makers and the public of the likely environmental consequences of the Proposed Action and No Action Alternative. This EA identifies, documents, and evaluates environmental effects of the construction and operation of a HQs Annex facility at Fort Belvoir, Virginia. Environmental effects would include those related to construction and operation of the proposed action as well as impacts of increased personnel and traffic to Fort Belvoir. The Proposed Action and alternatives, including the No Action Alternative and other alternatives considered, are described in Section 2.0.

Section 3.0 outlines the existing conditions of the Affected Environment and the baseline, No Action Alternative, for which other alternatives are measured against to analyze the effects of the construction of the DIA HQ Annex. The following resources are evaluated at Belvoir: geological and soil resources, water resources, biological resources, air quality, cultural resources, hazardous materials and waste, munitions, utilities, airspace, socioeconomics, noise, traffic, and transportation.

1.4 PUBLIC INVOLVEMENT

Fort Belvoir solicited comments from the U.S. Fish and Wildlife Service (USFWS) and the State Historic Preservation Office (SHPO). Additionally, a Public Notice was sent to agencies and organizations known to have an interest in the site, on {INSERT DATE}, soliciting public input on the proposed action.

Public participation opportunities with respect to this EA and decision making on the proposed action are guided by 32 CFR Part 651. Upon completion, the EA will be made available to the public for 30 days, along with a draft FNSI. At the end of the 30-day public review period, the Army will consider any comments submitted by individuals, agencies, or organizations on the proposed action, the EA, or draft FNSI. As appropriate, the Army may then execute the FNSI and proceed with implementation of the proposed action. If it is determined prior to issuance of a final FNSI that implementation of the proposed action would result in significant impacts, the Army will publish in the *Federal Register* an NOI to prepare an EIS, commit to mitigation actions sufficient to reduce impacts below significance levels, or not take the action.

1.5 ENVIRONMENTAL LAWS AND REGULATIONS

Army decisions that affect environmental resources and conditions occur within the framework of numerous laws, regulations, and Executive Orders (EO). Some of these authorities prescribe standards for compliance while others require specific planning and management actions to protect environmental values potentially affected by Army actions. Key provisions of appropriate statutes and EOs are described in more detail throughout the text of this EA and in Table 1-1.

ACTS	Compliance
Archaeological Resources Protection Act (ARPA) of 1979	FULL
Army Regulation 200-1, Environmental Protection and Enhancement	FULL
Clean Air Act, as amended (42 United States Code [U.S.C.] ch. 85, subch. I	FULL
Clean Water Act, as amended (33 U.S.C. ch. 23 §1151)	FULL
Comprehensive Environmental Response, Compensation, and Liability Act of 1980, as amended by the Superfund Amendments and Reauthorization Act of 1986 (42 U.S.C. §9601 et seq.)	FULL
Endangered Species Act of 1973, as amended (16 U.S.C. ch. 35 §1531 et seq.)	FULL
Energy Independence and Security Act of 2007, Section 438	FULL
Farmland Protection Policy Act (7 U.S.C 4201)	FULL
Fish and Wildlife Coordination Act, as amended (16 U.S.C. 661-667e)	FULL
Migratory Bird Treaty Act (16 U.S.C §§703-712, et seq.)	FULL
National Environmental Policy Act of 1969 (42 U.S.C. §4321 et seq.)	FULL
National Historic Preservation Act of 1966, as amended (16 U.S.C. ch. 1A, subch.II §470 et seq.)	FULL
Noise Control Act of 1972, as amended (42 U.S.C. §§4901-4918, et seq.)	FULL
North American Wetlands Conservation Act (16 U.S.C. 4401-4412)	FULL
Resource Conservation and Recovery Act (42 U.S.C. ch. 82 §6901 et seq.)	FULL
Safe Drinking Water Act, as amended (42 U.S.C. §300f)	FULL
Sikes Act, as amended (16 U.S.C. 670a-670o)	FULL
Solid Waste Disposal Act of 1965, as amended (42 U.S.C 6901 et seq.)	FULL
Toxic Substances Control Act of 1976 (15 U.S.C. ch.53, subch. I §§2601-2629)	FULL
Watershed Protection and Flood Prevention Act of 1954 (16 U.S.C. §1101, et	FULL

Table 1-1: Compliance with Federal Environmental Statutes and Executive Orders

Wild and Scenic Rivers Act (16 U.S.C. 1271, et seq.)	
Executive Orders (EO)	
Protecting Public Health and the Environment and Restoring Science to Tackle the Climate Crisis (EO 13990)	FULL
Floodplain Management (EO 11988)	FULL
Protection of Wetlands (EO 11990)	FULL
Environmental Justice in Minority Populations and Low-Income Populations (EO 12898)	FULL
Federal Compliance with Pollution Control Standards (EO 12088)	FULL
Protection of Children from Environmental Health Risks and Safety Risks (EO 13045)	FULL
Invasive Species (EO 13112)	FULL
Consultation and Coordination with Indian Tribal Governments (EO 13175)	FULL
Efficient Federal Operations (EO 13834)	
Chesapeake Bay Protection and Restoration (EO 13508)	FULL

2.0 DESCRIPTION OF PROPOSED ACTION AND ALTERNATIVES

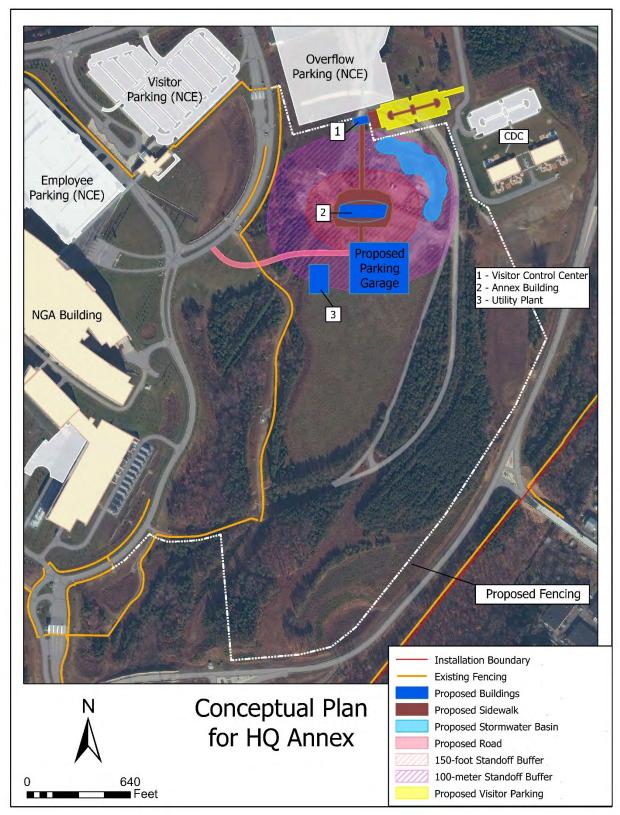
The Proposed Action involves the construction of the HQ Annex building within Fort Belvoir's North Area (FBNA), in the vicinity of the National Geospatial-Intelligence Agency (NGA) complex (see Figure 2-1). The proposed HQ Annex building would be approximately 116,080 SF and would include a multi-story administrative building with offices, cubicles/workstations, publications rooms, conference rooms, break rooms, server rooms, a multi-purpose auditorium, a café/cafeteria, a gym/fitness center, a utility plant, visitor control center, visitor parking, a secured employee parking structure, and a perimeter security fence; all to support stationing of approximately 650 personnel. The Area Development Plan (ADP) for FBNA currently being drafted by Fort Belvoir envisions the area immediately east of the NGA Headquarters as an Intelligence Community Campus. The HQ Annex will be the first programmed and funded construction under this ADP. The proposed perimeter fence alignment is intended to accommodate potential long-term build-out of the FBNA without resulting in the need to realign the fence as additional structures are built.

Numerous authorities impose design and stand-off requirements for the proposed facility, to include the Joint Mission Assurance Assessment Report, the National Counterintelligence and Security Center's Technical Specifications for Construction and Management of Sensitive Compartmented Information Facilities, Version 1.4 (2017), and information security (Infosec) requirements. Design and stand-off requirements include a 100-meter Telecommunications Electronics Materials Protected from Emanating Spurious Transmissions (TEMPEST) stand-off.

Screening criteria for the Proposed Action require that the activity be:

- economically viable in terms of project cost and resulting community impact;
- compatible with adjacent land uses and avoid potential encroachment;
- cognizant of the availability of buildable space and access to utilities, support services, and transportation infrastructure;
- compatible with the Fort Belvoir ADP;
- result in minimal to low environmental impacts;
- pose a minimal security risk to operations; and,
- consider human health and safety impacts.

FBNA is classified as a Military Munitions Response Program site (See Section 3.4.2). Consequently, land use controls are in effect that require munitions clearances for all military construction projects, restrict the use of groundwater, and require vapor barriers on new construction due to groundwater contamination.





2.1 OTHER ALTERNATIVES CONSIDERED BUT ELIMINATED

A smaller, 4.1-acre site was identified at the corner of Doerr Road and 3rd Street, next to the hospital in Fort Belvoir's 1400 East Area. This alternative was screened from further consideration because the area is too small to accommodate the setbacks necessary to comply with anti-terrorism/force protection (AT/FP) and agency security standards, particularly the placement of a fully, secure building and parking structure in such close proximity to the post hospital, where the need for less-restricted access would conflict with the agency's security mandates.

Another alternative involved the construction of the HQs Annex building within Fort Belvoir's 1400 Area East, near the southwest corner of 1st Street and Doerr Road. This approximately 16.9acre area has been reviewed under Belvoir's ADP and designated for future development/siting of the DIA. However, as shown in Figure 2-2 below, the required TEMPEST buffers expand beyond the physical footprint of the site identified in the ADP. In order for the required buffers to be implemented, public access to 1st Street would be eliminated, resulting in impacts to a heavily used public and ambulance access route to the nearby hospital. This second-order impact represents a large roadblock to the project. This alternative does not meet the screening criteria for the security that is required to accommodate the sensitive nature of the DIA's mission, or for compatibility with the overall installation master plan.

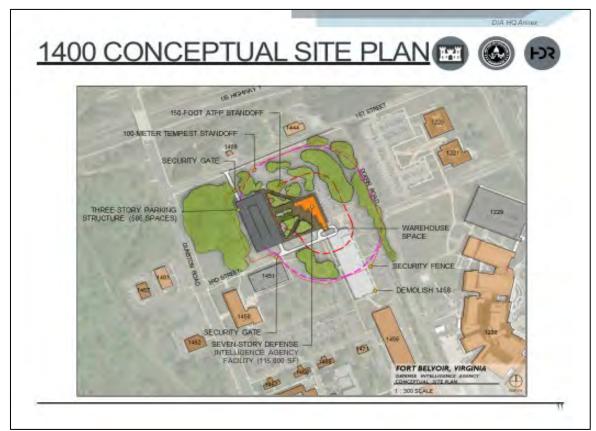


Figure 2-2: DIA HQ Annex Alternative Project Location

2.2 NO ACTION ALTERNATIVE

Under the No-Action alternative DIA would not construct the headquarters facility on Fort Belvoir, resulting in the continued use of multiple leased spaces spread throughout the NCR, which is not secure or efficient and does not meet safety standards. Additionally, the existing facilities are not compliant with current DoD antiterrorism and force protection requirements, nor with OMB guidance to reduce dependency on leases.

The Proposed Action and the No Action Alternative will be carried forward for analysis in this EA.

3.0 AFFECTED ENVIRONMENT AND ENVIRONMENTAL CONSEQUENCES

3.1 GEOLOGY, TOPOGRAPY AND SOILS

3.1.1 Geology

Fort Belvoir North Area is located within the Piedmont geologic province, characterized by gently rolling topography with thick soils underlain by deeply weathered bedrock (<u>http://geology.blogs.wm.edu/piedmont/</u>). In Virginia, the Piedmont province is bounded by the Blue Ridge Mountains to the west and the Fall Line, roughly demarcated by I-95, to the east. The underlying bedrock of the Piedmont is as much as 1,070 million years old and is comprised of rocks of sedimentary and metamorphic origins.

A finger of Piedmont Upland province bedrock extends from north to south along Accotink Creek, forming the bed and adjacent slopes of the creek that roughly bisects FBNA. Most of the more gently sloping areas to the east and west of the creek consist of unconsolidated sediment deposits typical of the Coastal Plain province found east of the Fall Line (U.S. Army, 2007).

3.1.2 Topography

The topography of FBNA is gently rolling, with steep slopes ranging from 20 to 30 percent grade forming a narrow valley along Accotink Creek. Within the proposed project area east of Accotink Creek, the land has been previously graded and is mostly flat with a gradual decrease in elevation from 240 to 235 feet above mean sea level (an approximately 1.5 percent slope) from north to south (Figure 3-1) (HDR, 2020).

3.1.3 Soils

Soils within the project area are comprised predominantly of Kingstowne sandy clay loam, 0 to 45 percent slopes, according to the United States Department of Agriculture (USDA), Natural Resources Conservation Service (NRCS) mapping (NRCS, 2020). Urban land is the next highest class, followed by Sassafras sandy loam, 7 to 15 percent slopes, Beltsville silt loam, 2 to 7 percent slopes, and Sassafras-Marumsco complex, 7 to 15 percent slopes (see Table 3-1 and Figure 3-2).

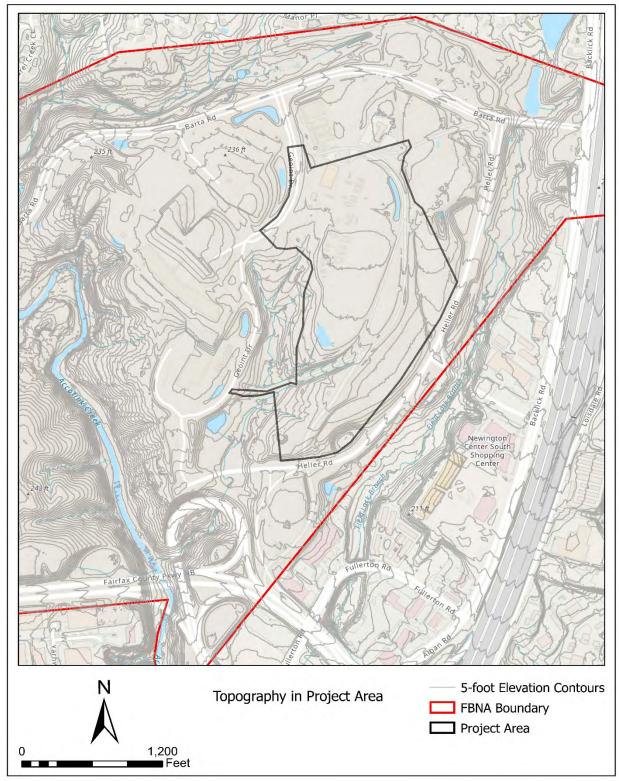


Figure 3-1: Topography in Project Area

Map Unit Symbol	Soil	Drainage Class	Hydric
95	Urban land	N/A	N/A
90C	Sassafras sandy loam, 7 to 15 percent slopes	Well drained	No
66	Kingstowne sandy clay loam, 0 to 45 percent slopes	Well drained	No
7B	Beltsville silt loam, 2 to 7 percent slopes	Moderately well drained	No
91C	Sassafras-Marumsco complex, 7 to 15 percent slopes	Well drained	No
Notes: Hydric criteria refer to the potential of a soil to support vegetation and/or hydric conditions indicative of wetlands. Source: NRCS, 2020			

Table 3-1: Soils in the Study Area

An area of soil and groundwater contamination is found within the study area as a result of three former underground fuel storage tank (UST) facilities associated with previous land use. Removal of the USTs and subsequent soil remediation to clean up contamination was conducted in 1996-1997. Five hundred and eight (508) tons of petroleum impacted soil were removed from two contaminated sites within the study area in 1996. Subsequent testing indicates little or no residual soil contamination within these sites (AECOM, 2021). Site contamination is discussed further in Section 3.4.

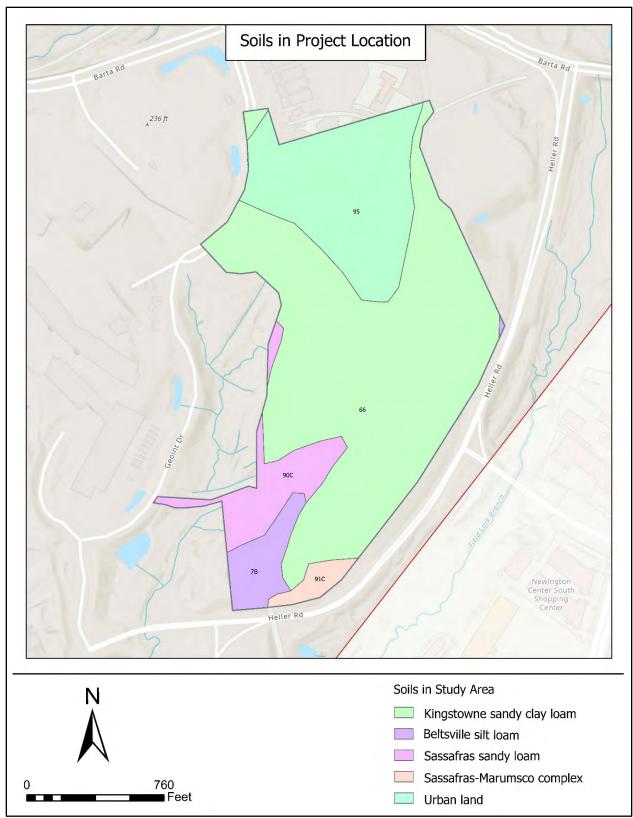


Figure 3-2: Soils in Study Area

3.1.4 Environmental Consequences

3.1.4.1 Threshold of Significance

Impacts on geology, topography, and soils are evaluated separately in the following sections. The impacts on geology are analyzed based on potential changes, caused by the Proposed Action, to bedrock, unique sensitive landforms, or rock foundations. The impacts on topography are analyzed on potential changes to surface features, especially steep slopes. Impacts to soil are analyzed based on potential changes to soil type, erosion, and sedimentation due to the implementation of the Proposed Action.

3.1.4.2 Impacts of the Proposed Action

Geology

The Proposed Action would have minimal effects, beneficial or adverse, on underlying geology. While some excavation into underlying bedrock would be required to establish the foundation for this multi-story building, these actions would alter only a small area within the larger, regional landscape and would not alter the underlying geological characteristics.

Topography

The Proposed Action would have minimal effects, beneficial or adverse, to the topography of this previously disturbed site, nor result in the alteration or destruction of any unique or noteworthy topographic features within FBNA. Excavating and grading would be employed to prepare the site for construction, and the elevations would be permanently altered to support the buildings, the parking areas and stormwater management. However, these effects would be beneficial in reducing accelerated rates of run-off from adversely affecting downstream receiving waters.

Soils

Minor adverse impacts to soil would occur under the Proposed Action. The Proposed Action would require clearing of vegetation, and grading and excavation of soils within the approximately 50-acre project area. These actions expose soils and increase the potential for erosion. Because of the well-established connection between erosion of exposed soils and introduction of increased sedimentation into downstream waters, regulations have been enacted by federal, state and local governments to require project proponents to develop and implement plans to control site conditions and prevent erosion. These regulations and the types of site control mechanisms are described in more detail in Section 3.2.6.

3.1.4.3 Impacts of the No Action Alternative

Geology

The No Action alternative would have no adverse effect on the underlying geology of the area, as no grading or other earthwork would occur.

Topography

The No Action Alternative would have no adverse effects on topography on FBNA or the project site because existing conditions would continue as no grading or other earthwork would occur.

Soils

Under the No Action alternative long-term, minor adverse impacts to soil quality would occur due to erosion. Soils within the gravel parking lot, along with surrounding areas of sparse vegetation, would continue to be exposed and subject to erosion. Vegetated areas provide root structure that stabilizes soils and continue to support infiltration of rainwater, among other important services.

3.2 WATER RESOURCES

3.2.1 Surface Waters

FBNA is located entirely within the highly urbanized 52-square-mile Accotink Creek watershed, which ultimately discharges to Accotink Bay and the Potomac River. Accotink Creek roughly bisects the 804-acre FBNA into eastern and western sections. The Proposed Action study area is located within the eastern half of FBNA.

As described in Section 3.1.2, the study area is relatively flat, sloping gradually downhill from north to south. The northern portion of the site is currently used as a gravel overflow parking lot, known as the North Subcontractor Parking Lot, a relic of its use as an equipment and materials staging area during the construction of the NGA facility in 2007-2008. On the northwestern side of the project area the land slopes down from this gravel parking lot through an area vegetated predominantly with Virginia pines (*Pinus virginiana*), until it meets the NGA perimeter security fence and patrol path. Several unnamed erosional features carry stormwater down gradient from the gravel parking lot in a northeast to southwest direction. A site visit by personnel from the U.S. Army Corps of Engineers (USACE), Baltimore District on September 17, 2020 indicated no Ordinary High Water Mark (OHWM) present within these features. Further, a more recent stormwater run-off pathway has established itself along an abandoned dirt road. A series of rock weirs have been installed along this pathway, but erosion is still evident (Photos 3-1 and 3-2).



Photos 3-1 and 3-2: Erosional feature on northwestern side of the study area.

Downstream and outside of the study area, the Fort Belvoir Integrated Natural Resources Management Plan (INRMP) (Fort Belvoir, 2017) has identified perennial streams with associated riparian wetlands to the west and southwest of the study area, and which connect into Accotink Creek north of its intersection with Fairfax County Parkway (Figure 3-3).

Within the eastern portion of the study area, an approximately 0.25-acre stormwater pond (Photo 3-3) captures runoff from both sheet flow over the gravel parking as well as from a network of stormwater pipes serving the same area. This stormwater pond was constructed between 2007 and 2008, based on a review of historic aerial photography available on Google Earth, and was associated with the site grading that occurred in support of the NGA construction lay-down area. It is not currently maintained as a stormwater management facility. The pond is bound to the east by the remnant of the former Engineering Proving Ground (EPG) concrete test track known as Heller Loop. No discharge pipe connecting this pond to downstream waters was observed during the September 17, 2020 inspection, although such a feature may have become silted in and overgrown with vegetation. Such a connection is not apparent on available Geographic Information System (GIS) mapping of the area; however, a potential down gradient discharge would be the swale underneath a stand of Virginia pines that parallels Heller Loop to the east until the land slopes downhill to the east, south of the North Belvoir Child Development Center (CDC). Stormwater in this area is detained and treated by a stormwater management facility located directly south of the CDC.



Photo 3-3: Stormwater pond on eastern edge of the study area.

The Accotink Creek watershed is 87% developed with commercial, industrial, transportation or residential land with 28% of the non-tidal portion of the watershed covered by impervious surface (Virginia Department of Environmental Quality [VADEQ], 2017). The quality of surface waters in such highly urbanized areas typically becomes degraded through increased amounts of sediments, chemicals, nutrients, and bacteria resulting from human activities. Pursuant to Section 303(d) of the federal Clean Water Act (CWA), which requires states to develop a list of impaired waterbodies, the VADEQ has identified Accotink Creek as an impaired water based on biological monitoring of benthic macroinvertebrate communities. Section 303(d) of the CWA further requires states to take steps to halt or counteract degradation through development of Total Maximum Daily Load (TMDL) standards for specific pollutants. TMDLs target the load reduction needed to reduce the pollutants of concern and represent the total pollutant loading that a waterbody can receive without exceeding water quality standards. For Accotink Creek, TMDLs are under development for sediment and chlorides.

3.2.2 Resource Protection Areas

The unnamed stream and associated riparian wetlands to the west of the study area are denoted as a Riparian Buffer Area (RPA) on Fort Belvoir's INRMP mapping (Figure 3-3). These features ultimately connect to Accotink Creek, which discharges to Accotink Bay, a tributary to the Potomac River and the Chesapeake Bay. Recognizing the Chesapeake Bay's critical role in the economy and health of the region and the importance of improving the health of the Bay, the State of Virginia's General Assembly adopted the Chesapeake Bay Preservation Act in 1988. The Act requires local governments within Tidewater Virginia to adopt implementing regulations that promote water quality protection measures. One of the key provisions of this Act requires the protection of vegetative buffers, known as RPAs, no less than 100 feet wide located adjacent to and landward of all tidal shores, tidal wetlands, water bodies with perennial flow, and non-tidal wetlands connected by surface flow and contiguous to tidal wetlands along water bodies with perennial flow. In Fairfax County, where Fort Belvoir is located, the Chesapeake Bay Preservation Ordinance (CBPO) is the applicable local regulation. Fort Belvoir recognizes the RPA designation but, being a federal entity, is not subject to the provisions of the Fairfax County ordinance. While Fort Belvoir does not use the RPA maps produced by Fairfax County, the Army does delineate RPAs on the installation, reflecting a spirit of compliance with the state and local requirements. Further, as part of the INRMP, Fort Belvoir designates a 35-foot RPA buffer for intermittent streams. The study area in relationship to installation-mapped RPAs is shown in Figure 3-3.

Establishing an RPA serves to limit adverse effects of development adjacent to streams and tidal wetlands by preserving vegetated buffers around sensitive aquatic resources. Vegetated buffers provide additional surface area for attenuation of surface water run-off velocity, thereby reducing erosion; filtration of excess nutrients and other pollutants carried by stormwater; and, additional habitat corridors. Development in these areas should be avoided and/or minimized. When impacts occur, an additional review is conducted to determine the extent of impact, as well as mitigation for the RPA infringement. Mitigation for RPA impacts typically includes the replanting of trees and/or shrubs at a predetermined ratio or the enhancement of a degraded RPA elsewhere on Fort Belvoir. RPAs are typically addressed during the wetland permitting process or the Coastal Zone Management Act (CZMA) consistency determination process.

It should be noted that EO 13508, *Chesapeake Bay Protection and Restoration*, must be addressed in terms of the Army's obligation to consider the protection and restoration of the Chesapeake Bay Watershed in terms of meeting the goals, outcomes and objectives set out in the Strategy for Protecting and Restoring the Chesapeake Bay Watershed. This document not only sets goals/outcomes/objectives of the federal government, but encourages coordination with state, local, and non-governmental partners to protect and restore the health of the Chesapeake Bay Watershed.

3.2.3 Floodplains

One-hundred-year floodplains on Fort Belvoir are protected under Executive Order (EO) 11988, *Floodplain Management* (May 24, 1977), which directs federal agencies to avoid, to the extent possible, the long and short term adverse impacts associated with the occupancy and modification of floodplains and to avoid direct and indirect support of floodplain development wherever there is a practicable alternative. The EO was issued in furtherance of NEPA, the National Flood Insurance Act of 1968, and the Flood Disaster Protection Act of 1973. Floodplains are defined in EO 11988 as the "lowland and relatively flat areas adjoining inland and coastal waters including flood prone areas of offshore islands, including at a minimum, that area subject to a one percent or greater chance of flooding in any given year."

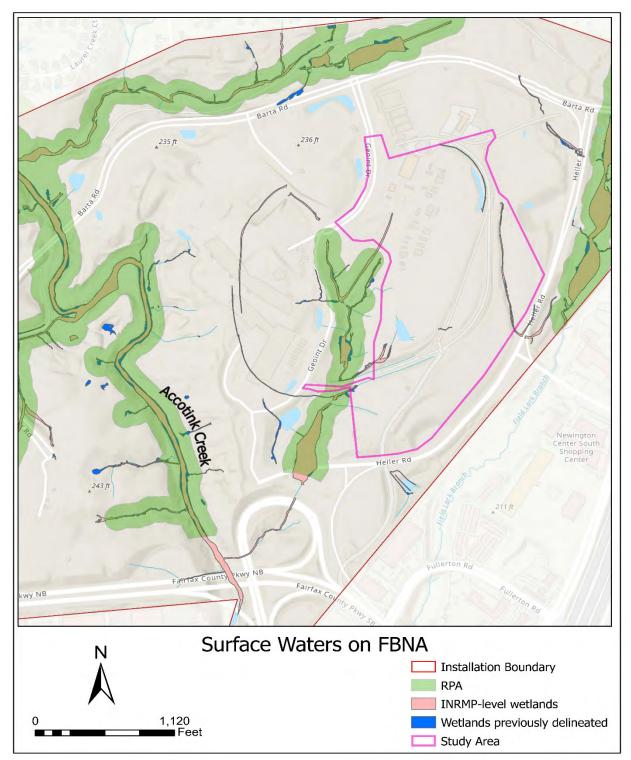


Figure 3-3: Surface Waters on FBNA

As a federal agency subject to this EO, Fort Belvoir is required to evaluate potential effects of any action occurring in a floodplain. The Proposed Action is located outside of the 100-year floodplain associated with Accotink Creek (Figure 3-4).

3.2.4 Wetlands

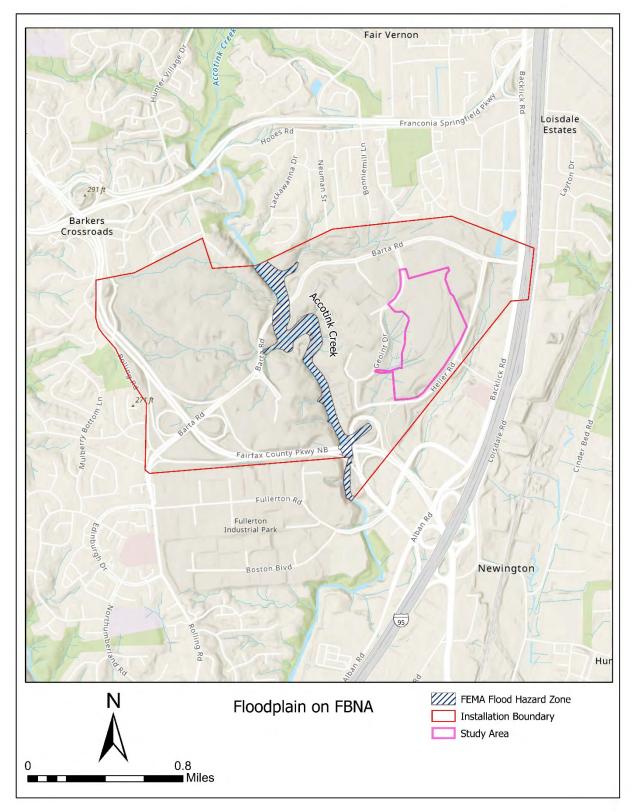
USACE defines wetlands as "those areas that are inundated or saturated with ground or surface water at a frequency and duration sufficient to support, and that under normal circumstances do support, a prevalence of vegetation typically adapted to life in saturated soil conditions. Wetlands generally include swamps, marshes, bogs, and similar areas" (33 CFR Part 328). Important wetland functions include water quality improvement, groundwater recharge and discharge, storm water attenuation and storage, sediment detention, fish and wildlife habitat, and erosion protection.

EO11990, *Protection of Wetlands* (May 24, 1977), requires federal agencies to take action to minimize the destruction, loss, or degradation of wetlands, and to preserve and enhance the natural and beneficial values of wetlands. Construction in jurisdictional wetlands and waters of the US is regulated by the USACE pursuant to Section 404 of the CWA as implemented in regulations contained in 33 CFR, Parts 320–330. Impacts to state waters, including wetlands, are regulated by the Virginia Water Protection Permit Program (9 Virginia Administrative Code [VAC] 25-210-10 et seq.), which serves as Virginia's 401 Water Quality Certification Program for federal Section 404 Permits.

The predominant wetland type on Fort Belvoir is palustrine forested (PFO) wetland, which tends to occur in association with the riparian areas of Accotink, Dogue, and Pohick Creeks. Wetlands generally occur along the perennial and intermittent streams that are drainages of these creeks (Fort Belvoir, 2017).

The stormwater pond on the eastern side of the project area, denoted on installation natural resources mapping as a wetland, was examined during the September 17, 2020 site inspection by USACE biologists during a natural resources survey for the project (USACE, 2020). Mapping of potential resources under the INRMP makes general assumptions based on a review of aerial photography, but site-specific verification using the 1987 Corps of Engineers Wetland Delineation Manual must be conducted to confirm and refine this high-level mapping. While the littoral zone of the pond supports an abundance of hydrophytic vegetation such as black willow (*Salix nigra*), barnyard grass (*Echinochloa muricata*), soft rush (*Juncus effusus*), and swamp smartweed (*Polygonum hydropiperoides*), the soils lacked hydric characteristics and were of a homogeneous distribution indicative of a man-made feature created as a result of grading. Therefore, the littoral zone of the pond does not meet the required parameters to be considered a wetland.

A March 23, 2021 inspection by biologists from USACE confirmed the presence of wetlands in the southwest portion of the study area (represented by the blue in Figure 3-3). The proposed perimeter security fence that would tie into the existing NGA perimeter structure in this area could cross over these wetlands.





3.2.5 Groundwater

The geology of the proposed action area lends itself to unconfined, shallow groundwater located approximately 15 to 20 feet below the ground surface (AECOM, 2021). Groundwater could become perched in lenses within the unconsolidated coastal plain sediments. Groundwater flow patterns on FBNA generally follow surface water drainage (U.S. Army, 2007). Fracture zones in the deeper, less weathered rock could result in higher water heads in some areas, but only minimal artesian conditions have been found to exist during previous sampling conducted on the vicinity of the study area (USACE, 2015). As a result of its previous uses under the EPG mission, there is contamination of groundwater, as outlined in the 2021 AECOM final feasibility report and described in more detail in Section 3.4. The contaminants include benzene, naphthalene, 2-methylnaphthalene, toluene, and ethyl benzene; however, the report, which includes data from 2006 to 2018 obtained from an array of monitoring wells within and adjacent to the project site, indicates the plume of contaminated groundwater is relatively stable and not migrating. Land use controls are in place on the site to prevent the withdrawal of groundwater for potable use.

The construction and operation of the Proposed Action will require the relocation of some of the monitoring wells. Any existing wells identified for relocation would be coordinated with Fort Belvoir Department Public Works (DPW) and closed in accordance with 12 VAC 5-630-450, *Well Abandonment*. See Section 3.4 for additional details of clean-up of the contamination.

3.2.6 Stormwater

As described in the earlier section on Surface Water (Section 3.2.1), the project site on FBNA is located within the Accotink Creek watershed. Existing stormwater management structures include the series of underground pipes draining the gravel parking area that discharge to the man-made stormwater pond on the eastern side of the study area. Stormwater not captured within this system is directed by existing topography, namely the downhill slope on the western portion of the Proposed Action study area that becomes characterized by the erosional gully that connects downstream into the unnamed tributary to Accotink Creek.

Stormwater runoff in urban areas is one of the leading sources of water pollution in the United States. Recognizing the importance of controlling stormwater generated from development, federal, state and local governments have adopted requirements. The following regulations apply:

Federal Requirements

• National Pollutant Discharge Elimination System (NPDES) - Section 402 of the Federal CWA, known as the NPDES program, requires permits for the discharge of pollutants from point sources and is administered by VADEQ through its Virginia Stormwater Management Program (VSMP). Fort Belvoir operates a municipal separate storm sewer system (MS4) for the entirety of the installation (including FBNA) pursuant to the NPDES regulations, and discharges stormwater runoff under VPDES Stormwater Permit No. VAR040093. Stormwater runoff generated by development on FBNA, including the

Proposed Action, would be included under the installation-wide permit, provided the proponent comply with its terms and conditions and coordinate with the appropriate personnel on Fort Belvoir.

Energy Independence and Security Act (EISA), Section 438 – federal projects 5,000 square feet or greater in size are required to maintain or restore pre-development hydrology. Guidance provided by the U.S. Environmental Protection Agency (EPA) promotes retaining rainfall on-site through infiltration, evaporation/transpiration, and re-use to the same extent as occurred prior to development. Section 438 requires that practices known as low impact development (LID) or green infrastructure, including reducing impervious surfaces and using vegetative practices, porous pavements, cisterns and green roofs be incorporated into development plans https://www.epa.gov/sites/production/files/2015-09/documents/eisa-438-factsheet.pdf.

LID is a stormwater management approach that emphasizes the retention of native vegetation and soils, reduces runoff, and seeks to approximate predevelopment hydrologic conditions. LID provides an effective alternative to more traditional stormwater management approaches that rely on engineered structures. When properly used, LID can be cost effective by reducing the reliance on hard structures. It can make more efficient use of land resources by reducing the need for large, centralized stormwater basins, decreasing the total amount of runoff generated, and providing water-quality improvements. (HDR, 2020)

VADEQ Requirements

- Stormwater Management Act (9VAC25-870)
 - o General Permit for Discharges of Stormwater from Construction Activities
 - Virginia BMP Clearinghouse
 - Virginia Runoff Reduction Method
- Erosion and Sediment Control Law (9VAC25-840)
 - Erosion and Sediment Control Plan
 - Virginia Erosion and Sediment Control Handbook
- Chesapeake Bay Preservation Area Designation and Management (9VAC25-830-130)
 - Construction activities disturbing one or more acres, requires:
 - o General Permit for the Discharge of Stormwater from Construction Activities
 - Stormwater Pollution Prevention Plan (SWPPP), developed by the project proponent, requires stormwater management measures as included in the approved site plan, and demonstration of how these measures would be maintained, identifying the responsible entity throughout duration of construction.

Installation Requirements

• Fort Belvoir, DPW, reviews all construction site plans involving 2,500 square feet or more of earth disturbance for compliance with the installation's municipal separate storm sewer

system (MS4) conditions, state requirements for stormwater management and erosion/sediment control, and the Fairfax County Public Facilities Manual.

3.2.7 Coastal Zone

The Coastal Zone Management Act (CZMA) of 1972 (16 USC §1451 et seq., as amended) provides assistance to the states, in cooperation with federal and local agencies, for developing land and water use programs in coastal zones. Section 307 (c)(1) of the CZMA Reauthorization Amendment stipulates that federal projects that affect land uses, water uses, or coastal resources of a state's coastal zone must be consistent to the maximum extent practicable with the enforceable policies of that state's federally approved coastal management plan. The Commonwealth of Virginia has developed and implemented a federally approved Coastal Resources Management Program (CRMP) describing current coastal legislation and enforceable policies. There are enforceable policies for:

- Fisheries management
- Subaqueous lands management
- Wetlands management
- Dune management
- Non-point source pollution control
- Point source pollution control
- Shoreline sanitation
- Air pollution control
- Coastal lands management

Virginia's Coastal Zone includes all of Fairfax County, including Fort Belvoir; therefore, federal actions at Fort Belvoir are subject to federal consistency requirements. The VADEQ serves as the lead agency for consistency reviews. The project area is characterized as previously disturbed, with a gravel parking lot, unpaved and paved roads, and areas of forest, wetlands, and grass/shrub groundcover. While there are streambanks adjacent to the project area, there is no coastline present, nor dunes.

The proposed construction would be consistent with Virginia's Coastal Resources Management Policies. Non-point source pollution would be managed through the use of temporary erosion and sediment control measures defined in the approved Erosion and Sediment Control Plan or permanent stormwater management BMPs, as appropriate. The Coastal Zone Consistency Determination will be submitted to the Commonwealth of Virginia as an appendix in the Final EA/Draft FNSI. Complete results of this coordination, including recommendations from VADEQ, when received, will be presented in Appendix A.

3.2.8 Environmental Consequences

3.2.8.1 Threshold of Significance

The threshold of significance for water resource and surface water quality impacts would be exceeded if the alternative would result in changes to regional groundwater patterns or depletion of groundwater, alteration of local surface water, or substantial degradation of water quality. The threshold of significance for wetlands/RPA and floodplains would be exceeded if the alternative would result in substantial degradation of wetlands without mitigation, and notable adverse impact on natural and beneficial floodplain values.

In regard to coastal zone resources, the threshold of significance would be exceeded if the alternative would not be consistent with the federal coastal zone policy, including consideration of the following:

- Impacts of the Proposed Action on any land or water use or natural resource of the coastal zone;
- Incremental impacts of the Proposed Action on any land or water use or natural resource of the coastal zone when added to past, present, and reasonably foreseeable future actions; and,
- Collective impacts of individual unrelated actions on any land or water use or natural resource of the coastal zone.

3.2.8.2 Impacts of the Proposed Action

Surface Waters and RPAs

Implementation of the Proposed Action would result in less-than-significant adverse effects to surface water. The Proposed Action could involve minimal construction in, on, or over surface waters (i.e. wetlands or streams) and could result in the disturbance, alteration, or filling of the adjacent RPAs on the eastern portion of FBNA. Short-term, less-than-significant effects would result from the destabilization of the soils within the limits of disturbance as a result of vegetation clearing and excavation/grading to prepare the site. This stage of construction exposes soils and increases the potential for erosion and discharge of sediment-laden stormwater to downstream receiving waters; however, appropriate erosion and sediment control measures would be implemented, pursuant to the construction SWPPP and the VSMP Construction General Permit, and would minimize any detrimental effects.

Construction of permanent stormwater management features will handle stormwater generated from the development and be designed to maintain pre-development levels of off-site discharge. It is expected that the overall effects of construction and operation of the building would be beneficial to downstream receiving waters through stabilization of soils through vegetation and retention and treatment of stormwater flows.

Through the site layout design process, all practicable steps will be made to avoid inclusion of the unnamed tributary to Accotink Creek, and its associated RPA, within the LOD. Any work within the stream and RPA would be appropriately permitted through the USACE and the Commonwealth of Virginia. Activities during construction would include appropriate BMPs to minimize sediment transport and erosion consistent with state and federal land and water quality criteria.

Wetlands

Implementation of the Proposed Action could affect wetlands, as there may be approximately 0.02 acres of jurisdictional wetlands within the limits of disturbance. As the project plans are in the early stages of development, project designers will be encouraged to consider avoidance of these wetlands by relocating the perimeter fence alignment. Any unavoidable impacts will be permitted through the USACE and Commonwealth of Virginia's wetland permitting programs. Stormwater generated from within the project site during construction would be appropriately handled through erosion and sediment control measures required through the permitting process, preventing adverse effects of sedimentation to downstream receiving waters that include wetlands. Permanent stormwater management features would maintain pre-development levels of stormwater discharge.

Groundwater

Under the Proposed Action, no adverse effects are expected to occur to groundwater. The construction of the Proposed Action would result in an increase of impervious surface area, reducing the infiltration of stormwater into the shallow, near-surface aquifer; however, due to the existing plume of groundwater contamination within the project footprint, stormwater management features for the Proposed Action will be required to retain all stormwater volume on site and will not be allowed to infiltrate into subsurface groundwater.

Floodplains

Under the Proposed Action, no adverse effects are expected to occur as a result of floodplain alterations. The Proposed Action is not located within a floodplain.

Coastal Zone

Both construction and operation of the Proposed Action would be consistent with Virginia's CRMP. Any impacts to streams would be mitigated through contributions to habitat restoration at the installation's mitigation sites. Non-point source pollution would be managed through the use of temporary erosion and sediment control measures defined in an approved Erosion and Sediment Control plan or permanent stormwater management BMPs, as appropriate.

Based on this EA, Fort Belvoir has determined that the Proposed Action would be consistent, to the maximum extent practicable, with the Commonwealth of Virginia CRMP's enforceable policies, as described in Appendix C, Determination of Consistency with Virginia's CRMP. Review and concurrence with the negative determination will be requested prior to initiating the Proposed Action.

Stormwater

Under the Proposed Action, less-than-significant adverse effects would occur to stormwater. The Proposed Action would add approximately 0.74 acres of impervious area within the Accotink Creek watershed, resulting in an increase in storm water volume from impervious surfaces that could cause an increase in erosion and sedimentation if not appropriately controlled. The Proposed Action will meet all applicable stormwater management regulations, ensuring consistent and measurable steps to minimize detrimental impacts to water quality in downstream waters. As stated earlier, approximately 87 percent of land (45 square miles) within the watershed is developed, while about 28 percent (14 square miles) is covered by impervious surfaces. In the context of this 52 square mile watershed in central Fairfax County, which encompasses all of FBNA, this increase would be minimal and would be offset by stormwater management strategies such as the approximately 2-acre stormwater management pond proposed within the eastern portion of the project area. Petroleum pollutants from the exposed surfaces of the parking garage and associated paved roadways would be treated through vegetated buffers and stormwater management structures.

Because the project is located within a Chesapeake Bay Preservation Area and would disturb more than 2,500 square feet, the contractor would be required to prepare an erosion and sediment control plan in compliance with the Virginia Erosion and Sediment Control Law (9 VAC 25-840) and in conformance with the Virginia Erosion and Sediment Control Handbook, Third Edition, 1992. The plan would be submitted to Fort Belvoir's Stormwater Permit Manager for review and approved by VADEQ's Northern Regional Office (NRO) and routine inspections would be conducted throughout construction to ensure compliance with these permits. As noted in Section 3.2.6, the contractor would also obtain a Construction General Permit and prepare a construction SWPPP to minimize sedimentation to downstream receiving water bodies.

This project and any construction activities associated with it has the potential to discharge pollutants in surface waters to monitored/permitted Industrial Stormwater Outfall (ISW RO-031 and RO-032). This outfall is continually monitored for Total Suspended Solids (TSS), Total Petroleum Hydrocarbons (TPH), chloride, specific conductance, nitrogen and phosphorous, along with other constituents, therefore, any uncharacteristically high sediment content in the stormwater discharge detected at sampling could result in a violation of the VA0092771 permit. The construction contractor must contact DPW's Industrial Stormwater Section when construction begins and ends, so that precautions can be employed in the course of routine sampling events for this outfall. Also, construction as-builts of the new stormwater system will be required and must be submitted to DPW's Environmental Division.

Construction best management practices (BMPs) would be implemented in accordance with federal, state, and local Fort Belvoir regulations, including Fort Belvoir's MS4 Program and VPDES Permit VA0400093, to protect downstream waters from sediment migration by ensuring adequate perimeter controls and buffers are used, including silt fencing, synthetic hay bales, and similar measures. While these measures would not completely eliminate the potential for erosion and sedimentation, they would ensure that short-term adverse impacts remain negligible.

Use of appropriate erosion and sediment control measures and long-term LID measures would ensure that neither the construction nor the operation of the Proposed Action would contribute to further degradation of water quality or exceed TMDLs established for Accotink Creek as regulated under Section 303(d). Therefore, short-term and long-term detrimental impacts on surface water quality on and in the vicinity of FBNA would be negligible.

The master plan for Fort Belvoir envisions the FBNA as a future center for an intelligence community integrated campus, with mid- and long-term additions of more buildings and associated infrastructure including roads, parking and stormwater management facilities. This additional build-out would add more impervious surfaces to FBNA. Construction of an extension of Heller Road, to form a loop (with Barta Road) around the eastern portion of FBNA could potentially impact Accotink Creek and associated wetlands. Project proponents would be expected to obtain coverage under applicable permits issued by USACE and VADEQ in accordance with the CWA and would adhere to avoidance, minimization and compensatory mitigation to ensure that impacts to regulated waters would remain minor, and the resulting cumulative impacts would not be significant.

3.2.8.3 Impacts of the No Action Alternative

Surface Waters and RPAs

Under the No Action alternative, less-than-significant adverse effects would occur to surface water; existing conditions on the project site would continue. There would be no man-made alteration of the current pattern of surface water flows across and discharging from the study area. The erosional feature discharging to the unnamed tributary to Accotink Creek would likely continue to experience further downcutting, contributing to sediment loads downstream. There would be no alteration or construction within the RPA.

Wetlands

The No Action alternative would not impact jurisdictional wetlands on FBNA. Runoff would continue to discharge with no enhanced treatment for volume, velocity or sedimentation downstream to tributaries of Accotink Creek and associated floodplain wetlands.

Groundwater

The No Action alternative would have no effect on groundwater. The current level of infiltration of stormwater would remain unchanged and would have a negligible effect on the rate of natural attenuation of the groundwater contamination documented within the site.

Floodplains

Under the No Action Alternative, no adverse effects are expected to occur as a result of floodplain alterations because no construction would occur within a floodplain.

Coastal Zone

The No Action Alternative would have no impacts on the Virginia Coastal Zone or future implementation of the Coastal Resources Management Plan.

Stormwater

There would be no increase in impervious surfaces on FBNA. Stormwater would continue to be directed to the existing stormwater management pond to the east of the project area, and through the erosional features downslope and west of the project area, which ultimately connect to the intermittent tributary to Accotink Creek. The compacted nature of the gravel parking lot allows for minimal infiltration of rainwater and the accelerated flows through these erosional features would continue to result in further erosion and sedimentation (the rock weirs emplaced would not function properly), thus resulting in a continued, minor, detrimental effect on downstream waters of Accotink Creek and Accotink Bay.

3.3 BIOLOGICAL RESOURCES

Located on the western shore of the Potomac River, within the larger metropolitan area of Washington, D.C., Fort Belvoir sustains its military mission while maintaining relatively large areas of native vegetation in terms of size, diversity and regional position. Fort Belvoir has recognized the ecological importance of its natural habitats by designating three refuges, two biological corridors, wetlands and steep-sloped areas as environmentally constrained areas (Fort Belvoir, 2017). These large areas of native vegetation afford a contiguous band of wildlife habitat within and extending outside of the installation. Fort Belvoir's natural resources management strategy, outlined in its INRMP, prioritizes preserving the native diversity of communities and species within communities and implements an ecosystem-based natural resources management program based in part on DoD Instruction 4715.3, *Natural Resources Conservation Program* and Army Regulation 200-1, *Environmental Protection and Enhancement*, to guide development on Fort Belvoir.

The Accotink Bay Wildlife Refuge, Jackson Miles Abbott Wetland Refuge, T-17 Refuge, Accotink Creek Conservation Corridor, and Forest and Wildlife Corridor are designated Special Natural Areas by Fort Belvoir.

The Accotink Creek Conservation Corridor, located within FBNA, was designated as a Special Natural Area in 2005 as a mitigation measure associated with the 2005-era base realignment and closure actions (BRAC) and serves to protect the Accotink Creek riparian area within the boundaries of FBNA. This predominantly forested 191-acre area serves as a wildlife migratory

corridor and supports potential habitat for federally listed small whorled pogonia and several other species of management concern (Fort Belvoir, 2017).

Biological resources discussed in the following sections include vegetation, wildlife, threatened and endangered species, and Partners in Flight habitat. Relevant regulations and policies are also discussed when applicable. The area of analysis for biological resources focuses on the project study area, taking into account a broader geographic range when appropriate.

3.3.1 Vegetation

Approximately seven (7) acres of the project area is occupied by a gravel parking lot. The southern portions of the site, bounded by Heller Loop, have been partially planted with Eastern red cedar (*Juniperus virginiana*). This approximately 9-acre area of plantings was established to offset removal of vegetation associated with construction of the additional NGA Campus East (NCE) 900-space, 7-acre overflow surface parking lot located to the north of the project area, as stipulated in a March 20, 2008 memorandum between USACE and Fort Belvoir (USACE, 2015). The cedars in this area remain less than five (5) feet in height and are surrounded by tall grasses, supporting avian species that require open field habitat.

The western periphery of the Proposed Action area slopes downward from the gravel parking lot into a previously disturbed area with uneven topography and a mixture of upland field grasses and Virginia pine stands until it is intercepted by GEOINT Drive. South of GEOINT Drive the Proposed Action area is characterized by a narrow (ranging from approximately 50 to 200 feet) swath of pines that form a visual screen between the NGA perimeter patrol path and the open, grassy field. The eastern periphery of the project area is formed by the remnant, concrete-paved test track of Heller Loop paralleled by a stand of Virginia pine.

In the above-referenced 2008 Memorandum, USACE committed to Fort Belvoir's tree replacement requirement by agreeing to restore areas of vegetation cleared outside the primary NCE construction area's limit of disturbance, including the North Subcontractor Parking Lot and adjacent areas serving as temporary construction management infrastructure. The intent was to restore these temporarily impacted areas to their original condition or better, replacing trees and vegetation lost as a result of that clearing. USACE developed a re-vegetation plan (USACE, 2010) for those areas in accordance with the requirements of the memorandum. This 2008 memorandum and the 2010 planting plan include the area within and surrounding the Proposed Action.

In partial fulfillment of the restoration requirements set forth in the 2008 USACE Memorandum and subsequent planting plan, USACE and NGA planted areas in the southwestern and eastern portions of the NCE project site with landscape size cedar trees at 20 trees per acre, and pine seedlings at 480 seedlings per acre (USACE, 2015). The full requirements of the planting plan have not been fulfilled to date. Fort Belvoir DPW, Environmental Division's natural resources staff perform routine, yearly surveys of this area, as it is designated as a mitigation area, and have indicated that in its current condition it does not meet the standards of the planting plan.

Fort Belvoir's Tree Removal and Protection Policy requires the protection of existing trees and, where tree loss is unavoidable, mitigation for the removal of trees must be performed unless expressly exempted. In-kind mitigation measures include replacing any trees four inches or greater in diameter at breast height (dbh) that are removed with the planting of two new trees. Out-of-kind compensatory mitigation, such as environmentally beneficial restoration, enhancement, or preservation measures may be completed if in-kind mitigation is not a feasible option (Fort Belvoir, 2018). Pursuant to the Tree Removal and Protection Policy, a Tree Protection Plan must be prepared in accordance with DPW requirements and included as part of the 35% design submittal for construction projects.

The Proposed Action will implement a mitigation planting plan in consideration of the installation's current tree removal policy and the existing mitigation status of the Proposed Action study area (Figure 3-5). USACE and DIA, as the project proponent, will work closely with DPW's natural resources staff to identify requirements.

3.3.2 Wildlife

A wildlife survey was conducted on FBNA in 2006 (U.S. Army, 2007). Mammals present consisted predominantly of white-tailed deer (*Odocoileus virginianus*), Virginia opossums (*Didelphis marsupialis*), and gray squirrels (*Sciurus carolinensis*). By 2008, much of the project area was cleared, graded, and supported construction equipment and temporary buildings associated with the NGA construction, but at the conclusion of construction, equipment and materials were removed and the area was allowed to revegetate as described in Section 3.3.1. The establishment and growth of Virginia pine trees has allowed populations of the common woodland mammals listed above to re-establish themselves. Further, the maintenance of the open grassland on the southern portion of the project area supports mammal species favoring old fields such as eastern cottontails (*Sylvilagus floridanus*), field mice (*Peromyscus* sp.), opossums, and groundhogs (*Marmota monax*). Reptile species that favor the mix of uplands and wetlands, as well as old-field habitat, on FBNA include eastern garter snakes (*Thamnophis sirtalis*), black racers (*Coluber constrictor constrictor*) and the eastern box turtle (*Terapene carolina carolina*).

Accotink Creek, along with its tributaries and associated floodplain wetlands, supports amphibian species including spring peepers (*Pseudacris crucifer*), American toads (*Bufo americanus*), Fowler's toads (*Bufo woodhousii fowleri*), and bullfrogs (*Rana catesbeiana*). The stormwater management pond on the eastern portion of the study area would also likely support these species.

The assortment of common animal species is typical of animals tolerant of disturbed, urbanized areas with fragmented stands of forest and in close proximity to traffic and associated human activity. More suitable habitat for biologically diverse species assemblages can be found west of the project site along the Accotink Creek Conservation Corridor.

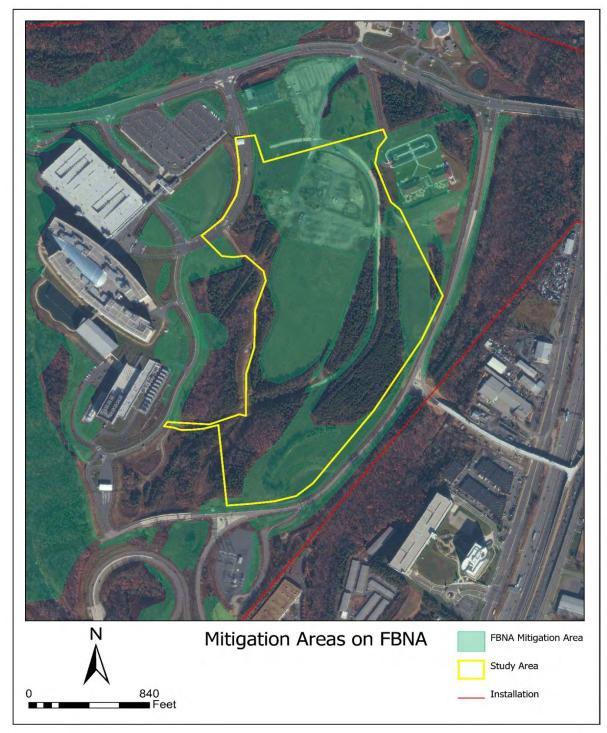


Figure 3-5: Tree Mitigation Areas on FBNA

3.3.3 Rare, Threatened and Endangered Species

Under the Endangered Species Act (ESA) of 1973, plant and animal species in danger of extinction throughout all or a significant part of their range are listed as "endangered." Species that are likely to become endangered within the foreseeable future are listed as "threatened." The USFWS is responsible for administering the ESA for terrestrial and freshwater organisms, as may be found within the project site and its vicinity. The ESA establishes the federal government's responsibility for protection and recovery of species considered to be in danger of extinction. The ESA requires federal agencies, in consultation with the USFWS to ensure that actions they authorize, fund, or carry out are not likely to jeopardize the continued existence of any listed species or result in the destruction or adverse modification of designated critical habitat of such species. Critical habitat can include areas not occupied by the species at the time of the listing, but that are essential to the conservation of the species.

Federally Listed Species

Section 7 of the ESA requires federal agencies to "request of the Secretary information whether any species which is listed or proposed to be listed may be present in the area of such proposed action" for any project that is conducted, permitted, funded, or licensed by any federal agency. Based on project area screening using the USFWS' Information for Planning and Conservation (IPaC) online tool, the northern long-eared bat (*Myotis septentrionalis*) (NLEB), listed as a threatened species under the ESA, may occur in forested areas on or near the project study area. No critical habitat has been designated for this species. White-nose syndrome, a fungal disease known to affect bats, is the most severe and immediate threat to NLEB survival and is the basis for the listing of the species as threatened. During the active season (April 1 to October 31), bats roost singly or in colonies in cavities, underneath bark, crevices, or hollows of both live and dead trees and snags.

USFWS signed a Programmatic Biological Opinion (BO) 5 January 2016 on the Final 4(d) Rule that addresses effects to the NLEB by federal actions and provides for a streamlined section 7 consultation. USFWS has not yet designated critical habitat for NLEB. However, incidental clearing of vegetation would not occur during the northern long-eared bat active season from April 15 through September 15.

The IPaC screening also lists small-whorled pogonia (*Isotria medeoloides*) as potentially present within the project area. The small-whorled pogonia is an orchid listed as federally threatened throughout its range and listed as state-endangered by the Commonwealth of Virginia. In Virginia, small-whorled pogonia is most typically found in deciduous second or third growth successional hardwood forests with fairly sparse ground cover and highly acidic, nutrient-poor, sandy loam soils, although plants have been found in a wider range of habitats in recent years. To date, FBNA is the only location in Fairfax County where the small-whorled pogonia has been found (U.S. Army, 2007). The small-whorled pogonia was observed in the summer of 2005 on steep, oak-dominated forested slopes on a first order tributary of Accotink Creek in the southwestern part of

FBNA. Areas of FBNA that have been identified as potential suitable habitat for the small-whorled pogonia are along the western and southern boundaries of FBNA.

No small-whorled pogonia have been documented within the project study area; however, it is Fort Belvoir's practice to require a survey for small-whorled pogonia at any site where suitable habitat may occur prior to issuance of a Fort Belvoir building permit, which would be required for the Proposed Action. If the small whorled pogonia, or any other listed species, is encountered at any proposed study area, Fort Belvoir would coordinate a biological assessment with USFWS before approving the project, to develop an appropriate mitigation plan if the plant cannot be avoided.

Mapping associated with the Fort Belvoir INRMP, shown in Figure 3-6, indicates there is an approximately 0.45-acre area of potentially suitable habitat for small-whorled pogonia within the southwest portion of the Proposed Action study area. This portion of the study area supports a vegetative community that has experienced much less disturbance compared to the remainder of the proposed study area.

State-Listed Species

Virginia has also promulgated a state endangered species act that provides endangered and threatened listings for species vulnerable to extinctions at the state level. The Virginia statute (4 VAC 15-20-130) prohibits the taking, transportation, possession, sale, or offer for sale within the state any species listed on the federal endangered species list or any other species designated by the state board. The Commonwealth also provides protection for plant and insect species through Chapter 10 §3.2- 1000 of the Code of Virginia. It is the role of Virginia's Department of Conservation and Recreation, Division of Natural Heritage to maintain listings and rarity (i.e., conservation) rankings of rare plant and animal species and ecological communities. Unlike endangered and threatened listings, rare species listings and their rankings are not legal designations and do not provide any protective status, but, rather, are used to prioritize resources for conservation.

Fort Belvoir has five state-listed animal species that occur on the installation, including the statelisted threatened wood turtle (*Glyptemys insculpta*), the state-listed endangered peregrine falcon (*Falco peregrinus*, during fall migration), the state-listed endangered little brown bat (*Myotis lucifugus*), the state-listed endangered tri-colored bat (*Perimyotis subflavus*), and the state and federally listed threatened NLEB. Potential habitat for the wood turtle is primarily located along Accotink Creek and its tributaries. However, this species is also known to traverse connected deciduous woodlands within 300 feet of resident waterways. The peregrine falcon has been regularly recorded on Fort Belvoir as it migrates through the regional area and takes advantage of foraging habitat along the Accotink Creek/Accotink Bay stream corridor. The little brown bat and the tri-colored bat have an active season similar to that of the NLEB. The conservation measures outlined by the state include time of year restrictions that fall within the bounds of the time of year restrictions already established for the NLEB. Therefore, the conservation measures required for protection of the NLEB would also be adequate for protection of the state-listed bat species. Although field surveys have not identified any listed threatened or endangered plant or animal species within the project study area, construction would be coordinated in accordance with Department of Wildlife Resources (DWR) guidance to avoid impacts to protected species. This would include conducting preconstruction protection surveys for wood turtles and installation of silt fencing around potential wood turtle habitat areas during the winter months to exclude wood turtles from proposed construction areas. Any turtles found during pre-construction screening of the fenced area shall be relocated by trained personnel in accordance with DWR guidance to avoid impacts. Preconstruction verification surveys for small-whorled pogonia would also be included as part of preconstruction activity coordination. Seasonal land clearing requirements would also be followed to reduce potential impacts to protected bird and bat species.

3.3.4 Partners in Flight

The DoD Partners in Flight (PIF) program uses a cooperative network of natural resources personnel from military installations across the United States to sustain and enhance the military mission through proactive, habitat-based conservation and management strategies that maintain healthy landscapes and training lands (<u>https://partnersinflight.org/</u>). The DoD PIF uses voluntary partnerships at local, state, regional, national and international levels to share information and develop ecosystem-based, proactive management programs and programmatic priorities that aim to "keep common birds common" and help recover species at risk. The USFWS, as well as state wildlife agencies such the Virginia Department of Wildlife Resources (VDWR), through the state nongame program, are also partners in this program.

As part of the PIF Program, DoD installations are encouraged to incorporate elements of the Partners in Flight Bird Conservation Strategy into their INRMPs. Such elements include habitat management practices such as prescribed burning and timber management programs. Designation of regional PIF priority bird species is the result of a cooperative/coordinated effort among various federal, state and private organizations. Fort Belvoir has designated approximately 4,200 acres of PIF habitat within its boundaries, most of it within the 1,480-acre Accotink Bay Wildlife Refuge along Accotink and Pohick Bays, and the 234-acre Jackson Miles Abbott Wetland Refuge along Dogue Creek, both areas of high-quality habitat located within Main Post. These large areas of habitat not only are valuable in and of themselves, but also provide for ecological connectivity through the installation to other regional habitats (USACE, 2015).

PIF Species of Concern (SOC) status and applicable conservation guidelines are part of a broader designation identified by the INRMP as Fort Belvoir Breeding Birds of Management Concern, and includes USFWS Birds of Conservation Concern, DoD PIF Mission Sensitive Species and Fort Belvoir Habitat Indicator Species in addition to the PIF SOC for Bird Conservation Region 30 (New England/Mid-Atlantic Coast). The prairie warbler (*Setophaga discolor*) and wood thrush (*Hylocichla mustelina*) are Fort Belvoir Breeding Birds of Management Concern species documented on FBNA (USACE, 2017). Documented occurrences of these species include GIS mapping of a 500-foot buffer to provide protections for potential nesting and foraging areas (Figure 3-6). FBNA supports approximately 396 acres of designated habitat for PIF species (USACE, 2015). PIF management recommendations include maintaining upland forest habitat (to support

wood thrushes) and creating and maintaining successional/shrub-scrub habitat (to support prairie warblers) (Fort Belvoir, 2017).

3.3.5 Environmental Consequences

3.3.5.1 Thresholds of Significance

The threshold of significance for biological resources impacts would be exceeded if the alternative would jeopardize the continued existence of any federally listed threatened or endangered species or result in destruction of critical habitat; decrease the available habitat for commonly found species to the extent that the species could no longer exist in the area; eliminate a sensitive habitat such as breeding areas, habitats of local significance, or rare or state-designated significant natural communities needed for the survival of a species; or substantially degrade or minimize habitat.

Potential impacts to plants, wildlife, and fish are evaluated in accordance with applicable regulations including but not limited to the Endangered Species Act of 1973, the Fish and Wildlife Conservation Act of 1980, the Magnuson-Stevens Fishery Conservation and Management Act, as amended, the Migratory Bird Treaty Act, and EO 13112 on Invasive Species. The Sikes Act provides for cooperation by the Department of the Interior and DoD with state agencies in planning, development, and maintenance of fish and wildlife resources on military reservations throughout the United States. The area of analysis for biological resources includes the project study area.

3.3.5.2 Impacts of the Proposed Action

Vegetation

Under the Proposed Action, less-than-significant adverse effects would occur to vegetation. Vegetation within the limit of disturbance for the project area would be cleared to prepare the site for construction. Upon completion of construction, the Proposed Action area would be landscaped with grass, shrub and tree species coordinated with the Fort Belvoir natural resources program staff to ensure no invasive species are utilized, and planting enhances wildlife habitat in a low-maintenance manner consistent with master planning objectives. While the character of the area would change from that of a mixture of grass field and pine/hardwood stands to a campus-like landscaped setting, it would provide for the continued removal of invasive vegetative species and upkeep of desirable, native species throughout the life cycle of the building, thus resulting in an overall long-term beneficial effect.

Removal of approximately 7 acres for construction of the Proposed Action would result in temporary, minor adverse effects on open field and pine stand habitat on FBNA. This would be offset by a combination of replanting within the project's limits of disturbance (LOD), and replanting and/or enhanced planting within other areas of Fort Belvoir, in consultation with Fort Belvoir natural resource specialists and in accordance with Fort Belvoir's Tree Removal and Protection Policy. A tree survey was conducted by USACE biologists on March 23, 2021 to

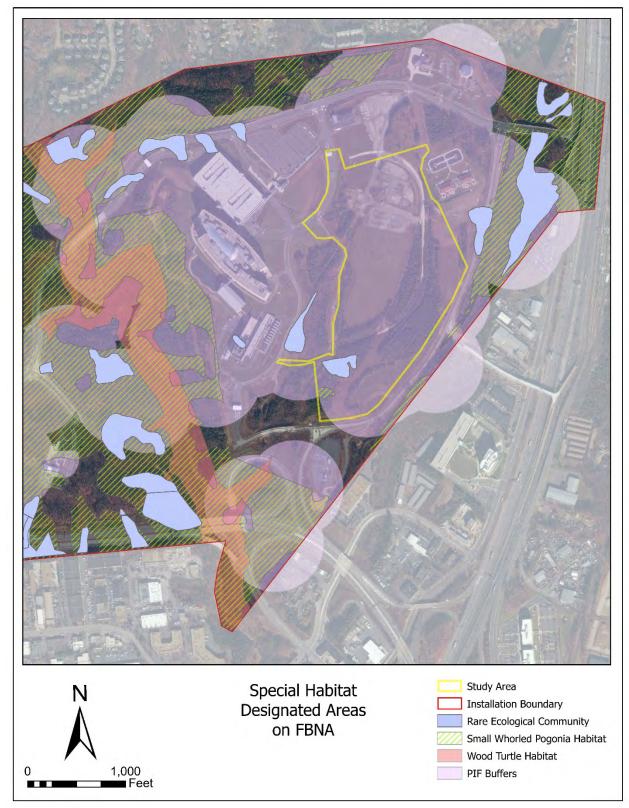


Figure 3-6: Special Habitat Designated Areas on FBNA

characterize and quantify the forest resources within the study area to support determination of appropriate mitigation (USACE, 2021).

Wildlife

Under the Proposed Action, less-than-significant adverse impacts would occur to wildlife. During construction of the Proposed Action, equipment noise, ground disturbance and vegetation removal would temporarily displace individual species of common wildlife residing in the LOD. There may be limited mortality to individual species that are not able to relocate during construction. However, population-level impacts would not reasonably occur due to the relatively small size of the construction area in relation to the overall size of FBNA. Additionally, most mobile species are able to safely avoid equipment. Therefore, construction activities associated with the Proposed Action are expected to result in short-term, negligible, direct, adverse effects on terrestrial wildlife resources located within the immediate work area.

To minimize impacts to birds, construction activities would avoid cutting and removal of vegetation from April 1 to July 15. If cutting and removal occurs in this time frame, a survey for birds and active bird nests is recommended. No bird, active nest, egg, or hatchling can be disturbed, removed, damaged, or destroyed per the Migratory Bird Treaty Act.

Following completion of construction, the Proposed Action would replace a vacant, infrequently used area into an administrative headquarters with associated parking areas, an operational plant and security fencing. Wildlife accustomed to frequent human activity would use the new environment, while species requiring less disturbance and more secrecy would likely relocate. Planting of native vegetation near buildings and in open spaces within the campus would support habitat needs of species typically found within the project area and would serve as an extension of the stream corridor to the west of the site. The long-term adverse or beneficial effects of operation of the Proposed Action on wildlife are expected to be negligible.

Rare, Threatened, & Endangered Species

Under the Proposed Action, less-than-significant effects would occur to Rare, Threatened and Endangered (RTE) species. The Proposed Action occurs in a location that has had extensive prior disturbance, most recently as a staging area for the NGA facility construction between 2007-2008, and prior to that as an area supporting testing facilities as part of the Engineering Proving Grounds mission from the 1950's to the 1990's. While the study area includes areas mapped as potential habitat for the small-whorled pogonia, its presence would be highly unlikely.

Despite the disturbed nature of the study area, clearing of vegetation associated with construction of the Proposed Action could adversely impact protected species if pre-construction surveys are not conducted. Surveys for the presence of the wood turtle and small-whorled pogonia would be conducted prior to site clearing, and the results of these surveys coordinated with Fort Belvoir's

natural resources staff and the appropriate wildlife agencies. Perimeter controls would be installed during the winter months to exclude the endangered wood turtle from proposed areas of construction activity, as necessary. In order to protect nesting bat species, no trees over 3 inches in diameter would be removed within the project study area between April 15 and September 15, in accordance with current USFWS guidelines and corresponding U.S. Army NLEB protection documents promulgated to protect the northern long-eared bat species.

Partners in Flight

Under the Proposed Action, less-than-significant effects would occur to Breeding Birds of Management Concern. DIA will work with Fort Belvoir natural resources personnel to identify means to offset the loss of PIF habitat associated with the construction of the Proposed Action.

3.3.5.3 Impacts of the No Action Alternative

Vegetation

The No Action Alternative would have no effect on vegetation and existing conditions would continue. The area of restoration plantings would not be developed and would continue to provide habitat for faunal species that need open field habitat, but maintenance of the area to prevent succession to forest would be dependent on continued maintenance by DPW. The adjoining subcontractor gravel parking lot would continue to be used for overflow parking, resulting in periodic episodes of human activity and disturbance.

Wildlife

Under the No Action alternative, no changes would occur to existing wildlife and wildlife habitat.

Rare, Threatened and Endangered Species

Under the No Action alternative, no changes would occur to RTE species.

Partners in Flight

Under the No Action alternative, no changes would occur to habitat within the study area that supports Breeding Birds of Management Concern.

3.4 HAZARDOUS MATERIALS AND MUNITIONS

3.4.1 Hazardous Materials

Hazardous and toxic materials or substances are generally defined as materials or substances that pose a risk (i.e., through either physical or chemical reactions) to human health or the environment. Regulated hazardous substances are identified through a number of federal laws and regulations. The most comprehensive list is contained in 40 CFR 302, Designation, Reportable Quantities, and Notification, and provides quantities of these substances that, when released to the environment, require notification to a federal agency. Further, hazardous wastes, defined in 40 CFR 261.3, are considered hazardous substances. Generally, hazardous wastes are discarded materials (e.g., solids or liquids) not otherwise excluded by 40 CFR 261.4 that exhibit a hazardous characteristic (i.e., ignitable, corrosive, reactive, or toxic), or are specifically identified within 40 CFR 261. Petroleum products are specifically exempted from 40 CFR 302, but some are also generally considered hazardous substances due to their physical characteristics (i.e., especially fuel products), and their ability to impair natural resources.

Fort Belvoir conducts its hazardous waste management program in compliance with the Comprehensive Environmental Response, Compensation, and Liability Act (CERCLA), 42 United States Code (U.S.C.) 9605, as amended by the Superfund Amendments and Reauthorization Act of 1986 (SARA), Pub. L. 99-499. Fort Belvoir has a Hazardous Waste Management/Waste Minimization Plan and a Master Spill Plan. Fort Belvoir also participates in the "Greening of Government" program (EO 13101, "Greening" the Government through Waste Prevention) that promotes the purchase of products to reduce solid and hazardous waste through implementation of a centralized system for tracking procurement, distribution, and management of toxic or hazardous materials. Fort Belvoir Directorate of Public Works Environmental Division also files annual hazardous material and toxic chemical reports in compliance with the *Emergency Planning and Community Right-to-Know Act*.

Installation Restoration Program (IRP)

The Fort Belvoir IRP operates in conjunction with the U.S. Army Environmental Command and the USACE to restore former military training areas, waste sites, and petroleum areas through regulatory closure. The IRP is a comprehensive program designed to address contamination from past activities and restore Army lands to useable conditions. It is one of two programs established under the Defense Environmental Restoration Program (DERP) to identify, investigate and clean up hazardous substances, pollutants, and contaminants that pose environmental health and safety risks at active military installations and formerly used defense sites. The IRP was established in 1975 and is achieving successful restoration of more than 11,000 identified active Army environmental cleanup sites.

IRP response actions (i.e., site identification, investigation, removal actions, remedial actions, or a combination of removal and remedial actions) correct other environmental damage (such as the detection and disposal of unexploded ordnance) that poses an imminent and substantial

endangerment to the public health or welfare or to the environment. IRP actions are conducted according to the provisions of CERCLA, EOs 12580 and 13016, and the National Oil and Hazardous Substances Pollution Contingency Plan (40 CFR Part 300).

Site CC-MPS-2009 is located in an area of light industrial activity on the FBNA (Figure 3-7). The FBNA, formerly known as the EPG, is an 804.07-acre noncontiguous portion of Fort Belvoir that is located about 1.5 miles northwest of Main Post. CC-MPS-2009 consists of three former Petroleum Storage Areas (PSAs) (PSA-2009, PSA-2033, and PSA-2034) located within the project site. The USTs, along with approximately 508 tons of petroleum-contaminated soils, were removed in 1996-1997. Monitoring wells were installed and five phases of Environmental Investigation (EI) were conducted between 2006 and 2008 to determine the extent and severity of possible remaining contamination in both soils and groundwater. The EIs revealed little or no residual soil contamination at the three sites (AECOM, 2021).

Groundwater monitoring pursuant to the EIs detected constituents above residential U.S. EPA Region 3 Risk-Based Concentrations (RBCs) at PSA-2009, to include benzene, naphthalene, 2-methylnaphthalene, toluene, and ethyl benzene. PSA-2033 had groundwater contamination of naphthalene and 2-methylnaphthalene, and PSA-2034 had groundwater contamination of carbon tetrachloride. None of the plumes extended outside the FBNA property (AECOM, 2021). Investigations by AECOM in 2019 indicated the network of monitoring wells appeared to have been modified as a result of the BRAC construction on FBNA. The monitoring well network would need to be re-established in order to conduct future field investigations that would allow closure of the site.

A Human Health Risk Assessment (HHRA) was performed in 2011 using the information collected through the EIs, which identified residential groundwater and residential vapor intrusion (VI) chemicals of concern (COCs) at PSA-2009, PSA-2033, and PSA-2034 (AECOM, 2021). The residential exposure thresholds are more conservative than commercial and industrial levels and were the benchmarks used for the HHRA.

Due to the VI COCs identified in the HHRA, a supplemental remedial investigation (RI) was conducted in 2018 to evaluate potential VI impacts to future construction on the site, and included the collection of grab groundwater and soil gas samples within the PSA-2009 and PSA-2033 benzene, naphthalene, and ethyl benzene plumes. PSA-2034 was not included as part of this investigation because the COC identified in the HHRA (carbon tetrachloride) did not exceed the Vapor Intrusion Screening Level (VISL) screening criteria or the Virginia Voluntary Remediation Program (VRP) construction worker in a trench screening criteria (AECOM, 2021).

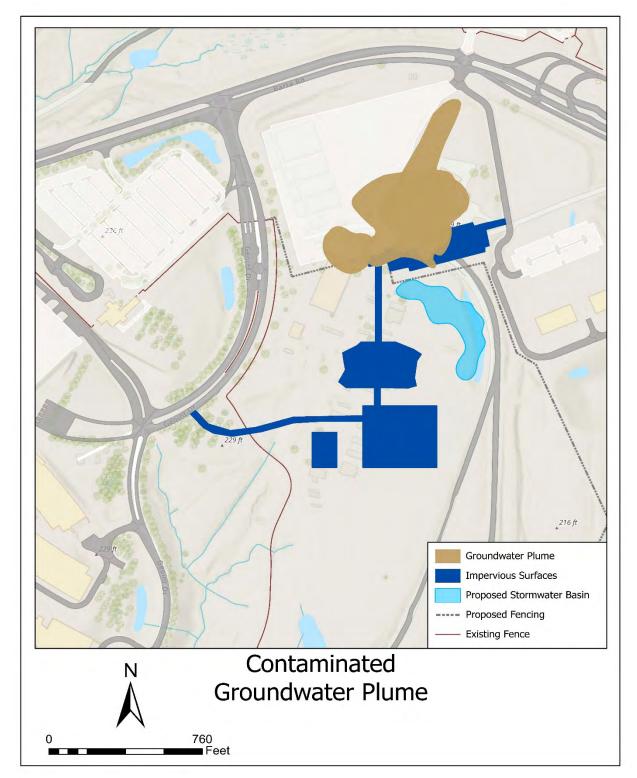


Figure 3-7: Contaminated Groundwater Plume

Based on an assessment of the groundwater samples collected during this investigation, concentrations have generally decreased relative to the sampling conducted between 2006 and 2008 for the chemicals relevant to the VI evaluation. The VI risk was determined to be unacceptable based on deep soil gas concentrations immediately above the groundwater table for PSA-2009 and their potential impact to indoor air. VI risk for PSA-2033 was acceptable for the future construction worker exposure scenario but unacceptable for the future commercial/industrial worker exposure scenario.

Subsequently, the *Focused Feasibility Study for CC-MPS-2009* (AECOM, 2021), which encompasses the three separate sites listed above, was commissioned to identify courses of action for safely closing out the contaminated sites. The remedial action objectives of the focused feasibility study (FFS) include limiting current and future use of the CC-MPS-2009 to non-residential; controlling, reducing or eliminating vapor intrusion and groundwater exposure pathways; and, achieving remedial goals for COC concentrations in groundwater.

While the series of investigations conducted between 2006 and 2018 indicated the dissolved phase COC plumes are stable and show signs of natural attenuation processes, there are no time-series data collected from consistent well locations to estimate biodegradation rates. All nonmetal COCs with concentrations above the PRGs are known to be biodegradable and, in general, the investigations have shown that the dissolved-phase concentrations have decreased over time (AECOM, 2021). A monitoring period of 10-20 years was recommended considering the remediation technologies available and recommended and the uncertainty of the attenuation rate resulting from the lack of consistent groundwater monitoring network. The alternatives recommended included 1) no action; 2) long-term monitoring and instituting land use controls (LUCs); 3) use of in situ chemical oxidation in conjunction with long-term monitoring and land use controls. Each alternative was evaluated considering effectiveness (including short and long-term effectiveness), implementability (technical and administrative feasibility, availability of services and materials, state and community acceptance), and cost.

Institutional controls (ICs) at CC-MPS-2009 will be implemented in the form of administrative, engineering, and access controls. Administrative controls prevent changes in land use or development at CC-MPS-2009. Administrative ICs will include notations in Fort Belvoir's Master Plan and GIS. Additionally, the Master Plan would include a notation requiring engineering controls to manage vapor intrusion risks for any new construction (i.e., vapor mitigation system or vapor barrier). The specific requirements are highly dependent on building design; however, for construction of new buildings, there are five basic components to effective vapor intrusion resistant construction that would need to be considered (NAVFAC, 2011):

- Permeable sub-slab support material (e.g., gravel),
- Venting all sub-slab areas below occupied spaces,
- Properly sized sub-slab and riser piping,
- A sealed vapor barrier, and
- If an active system is specified, a properly sized blower to maintain sufficient negative pressure beneath the slab.

Performance monitoring of the chosen vapor mitigation measure for future construction would also be required to demonstrate the effectiveness of the measure at restricting vapors from entering the structure. The performance monitoring approach would be developed based on the mitigation measure chosen.

Engineering and administrative controls will restrict the use of groundwater for potable or industrial purposes and would also require the installation of a vapor barrier for any building built in the area until preliminary remediation goals (PRGs) are met. A Land Use Control Implementation Plan (LUCIP) will also be generated to establish LUCs. Activity Hazard Analyses (AHAs) and Site Safety and Health Plans (SSHPs) would be required for any future intrusive work at the site.

The CC-MPS-2009 Feasibility Study (FS) was finalized in March 2021. Following the FS, the Proposed Plan (PP) and Record of Decision (ROD) will be completed within 18 months of award (expected completion in 2023). The PP and ROD will outline the formal decision to get the site to closure. Following completion of the PP and ROD, the remedial design (RD), restoration in place (RIP), and any necessary monitoring will be completed (expected to begin in 2024 and carry through the duration of the monitoring).

3.4.2 Munitions

Congress established the Military Munitions Response Program (MMRP) in 2001, under the DERP, to address munitions-related concerns, including explosive safety, environmental, and health hazards from releases of unexploded ordnance (UXO), discarded military munitions (DMM), and munitions constituents (MC) found at locations other than operational ranges on active and BRAC installations and Formerly Used Defense Sites (FUDS) properties. The MMRP provides a focused program to address the challenges presented at sites called munitions response sites. Munitions responses are response actions, including investigation, removal actions and remedial actions that address the explosives safety, human health or environmental risks presented by UXO, DMM, and MC (<u>https://aec.army.mil/index.php?cID=365</u>). Munitions response actions will be conducted under the process outlined in the National Contingency Plan (NCP) (40 CFR 300) as authorized by the CERCLA.

Given its historical use and concentration of ranges, all FBNA is considered a Military Munitions Response Program site (US Army, 2014). The ranges at FBNA were used for mine warfare material testing, research, and development. In 2006, the 10 closed ranges on FBNA were determined to be eligible for the DERP and were subsequently enrolled in the MMRP. Several former FBNA training ranges were successfully cleared of ordnance and explosives from 2003 through 2005 in preparation for the proposed land transfer for the Fairfax County Parkway right-of-way. Subsequent clearance occurred between 2006 and 2010 for the areas outside of the Fairfax County Parkway right-of-way in support of the 2005 BRAC-related construction. Fort Belvoir developed a Focused Feasibility Study (FFS) to evaluate remedial alternatives, as required by CERCLA (AECOM, 2021).

In preparation of the FBNA for re-development under BRAC, site investigations were conducted in 2007-2008 to characterize the nature and extent of potential munitions left over from the use of the area as a testing ground. The investigations gave particular focus to two former range areas on the western portion of FBNA and the former Ebee Field on the northern portion of eastern FBNA. Also, two larger, non-range areas, located on either side of Accotink Creek, were characterized using linear transects with surface and subsurface to two feet intrusive investigations.

The 2021 FFS indicates Fort Belvoir will implement LUCs at the FBNA. As part of the LUCs, all future ground disturbances and construction activities will be required to conduct munitions clearance per the U.S. Army Garrison (USAG), Fort Belvoir, Policy Memorandum #28 (USAG, 2014). Once the full munitions clearance is complete for areas prior to development, then the level of munitions clearance and construction support will depend on the results of the full clearance and the recommendations of munitions experts on a case-by-case basis. VADEQ will be notified of any munitions and explosives of concern (MEC)/discarded military munitions (DMM) discovered during these activities (AECOM, 2021).

3.4.3 Environmental Consequences

3.4.3.1 Thresholds of Significance

Effects on hazardous materials and wastes are assessed by evaluating the degree to which a proposed action could cause worker, resident, or visitor exposure to hazardous materials; whether the Proposed Action would lead to noncompliance with applicable federal or state regulations or increase the amounts generated or procured beyond current waste management procedures and capacities; and whether the Proposed Action would disturb a hazardous waste site, create a hazardous waste site, or contribute to a hazardous waste site resulting in adverse effects on human health or the environment.

Effects from UXO would occur if military munitions are inadvertently encountered, causing an unintended detonation or the release of munition chemicals to the environment.

3.4.3.2 Impacts of the Proposed Action

Hazardous Materials and Waste

Under the Proposed Action, no significant impacts would occur to hazardous material and waste. The construction contractor would be required to prepare and adhere to a Spill Prevention, Control, and Countermeasure (SPCC) plan that identifies practices to minimize the potential for accidental spills of petroleum products or other hazardous substances and the procedures for containing and cleaning up any accidental spills that may occur.

Construction of the building may require measures to prevent vapor intrusion in the below-ground levels. Also, site preparation may require the relocation of existing monitoring wells and re-

establishment of wells removed during previous site alterations. Re-establishment of the monitoring well network will be coordinated with Fort Belvoir DPW.

Implementation of the Proposed Action would not result in a significant effect on hazardous material concerns within the project area. Ongoing remedial actions would be enhanced through the re-establishment of an effective groundwater monitoring well system that would be able to more accurately characterize the contamination plume. Soils excavated or otherwise disturbed during the project's construction phase would be tested in accordance with established Fort Belvoir policies and procedures. If concentrations of contaminants in soils are determined to exceed applicable regulatory thresholds for re-use on the site, any affected soils would be removed from the site and disposed of at a permitted facility off FBNA in accordance with Virginia Solid Waste Disposal Regulations as well as all other federal, state and local laws and regulations.

Munitions

Under the Proposed Action, less-than-significant effects would occur to munitions. As previously described, land use controls require all future ground disturbances and construction activities to complete munitions clearance. Prior to construction of the Proposed Action, munitions clearance would be conducted and coordinated with Fort Belvoir DPW and the VADEQ. The Proposed Action would have a beneficial, permanent effect in alleviating safety concerns related to possible munitions remaining on the surface or buried near the surface by screening the project area prior to construction. In addition, standard practice involves training of on-site personnel in the identification of potential munitions in order to prevent injury from unintentional detonations due to incorrect handling of discarded ordnance materials.

3.4.3.3 Impacts of the No Action Alternative

Hazardous Materials and Waste

The No Action alternative would have no effect on hazardous materials or hazardous wastes on FBNA. Long-term monitoring of the benzene plume suggests it is relatively stable and is expected to naturally attenuate over time even as current levels remain above acceptable limits. Land use controls prohibit extraction of groundwater for potable use and development of the site into another use unless determined to be compatible with applicable land use control policies and the Fort Belvoir ADP.

Munitions

The No Action alternative would have no effect on munitions concerns on FBNA.

3.5 UTILITIES

Utility representatives have been engaged in the scoping process and indicated the existing systems on FBNA are in good shape and have capacity to support the new construction. Wherever possible,

new utilities associated with the construction will tie into existing supporting infrastructure (i.e. lift stations, transformers, etc.). Utility additions and modifications will take into consideration current and surge demands and will have the capability to support future expansion requirements (HDR, 2020).

3.5.1 Electric

Electrical power is provided to FBNA by Dominion Virginia Power (DVP) using a 34.5-kilovolt (kV) distribution infrastructure, including a substation on the south portion of FBNA and a network of overhead and buried cables. The system is in good condition and has ample capacity for additional loading that would result from the new construction. DVP entered into a 50-year Utilities Privatization (UP) services contract with Fort Belvoir in 2007, under which DVP is responsible for operation and maintenance of the electrical distribution center as well as upgrades. As of 2016, more than 112 miles of overhead and underground electric line, three switching stations, and one substation are present on Fort Belvoir. DVP also owns and operates medium-sized emergency diesel generators to provide back-up power for critical-functions throughout the installation. There are no generating stations on FBNA that would be capable of powering the entire post. Backup generators, to include 48 hours of dedicated fuel supply, are necessary for the facility (HDR, 2020).

3.5.2 Potable Water and Wastewater

Potable water at FBNA is purchased from Fairfax County Water. No treatment facilities or groundwater wells supply potable water on post. The majority of the water distribution system at FBNA is owned by American Water under a 50-year utilities privatization (UP) contract to provide wastewater and wastewater services.

The water distribution system was designed and has the capacity to support full build-out of the FBNA campus. Currently, only 1.0 million gallons per day (MGD) is used out of a capacity of 3.0 MGD. A 1.5-million-gallon water storage tank that serves FBNA is located north of Barta Road, north of the project site.

Wastewater for the entire post is collected by a 14-inch line that runs to the Fairfax County Sewer stub-out at the south end of campus.

3.5.3 Natural Gas

Washington Gas operates the natural gas distribution system serving FBNA since a privatization contract in 1998. There are no natural gas production storage facilities on post. As of 2016, the natural gas distribution system has a network of approximately 120 miles of pipes. The existing gas distribution at FBNA is a high-pressure gas system with an 8-inch pipe that enters from the south side of the installation and runs west along Heller Road where it connects to the NGA facility's utility plants line. Fort Belvoir can receive approximately 160 million cubic feet per day of natural gas through two delivery points.

3.5.4 Environmental Consequences

3.5.5.1 Thresholds of Significance

Thresholds of significance for utilities dictate that a significant adverse effect would be to overload the capacity of existing utilities to the extent that current levels of service are compromised, resulting outages or shutdown of water or wastewater service.

3.5.5.2 Impacts of Proposed Action

Electric

Under the Proposed Action, less-than-significant, long-term effects would be expected. The electrical distribution system is new and in good condition with significant capacity for additional loading (HDR, 2020).

Two new utility feeders and two service transformers with a double-ended, main service entrance switchboard, in the Main-Tie-Main configuration would be provided for the new HQ DIA Annex. Pad-mounted, oil-cooled outdoor substation transformers would be utilized. The transformers would be configured for N+1 redundancy and would be sized based on the required load plus 25 percent spare capacity. The configuration of the utility feeder would be primary selective, utilizing automatic transfer circuit breakers or manual transfer switches. An automatic tie breaker in the Main-Tie-Main switchboard would be used.

Backup generators to support N+1, including 48 hours of dedicated fuel supply, would be required for the HQ DIA Annex facility. Cathodic protection systems and bonded protective coatings should be provided on buried or submerged utility piping where the electrolyte (soil or water) resistivity is less than 30,000 ohms per centimeter (cm) at the installation depth at any point along the piping installation, in accordance with UFC 3-570-01, Cathodic Protection.

Wastewater

Less-than-significant, long-term effects to wastewater are expected under the Proposed Action. The current usage of water distribution center is only 1/3 of the maximum usage available on post. The water distribution system at FBNA was designed to accommodate future development and is considered to be in good working condition. Connections to the primary distribution network are planned at the Fairfax County Sewer stub-out. Minimal industrial water is expected because the most cost-effective way to accomplish a dual-path, chilled-water system is to provide a looped piping system (HDR, 2020).

The wastewater system was also designed and built in anticipation of full build-out of the FBNA campus and therefore has the capacity to accommodate the wastewater generated by construction and operation of the Proposed Action. A new high-density polyethylene (HDPE) line will be installed along GEOINT Drive to connect the project site to the southern stub-out (HDR, 2020). Because the Proposed Action site is at a higher elevation than the sewer connection point, a

gravity-flow system can be used. Low-flow toilets, sinks and showers will be installed wherever possible to minimize impacts on water. Potable water and fire suppression will be supplied by at least an 8-inch service pipe and a redundant 6-inch pipe. A fire hydrant loop around the facility will be provided.

Natural Gas

Under the Proposed Action, less-than-significant, long-term impacts would occur to natural gas distribution. No system problems or capability issues would occur should the Proposed Action move forward (HDR, 2020). Connection to the HQ DIA Annex facility will start at the main lines off Heller Road and will run east along Heller Road until the closest connection can be made. Proposed construction would increase the natural gas demands of the current system; however, it was built with expansion in mind and is more than adequate to provide increased gas demands.

3.5.3 Impact of No Action Alternative

Under the No Action Alternative, no changes would be expected to any utilities. All operations at FBNA would remain the same, with no fluctuations in utility demands.

3.6 NOISE

Noise is generally defined as unwanted sound. It can be any sound that is undesirable because it interferes with communications or other human activities, is intense enough to affect hearing, or is otherwise annoying. Noise may be intermittent or continuous, steady, or impulsive. Human response to noise varies, depending on the type of the noise, distance from the noise source, sensitivity, and time of day.

The *Noise Control Act* of 1972 (PL 92-574) directs federal agencies to comply with applicable federal, state, interstate, and local noise control regulations. The applicable local noise control regulation is the Fairfax County Noise Ordinance (29-15-108.1), which states "no person shall permit, operate, or cause any source of sound or sound generation to create a sound which exceeds the limits set forth in the following table titled 'Maximum Sound Levels' when measured at the property boundary of the sound source or at any point within any other property affected by the sound". As shown in Table 3-2, the maximum sound levels from continuous sounds sources (such as a jackhammer) in residential areas should not exceed 60 dBA during the day and 55 dBA at night. An impulse sound is generally characterized by a sound event that lasts for no more than one second, such as sounds from weapons, pile drivers, or blasting.

Land use guidelines identified by the Federal Interagency Committee on Urban Noise are used to determine compatible levels of noise exposure for land use planning and control. Chapter 14 of AR 200-1 implements federal regulations associated with environmental noise from Army activities. There are three Noise Zones (I, II, and III), which correlate to increasing noise levels (see Table 3-3). These zones are established based on average day-night levels (DNL) of noise

over 104 days. Additionally, there is the Land Use Planning Zone (LUPZ), which is the portion of Noise Zone I exposed to noise levels within 5 decibels (dB) of Noise Zone II levels.

		MAXIMUM SOUND LEVELS		
Use and Zoning District Classification	Time of Day	Continuous Sound (dBA)	Impulse Sound (dBA)	
Residential Areas in Residential Districts	7 a.m. to 10 p.m.	60	100	
Residential Areas in Residential Districts	10 p.m. to 7 a.m.	55	80	

The decibel is the accepted unit of measurement for noise level and uses a logarithmic scale. For low-frequency events such as artillery fire, C-weighted decibels may be used to calculate measurements like DNLs. The final noise metric relevant to this discussion is peak sound level (dBP), which is the maximum instantaneous sound level of an event. The dBP is neither weighted nor time integrated and is used to further define noise zones.

Table 3-3: Noise I	Limits Definitions (A	Army Regulation 200-1)

Noise Zone	Population	Transportation	Impulsive Noise	Small Arms
	Highly Annoyed	Noise	CDNL (dBC)	Noise
	(%)	ADNL (dBA)		(dBP)
Ι	Less than 15	Less than 65	Less than 62dBC	Less than 87
II	15-39	65-75	62-70	87-104
III	More than 39	More than 75	More than 70	More than 104

* dBA = decibels, A-weighted ,dBC = decibels, C-weighted ,dBP = decibels, unweighted

Table 3-4: Sensitive Land Use

Noise Zone	Noise Sensitive Land Use	Demolition and Large Caliber Activity dB CDNL
LUPZ	Generally Compatible	57-62 db
Zone I	Generally Compatible	<62 db
Zone II	Generally Compatible	62-70 db
Zone III	Not Compatible	>70 db

The nearest potential noise-sensitive receptors to the Proposed Action are the North Belvoir CDC, located adjacent and to the east of the project site, and the existing NGA offices, located adjacent to the west. The somewhat isolated enclave of the project site, NGA and CDC is surrounded by Barta Road to the west and north, Heller Road to the east and south, and Fairfax County Parkway to the south. The major thoroughfare of Interstate 95 (I-95) is located approximately 0.3-mile to the east of the project site. Currently, the major noise source in the project vicinity is generated

from vehicular traffic on Fairfax County Parkway and I-95. Activities at the Davidson Army Air Field (DAAF), including airplane and helicopter takeoffs and landings, are also apparent at the site, located approximately 2.25 miles to the northwest.

3.6.1 Environmental Consequences

3.6.1.2 Threshold of Significance

Noise impacts would be significant if the Proposed Action created appreciable long-term noise increases in areas of incompatible land use, would substantially increase noise resulting from traffic, or result in substantial disruptions to nearby sensitive receptors. Additionally, continuous construction noises above 60 dBA may be considered a nuisance if audible at residential properties during daytime hours (07:00 to 22:00) per the Fairfax County noise ordinance. Furthermore, noise levels exceeding National Institute for Occupational Safety and Health (NIOSH) or Occupational Safety and Health Administration (OSHA) guidance can be harmful to workers.

3.6.1.2 Impacts of Proposed Action

Less-than-significant, long-term adverse effects to noise would be expected under the Proposed Action. The primary use of the proposed facilities would be administrative office space.

Construction. The Proposed Action would require construction activities on FBNA. Individual pieces of construction equipment typically generate noise levels of 80 to 90 dBA at a distance of 50 feet. With multiple items of equipment operating concurrently, noise levels can be relatively high during daytime periods at locations within several hundred feet of active construction sites. The zone of relatively high construction noise typically extends to distances of 400 to 800 feet from the site of major equipment operations. Locations more than 1,000 feet from construction sites seldom experience noteworthy levels of construction noise. Given the temporary nature of proposed construction activities and the limited amount of noise that construction equipment would generate, this effect would be considered minor.

Existing sounds generated from aircraft traveling to and from the DAAF, and from vehicle traffic on Fairfax County Parkway and I-95 dominate the noise profile in the area, making construction-related sounds at the proposed project site less likely to be perceived or considered a nuisance to nearby receptors.

During the construction period, sources of noise would include equipment used to construct the Proposed Action. Noise produced by construction equipment varies depending on the type of equipment used and its duration. Equipment associated with constructing the Proposed Action would include cement and mortar mixers, cranes, excavators, forklifts, graders, pavers, rollers, and skid steer loaders.

To minimize the potential adverse impact from these noises, construction vehicles would be equipped with noise-dampening equipment including mufflers which would be operated according to the manufacturers' instructions. Construction vehicles and equipment would be turned off when not in use for more than five minutes. Additionally, construction would take place during daylight hours on weekdays, unless there is a specific action that would require working outside of this normal timeframe, such as mobilizing oversized materials or equipment to the site.

Construction noises would be further dampened by maintaining vegetated borders which act as natural sound barriers. Therefore, construction noises would be minimally evident to nearby noise-sensitive receptors.

OSHA regulations require that employers make hearing protectors available to those employees who are exposed to work conditions at or above 85 dBA (OSHA, 2002). Thus, potential impacts from construction equipment noise on workers would be minimized by following OSHA regulations and the USACE *Safety and Health Requirements Manual EM 385-1-1* (USACE, 2014). Therefore, construction noise associated with the Proposed Action would have short-term, direct, negligible adverse impacts to workers and to nearby receptors.

Noise levels on the FBNA could increase as a result of additional commuters, primarily during weekday mornings (06:00-09:00) and afternoons (15:00-18:00). However, noise levels for noise-sensitive receptors (NSR) adjacent to the main traffic routes near the FBNA, Main Post, and the surrounding area would not exceed the noise-abatement criterion (67 dBA) for residential land uses and Zone III noise levels would not occur (U.S. Army, 2007).

3.3.1.3 Impacts of No Action Alternative

Under the No Actional Alternative, no changes would occur to noise. All operations at FBNA would remain the same, with no fluctuations in noise production.

3.7 AIR SPACE

The DAAF occupies about 400 developed acres of land west of Fairfax County Parkway. The mission of the Davison Army Airfield is to transport passengers and freight for the Army and DoD to, from, and within the NCR.

Building height restrictions are governed by guidelines and regulations relating to the identification and construction of obstructions within airspace are established in the Federal Aviation Regulations (FAR Part 77, Objects Affecting Navigable Airspace). Building restrictions within the conical surface begin at the 150 feet level above the runway at the boundary with the inner horizontal surface and extend outward at a slope of 20:1 (horizontal: vertical) for a distance of 7,000 feet to an elevation of 500 feet above the airfield. The majority of the remaining portion of the Main Post (with the exception of the extreme northeast and southeast sections) and FBNA fall within the 150- to 500-foot building height restriction within the conical surface (U.S. Army, 2007).

3.7.1 Environmental Consequences

3.7.1.1 Threshold of Significance

The Proposed Action and No Action Alternative were evaluated against the following significance criteria to determine if they would result in a significant impact on the airspace environment:

- Airspace would be obstructed by building heights
- Aircraft operations would be substantially altered to accommodate new construction

3.7.1.2 Impacts of Proposed Action Alternative

Under the Proposed Action, less-than-significant impacts to airspace would occur. The Proposed Action would include a 6-story headquarter buildings as its tallest structure, remaining within the vertical limits of the applicable airspace restrictions and consistent with the height of the adjacent NGA complex.

3.7.1.3 Impacts of No Action Alternative

Under the No Actional Alternative, no changes would be expected to airspace. All operations at FBNA would remain the same, with the same aircraft operation and airspace available.

3.8 AIR QUALITY

Air pollution occurs when harmful substances, including solid particles and gases, are introduced into the earth's atmosphere. It can cause harm to the natural environment, including humans, animals, and plants. Air quality refers to the pollution-free ambient air. The lower the air quality the more polluted the air, and the higher the quality the more pollutant-free the air. In the following sections, air quality in the vicinity of the Proposed Action site is described, applicable laws and regulations are explained, and potential impacts are assessed.

3.8.1 NAAQS

The United States Environmental Protection Agency (USEPA), under the requirements of the 1970 *Clean Air Act* (CAA) as amended in 1977 and 1990, has established National Ambient Air Quality Standards (NAAQS) for the following six contaminants, referred to as criteria pollutants (40 CFR 50):

- Carbon monoxide (CO)
- Lead
- Nitrogen dioxides (NO_x)
- Ozone (O₃)
- Sulfur dioxide (SO₂)
- Particulate matter (PM), divided into two size classes:
 - o Aerodynamic size less than or equal to 10 micrometers (PM_{10})

• Aerodynamic size less than or equal to 2.5 micrometers ($PM_{2.5}$)

The NAAQS include primary and secondary standards. The primary standards were established at levels sufficient to protect public health with an adequate margin of safety. The secondary standards were established to protect the public welfare from the adverse effects associated with pollutants in the ambient air. Table 3-5 shows primary and secondary air quality standards.

NAAQS Pollutant	Primary/ Secondary	Averaging Time	Level	Form
Carbon	Primary	8-hour	9 ppm	Not to be exceeded more than once per
Monoxide	I IIIIai y	1-hour	35 ppm	year
Nitrogen	Primary	1-hour	100 ppb	98th percentile, averaged over 3 years
Dioxide	Primary and secondary	Annual	53 ppb	Annual Mean
Ozone	Primary and secondary	8-hour	0.070 ppm	Annual fourth-highest daily maximum 8-hr concentration, averaged over 3 years
Particular	Primary	Annual	12 µg/m3	Annual mean, averaged over 3 years
Matter	Secondary	Annual	15 µg/m3	Annual mean, averaged over 3 years
(PM _{2.5})	Primary and secondary	24-hour	35 µg/m3	98th percentile, averaged over 3 years
Particular Matter (PM ₁₀)	Primary and secondary	24-hour	150 µg/m3	Not to be exceeded more than once per year on average over 3 years
Lead	Primary and secondary	Rolling 3- month average	0.15 μg/m3	Not to be exceeded
Sulfur	Primary	1-hour	75 ppb	99th percentile of 1-hour daily maximum concentrations, averaged over 3 years
Dioxide	Secondary	3-hour	0.5 ppm	Not to be exceeded more than once per year

Table 3-5: National Ambient Air Quality Standards

The CAA, as amended in 1990, mandates that state agencies adopt State Implementation Plans (SIP) that target the elimination or reduction of the severity and number of violations of the NAAQS. SIPs set forth policies to expeditiously achieve and maintain attainment of the NAAQS. While each state has the authority to adopt standards stricter than those established under the federal program, the Commonwealth of Virginia accepts the federal standards.

The Commonwealth of Virginia, in coordination with Metropolitan Washington Council of Governments (MWCOG), developed a SIP that outlined actions to achieve the NAAQS. The current EPA-approved regional air quality plan is the *Plan to Improve Air Quality in the Metropolitan Washington, DC-Maryland (MD)-VA Region: State Implementation Plan (SIP) for*

8-Hour Ozone Standard (MWCOG, 2007). Within this plan, VADEQ compiles a regional emissions inventory and sets regional emissions budgets.

Federal regulations designate Air Quality Control Regions (AQCR) that have concentrations of one or more of the criteria pollutants that exceed the NAAQS as *nonattainment areas*, while AQCRs with levels below the NAAQS are designated as *attainment areas*. Further, *maintenance areas* are AQCRs that have previously been designated nonattainment and have been redesignated to attainment for a probationary period through implementation of maintenance plans. According to the severity of the pollution problem, O_3 and PM_{10} nonattainment areas can be categorized as marginal, moderate, serious, severe, or extreme. Where insufficient data exist to determine an area's attainment status, it is designated unclassifiable or in attainment.

Fairfax County (which encompasses Fort Belvoir) is within the National Capital Interstate AQCR (AQCR 047, or "DC-MD-VA AQCR") (40 CFR 81.12). AQCR 047 is in the ozone transport region that includes 12 states and Washington, DC.

The EPA (as of February 28, 2021) has classified Fairfax County as being in marginal nonattainment for 8-hour ozone; Fairfax County is in attainment with the remaining NAAQS (USEPA, 2021).

3.8.2 Clean Air Act Conformity

The 1990 amendments to the CAA require federal agencies to ensure that their actions conform to the SIP in a nonattainment area. Under Section 176(c) of CAA, a project is in "conformity" if it corresponds to a SIP's purpose of eliminating or reducing the severity and number of violations of the NAAQS and achieving their expeditious attainment.

Conformity further requires that such activities would not:

- cause or contribute to any new violations of any standards in any area;
- increase the frequency or severity of any existing violation of any standards in any area; or
- delay timely attainment of any standard or any required interim emission reductions or other milestones in any area.

The EPA published final rules on general conformity (40 CFR Parts 51 and 93) in the Federal Register on November 30, 1993. The General Conformity Rules (GCR) apply to federal actions in nonattainment or maintenance areas for any of the criteria pollutants. The rules specify *de minimis* emission levels by pollutant to determine the applicability of conformity requirements for a project. The corresponding *de minimis* levels for the ozone precursors for marginal O_3 nonattainment areas are 100 tons per year for NO_x and 50 tons per year for volatile organic compounds (VOCs). A federal action is exempt from the GCR requirements if the action's total net emissions are below the *de minimis* threshold or are otherwise exempt per 40 CFR 51.153. There are two main components to the overall process: an applicability analysis to determine whether a conformity determination is required and, if it is, a conformity determination to demonstrate that the action conforms to the SIP.

3.8.3 Hazardous Air Pollutants

In addition to criteria pollutant standards, EPA also regulates hazardous air pollutant (HAP) emissions for each state. HAPs differ from criteria pollutants for they are known or suspected to cause cancer and other diseases or have adverse environmental impacts. The National Emission Standards for Hazardous Air Pollutants regulate 188 HAPs based on available control technologies. Sources of HAP emission at Fort Belvoir include stationary, mobile, and fugitive emissions sources. Stationary sources include boilers, incinerators, fuel storage tanks, fuel-dispensing facilities, vehicle maintenance shops, laboratories, degreasing units, and similar testing units. Mobile sources of emissions include private and government-owned vehicles. Fugitive sources include dust generated from demolition activities and roadway traffic.

3.8.4 Greenhouse Gas Emissions and Climate Change

Greenhouse gases (GHGs) are compounds that contribute to the greenhouse effect. The greenhouse effect is a natural phenomenon where gases trap heat within the surface-troposphere (lowest portion of Earth's atmosphere) system, causing heating at the Earth's surface. The primary long-lived GHGs directly emitted by human activities are carbon dioxide (CO₂), methane (CH₄), nitrous oxide (N₂O), hydrofluorocarbons (HFCs), perfluorocarbons (PFCs), and sulfur hexafluoride (SF_6) . The heating effect from these gases is considered the probable cause of the global warming observed over the last 50 years (NASA, 2019). Global warming and climate change can affect many aspects of the environment. In the past, the EPA has recognized potential risks to public health or welfare and signed an endangerment finding regarding GHGs under Section 202(a) of the CAA (74 Federal Register 66496, December 15, 2009), which found that the current and projected concentrations of the six key well-mixed GHGs in the atmosphere (CO₂, CH₄, N₂O, HFCs, PFCs, and SF₆) threaten the public health and welfare of current and future generations. To estimate global warming potential (GWP), all GHGs are expressed relative to a reference gas, CO₂, which is assigned a GWP equal to one (1). All six GHGs are multiplied by their GWP and the results are added to calculate the total equivalent emissions of CO₂ (CO₂e). However, the dominant GHG gas emitted is CO₂, accounting for 81% of all GHG emissions as of 2018, the most recent year for which data are available (USEPA, 2020). Current GHG emission sources at Fort Belvoir include combustion engines, boilers, chillers, and water heaters.

One of the key ways the DoD achieves reduction in GHG emissions in building construction and operation is through the Leadership in Energy and Environmental Design (LEED) certification program, an internationally recognized green building certification system providing third-party verification that a building or community was designed and built using measures to reduce energy and water use, GHG emissions and the amount of construction waste sent to landfills. The Energy Independence and Security Act of 2007 requires federal agencies to use a green building certification system for new construction and major renovations of buildings. Pursuant to DoD policy, the Proposed Action will be designed to achieve an LEED rating of Silver.

It is noted that EO 13990, signed January 20, 2021, reinstated the final guidance issued on August 5, 2016 by the CEQ that required federal agencies to consider GHG emissions and the effects of climate change in NEPA reviews. DoD has committed to reduce GHG emissions from non-combat

activities 42% by 2025 (DoD, 2016). Accordingly, estimated CO₂e emissions associated with the Proposed Action are provided in this EA for informative purposes.

Current GHG emission sources at Fort Belvoir include combustion engines, boilers, chillers, and water heaters. The total CO2e for Fort Belvoir is inclusive of Main Post and FBNA. However, FBNA sources only account for 0.1% (natural gas) of the total 27,366.02 metric tons CO2e for calendar year (CY) 2020. The emission total is the amount reported annually under the requirements of 40 CFR Part 98 and does not include GHG emissions from mobile sources or emergency generator use. Fort Belvoir is required to report to EPA through the electronic GHG tool (e-GRRT) as the installation has exceeded 25,000 metric tons per year for CO_2e for the last five years.

3.8.5 Emissions Reporting

Title V of the CAA Amendments of 1990 requires states and local agencies to permit major stationary sources. As a major stationary source for emissions, Fort Belvoir (Main Post) operates under a Title V Permit (Registration Number 70550, issued on March 21, 2003). Fort Belvoir also operates under a minor New Source Review (mNSR) permit for Main Post (same Registration Number 70550).

The Title V and mNSR permits for Main Post do not apply to FBNA emission sources, as this area is non-contiguous from Main Post and considered a separate source. Stationary emission sources at FBNA include large boilers, generators, heaters, above ground storage tanks and emergency generators. FBNA emission sources are operated under a separate synthetic mNSR air permit (Registration Number 73630). As a synthetic minor source, the FBNA annual update report does not include the requirement for an emission statement. The FBNA annual update report provides specific total throughput (million cubic feet burned and/or gallons burned) for the permitted equipment. However, as a requirement of the permit, Fort Belvoir Air Program maintains a rolling 12-month total for the criteria pollutant emissions from the FBNA sources, as found in Table 3-6.

Table 5-0. Emissions from Stationary Sources (tons/year) for C 1 2020					
SO ₂	СО	PM10	PM2.5	NO ₂	VOCs
0.15	1.65	0.25	0.25	6.31	0.35

Source: Belvoir, Air Program

3.8.6 Sensitive Receptors

CEQ NEPA regulations require evaluation of the degree to which the Proposed Action affects public health (40 CFR 1508.27). Children, elderly people, and people with illnesses are especially sensitive to the effects of air pollutants; therefore, hospitals, schools, convalescent facilities, religious facilities, and residential areas are considered to be sensitive receptors for air quality impacts, particularly when located within one mile from the emissions source. There are several Fort Belvoir-based medical facilities, schools, residential areas, and religious institutions on FBNA, with the North Belvoir CDC located within a one-mile radius of the project study area.

3.8.7 Environmental Consequences

3.8.7.1 Threshold of Significance

The threshold of significance for air quality impacts would be exceeded if the alternative would result in any of the following:

- Causing or contributing to new violations of NAAQS,
- Contributing to the worsening of existing violations of the NAAQS,
- Delaying the attainment of the NAAQS

Thus, an impact could be significant if emissions exceed "*de minimus*" standards as designated in federal or state air quality regulations during construction or operation.

3.8.7.2 Impacts of the Proposed Action

Air Quality General Conformity

Construction. The Army has considered net emissions generated from all direct and indirect sources of air emission that are reasonably foreseeable. *Direct emissions* are emissions that are caused or initiated by a federal action and occur at the same time and place as the action. *Indirect emissions* are defined as reasonably foreseeable emissions that are caused by the action but might occur later in time and/or be farther removed in distance from the action itself, and that the federal agency can practicably control.

Specifically, direct emissions would result from using construction equipment needed to build the HQs Annex, parking garage and appurtenant structures described for the Proposed Action in Section 2.1. Following completion of the construction phase, no additional construction equipment would be required to operate the Proposed Action.

As previously described, Fairfax County has been classified as a marginal non-attainment area for 8-hour ozone and is in attainment for all other criteria pollutants. Therefore, since construction associated with the Proposed Action would result in the emissions of precursors of this air pollutant, a review has been conducted to determine if the Proposed Action is subject to a general conformity determination.

The type of construction equipment and hours of operation to be used during the construction phase were estimated based on experience on similar projects. This information was then used to calculate the emissions associated with the construction phase of the Proposed Action. The total project construction emissions associated with the use of off-road construction equipment (e.g., bulldozers, backhoes), on-road construction equipment (e.g., haul trucks), workers' vehicles, and fugitive dust from surface disturbances are presented in Table 3-7.

As shown in Table 3-7, the total estimated emissions for construction of the Proposed Action would be below the GCR *de minimis* thresholds. Therefore, the Proposed Action does not require

a formal conformity determination. The U.S. Army has prepared a Record of Non-Applicability (RONA) for CAA conformity (refer to Appendix B of this EA).

Pollutant	2024-2025 Proposed Action Alternative Emissions (tpy) ³	2025-2026 Proposed Action Alternative Emissions (tpy)	Operational Emissions (generator) (tpy)	De minimis Level (tpy) 1	Major Source Threshold (tpy) ²
VOCs	3.51	3.51	0.22	50	
NO _x	34.11	34.11	8.05	100	
SO_2	2.64	2.64	0.004		100
CO	18.61	18.61	1.84		100
PM10	34.86	34.86	0.24		100
PM _{2.5}	34.77	34.77	0.24		100
CO ₂ e	4,216.89	4,216.89	388.89		25,000 ⁽⁴⁾

 Table 3-7: Air Quality Calculations for the Proposed Action

- 1. Deminimis thresholds are not applicable to pollutants for which the area is in attainment for the NAAQS. Deminimis levels for an 0_3 non-attainment area in the ozone transport region.
- 2. Major source threshold for criteria pollutants.
- 3. A two-year construction window is anticipated, from 2024-2026.
- 4. In 40 CFR Part 98, the EPA established a requirement of mandatory reporting of greenhouse gases (GHG) from large GHG emissions sources in the United States. The threshold for reporting is 25,000 metric tons or more of carbon dioxide (CO₂e) equivalent per year.

Operation. The Proposed Action would result in long-term, direct, negligible adverse impacts from the additional buildings and associated maintenance activities. Operational emissions would be generated from landscaping, boiler and emergency generator emissions. Landscaping emissions resulting from the operation of the Proposed Action would be negligible.

No potentially significant adverse effects on air quality were identified by analysis; therefore, no mitigation measures would be required. The following management measures and/or BMPs would be implemented to further reduce the anticipated less-than-significant, adverse effects:

- Truck beds would be covered while in transit to limit fugitive dust emissions.
- Water would be sprayed on any unpaved roads or stockpiles to limit fugitive dust emissions.
- Ultra-low sulfur diesel would be used as a fuel source where appropriate to minimize oxides of sulfur emissions.
- Clean diesel would be used in construction equipment and vehicles through the implementation of add-on control technologies such as diesel particulate filters and diesel oxidation catalysts, repowers, and/or newer and cleaner equipment. When feasible, electric-powered equipment would be used in lieu of diesel-powered equipment.

- Control measures for heavy construction equipment and vehicles, such as minimizing operating and idling time, would be implemented to limit criteria pollutant emissions.
- Air quality permits would be obtained for the Proposed Action Alternative, as necessary, in compliance with federal, state, and local standards.
- Building design would achieve the LEED-Silver certification, ensuring reductions in energy and water use and greenhouse gas emissions over the life cycle of the building.

3.8.7.3 Impacts of the No Action Alternative

Under the No Action alternative, no short- or long-term changes in emissions quantities or types would occur. Therefore, under the No Action alternative, current baseline air emissions would continue unchanged for the foreseeable future.

3.9 TRAFFIC

This section describes the existing road network serving the Proposed Action at FBNA. A Traffic Impact Study (TIS) was conducted to evaluate existing conditions and the potential impacts of the Proposed Action to traffic patterns in the vicinity (see Appendix D). Eleven key intersections were identified in the traffic study area. Turning Movement Counts (TMCs) and roadway volume counts were conducted at the eleven locations shown in Figure 3.8.



Figure 3-8: Traffic Count Locations

Count	Intersection	Count Date
ID		
1	Barta Road with GEOINT Drive	2021-03-23
2	Barta Road with Heller Road	2021-03-23
3	Barta Road with Backlick Road	2021-03-23
4	Barta Road / Fairfax County Parkway (VA 286) NB Ramps	2021-03-24
5	Barta Road / Fairfax County Parkway (VA 286) SB Ramps	2021-03-24
6	Heller Road with I-95 NB/I-95 SB Express Lane	2021-03-23
7	Heller Road with I-95 SB	2021-03-23
8a	Heller Road with NGA South Gate (inbound)	2021-03-23
8b	Heller Road with NGA South Gate (outbound)	2021-03-24
9	Barta Road at NGA West Gate Entry	2021-03-24
10	Barta Road at NGA West Gate Exit	2021-03-24
11	GEOINT Drive Visitor Parking Lot Access Lane	2021-03-24

Table 3-8: Traffic Volume Count Locations

Existing Traffic Volumes

Traffic counts were conducted at the previously referenced 11 intersections in the study area. Lower volume intersections were counted manually, while automated recording systems were used at the higher volume intersections. The counts were conducted between March 22 and April 7, 2021.

The peak hour represents the four consecutive 15-minute periods with the highest total traffic volume for the intersection as a whole. In the study area, the PM peak hour volumes were higher than the AM peak hour volumes. A review of the traffic count data indicates that the weekday morning and afternoon peak hours are not consistent among the study intersections. The respective peak hour for each intersection is shown in Table 3-9.

Count ID	Location	Peak Hour		
Count ID Location		AM	PM	
Alternative	1 – FBNA			
1	Barta Road with GEOINT Drive	6:45–7:45	4:30-5:30	
2	Barta Road with Heller Road	7:15-8:15	3:45-4:45	
3	Barta Road with Backlick Road	7:00-8:00	4:00-5:00	
4-5	Barta Road with Fairfax County Parkway (VA 286) NB	6:45-7:45	3:45-4:45	
	Ramps (WB Barta Road)			
6	Heller Road with I-95 NB/I-95 SB Express Lane	12:00-1:00	5:45-6:45	
7	Heller Road with I-95 SB	7:45-8:45	3:00-4:00	
8	Heller Road with NGA South Gate (inbound)	7:30-8:30	8:45-9:45	

9	Barta Road at NGA West Gate Entry	9:30-10:30	-
10	Barta Road at NGA West Gate Exit	-	5:45-6:45
11	GEOINT Drive Visitor Parking Lot Access Lane	7:15-8:15	2:45-3:45

Based on the results of the traffic count data, the AM peak hour was modeled as 7:45 AM to 8:45 AM and the PM peak hour was modeled as 4:00 PM and 5:00 PM. Each of these peak hours were chosen as they represent the highest peak hour volume for the FBNA study area in their respective time periods.

The existing traffic operating conditions in the study area were analyzed using Trafficware's Synchro 11 traffic analysis software and the methodology in the Highway Capacity Manual 6th Edition. The existing peak hour traffic volume (AM peak and PM peak hours) and the existing lane-use configuration were used in performing the existing (2021) operational analysis.

To account for the COVID-19 pandemic and its effect on traffic patterns on and in the vicinity of the study area, the existing (2021) peak hour volumes were adjusted upwards, assuming 60% of "normal" personnel were counted during March/April 2021 traffic counts. Gate counts were provided from inbound Tulley, Pence, Kingman, and Farrar gates for a similar Monday through Friday time period in January 2020 and January 2021 that validate this volume adjustment.

Level of Service Standards

Level of service is a qualitative measure describing operational traffic conditions and the perception of these conditions by drivers or passengers. These conditions include factors such as speed, delay, travel time, freedom to maneuver, traffic interruptions, comfort, convenience, and safety. Levels of service are given letter designations from A to F, with LOS A representing the best operating conditions (free flow, little delay) and LOS F representing the worst (congestion, long delays). Generally, LOS A and B are considered high level of service, LOS C and D are considered moderate, and LOS E and F are considered low.

In general, the standards are LOS D in urban areas and LOS C in rural areas. The results of the operational analysis at FBNA using Synchro are provided in Table 3-10 below.

Table 3-10: Existing (adjusted) Interse	ection Operational Analysis
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	Cionalina d	AM	PM	AM	PM
Intersection	Signalized (Y/N)	Delay (s/veh)		LOS	
Barta Road / FBNA Facilities Access	Y	1.7	1.1	Α	А
West Gate Entrance	Ν	-	-	Α	Α
Barta Road / Parking Garage Exit	Y	0.0	10.4	Α	В
Barta Road / Main Guest Access	Ν	-	-	Α	А
Barta Road / GEOINT Drive	Y	5.5	13.3	Α	В

Barta Road / Heller Road	Y	9.8	0.6	А	Α
Barta Road / Backlick Road	Y	7.9	20.1	Α	С
Heller Road / HOV Entrance Ramp	Ν	-	-	Α	Α
95 Exit Ramp / Heller Road	Ν	-	-	Α	Α
South Gate Entrance	N	-	-	Α	Α

As shown in the table, all intersections are operating at LOS C or better.

<u>Transit</u>

There are three bus transit routes that pass near Fort Belvoir and FBNA:

- Route 171
- Route 335
- REX (Richmond Highway Express)

Routes 171 and 335 are operated by the Fairfax Connector, and the REX is operated by Washington Metropolitan Area Transit Authority.

Non-motorized Facilities

There are sidewalks and pedestrian crossings in the study area, however few pedestrian movements were noticed during the traffic counts. Surrounding streets do not have marked bicycle lanes, and no bicycle movements were observed during the traffic counts.

3.9.2 Environmental Consequences

3.9.2.1 Thresholds of Significance

Roadway traffic resulting from operations of the Proposed Action could result in changes to the level of service (LOS) provided by existing road systems. Key issues of concerns regarding potential traffic impacts of the Proposed Action include:

- Maintaining a LOS on affected roadways that meets an acceptable standard;
- Minimizing the effect of 650 additional employees at the Access Control Points (ACPs) serving FBNA.

3.9.2.2 Impacts of Proposed Action

The Proposed Action is estimated to generate 650 additional staff positions. The analysis assumes that each additional staff member generates one (1) additional AM and PM peak hour trip for both 650 additional staff and 1000 additional staff scenarios. A sensitivity analysis that assumes 1000 additional employees was conducted to determine operational levels for possible future staff. The distribution between site access points was determined utilizing the March 2021 count data.

Peak Period Vehicular Traffic Impacts

Table 3-11 presents the general traffic operations summary for all scenarios analyzed for the Proposed Action.

Int. ID	Intersection	Signalized (Y/N)	650 Added Personnel				1000 Added Personnel			
			AM	PM	AM	PM	AM	PM	AM	PM
			Delay (s/veh)		LOS		Delay (s/veh)		LOS	
В	Barta Road / FBNA Facilities Access	Y	2.0	1.3	А	А	2.2	1.5	А	А
С	West Gate Entrance	Ν	-	-	А	А	-	-	А	А
D	Barta Road / Parking Garage Exit	Y	0.1	10.0	А	А	0.1	10.0	А	А
Е	Barta Road / Main Guest Access	Ν	-	-	А	А	-	-	А	А
F	Barta Road / GEOINT Drive	Y	8.7	21.5	А	С	11.1	67.2	В	Е
G	Barta Road / Heller Road	Y	11.5	3.1	В	А	12.2	2.9	В	А
Н	Barta Road / Backlick Road	Y	8.0	21.5	А	С	20.4	20.9	С	С
Ι	Heller Road / HOV Entrance Ramp	Ν	-	-	А	А	-	-	А	А
J	95 Exit Ramp / Heller Road	Ν	-	-	А	А	-	-	А	А
K	South Gate Entrance	N	-	-	А	А	-	-	А	А

 Table 3-11: Build Condition (2021 adjusted) Intersection Operational Analysis

Under the Proposed Action, all intersections (AM and PM) would operate at LOS B or better with the exception of the intersections of:

- Barta Road / GEOINT Drive (LOS C during the PM peak hour) Exiting traffic from GEOINT Drive creates queues while waiting to turn on to Barta Road.
- Barta Road / Backlick Road (LOS C during the AM peak hour) Backlick Road NB left turns queue and saturate the lanes waiting for SB Backlick thru movements to clear.

The TIS concludes that FBNA can accommodate the anticipated additional traffic generated by the Proposed Action. There also appears to be excess capacity if additional site traffic generators are proposed. Gate SMART Evaluator -Quick Calculator was used to determine potential staffing and lane needs for ACPs. Based on 650 added vehicles to the AM peak hour at each gate, the analysis determined that all gates have excess number of receiving lanes and no additional manpower or lanes would be required to handle the additional volume.

3.9.2.3 Impacts of the No Action Alternative

Currently, the primary users of FBNA are government employees of NGA and their visitors. No growth in background traffic volumes in the study area would result from the No Action Alternative.

3.10 CULTURAL AND HISTORIC RESOURCES

3.10.1 Site History

The Army acquired FBNA (formerly EPG) in the early 1940s to support the Research, Development and Engineering Center for the testing of a wide range of engineering equipment and supplies, including methods and equipment for the deployment, detection, and neutralization of landmines. The Army used FBNA for these purposes from the 1940s through the 1970s (U.S. Army, 2007). The highest level of activity at EPG occurred during the 1940s to the mid-1950s. Commercial and residential encroachment in the vicinity of FBNA in the 1960s and 1970s contributed to the reduction of testing activities at this location.

The historical testing and training activities on the eastern portion of FBNA included the following (U.S. Army, 2007):

- Construction, material handling, maintenance, railway, power generation, air compression, and bridging equipment
- Fuels and fuel handling and storage equipment, mobile water purification equipment, and waste and sewage structures
- Climatic effects on paints, tactical sensors, and anti-mine systems and techniques.

Several federal laws and regulations—including the National Historic Preservation Act (NHPA) of 1966, as amended, , the Archaeological and Historic Preservation Act of 1974, the AIRFA of 1978, the Archaeological Resource Protection Act of 1979 (ARPA), and the NAGPRA of 1990— have been established to manage cultural resources. Cultural resources include "historic properties" as defined by the NHPA "cultural items" as defined by the Native American Graves Protection and Repatriation Act of 1979 (NAGPRA), "archaeological resources" as defined by the ARPA, "sacred sites" as defined by EO 13007 to which access is afforded under the American Indian Religious Freedom Act of 1987 (AIRFA), and collections and associated records as defined in 36 CFR 79.

Archaeological resources consist of locations where prehistoric or historic activity measurably altered the earth or produced deposits of physical remains. Architectural resources include standing buildings, districts, bridges, dams, and other structures of historic significance. Traditional cultural properties include locations of historic occupations and events, historic and contemporary sacred and ceremonial areas, prominent topographical areas that have cultural significance, traditional hunting and gathering areas, and other resources that Native Americans or other groups consider essential for the persistence of their traditional culture.

The NHPA outlines federal policy to protect historic properties and promote historic preservation in cooperation with other nations, tribal governments, states, and local governments. Sections 106 and 110 of the NHPA require federal agencies to identify, evaluate, inventory, and protect historic

properties (i.e. those listed or eligible for listing in the National Register of Historic Places [NRHP]) that are under their jurisdiction and control. Federal agencies must delineate the Area of Potential Effect (APE) within which impacts from a proposed action may occur, identify historic properties present within the APE, assess the potential effects of the undertaking on those historic properties and consider ways to avoid, minimize and mitigate any adverse effects. The APE is the geographic area in which an undertaking may directly or indirectly cause changes in the use or character of a historic property. An undertaking is any federal action with the potential to affect historic properties. Federal agencies are further required to initiate consultation with the State Historic Preservation Officer (SHPO) for actions that may impact historic properties. The Virginia Department of Historic Resources (VDHR) serves as the SHPO in Virginia.

Section 110 of the NHPA requires federal agencies to establish their own programs to locate, inventory, nominate, and protect historic properties owned or controlled by the agency that may qualify for inclusion in the National Register, The intent of Section 110 is to identify the historic properties that should be considered when federal agencies make planning decisions to ensure that these agencies provide good stewardship to the nation's significant cultural resources. In compliance with Section 110, a comprehensive archaeological survey was completed for the former EPG area in 1993, and no archaeological properties eligible for the National Register of Historic Properties (NRHP) were present (MAAR Associates, 1993). Only one archaeological resource, an isolated prehistoric artifact, has been discovered on FBNA, but evaluated as not eligible for the NRHP (New South Associates, 2007).

A comprehensive architectural survey of all extant properties on FBNA was completed in 2006 and none were eligible for the National Register, nor listed on any state or local resister. The findings of this report were reviewed and concurred by Virginia SHPO. Further, a review of the Fairfax County Inventory of Historic Sites, current Fairfax County Historic Overlay Districts, the Virginia Landmarks Register, and the National Register indicated that no listed resources or historic overlay districts are in close proximity to FBNA (U.S. Army, 2007).

The APE for the Proposed Action is defined as the project area outlined in Figure 3-8. The APE also considered a 1-mile buffer surrounding the study area to account for any potential effects on the viewshed of historic districts in the vicinity. Based on the information provided in the above paragraphs, Fort Belvoir has concluded that no historic properties exist within the APE or in close proximity.

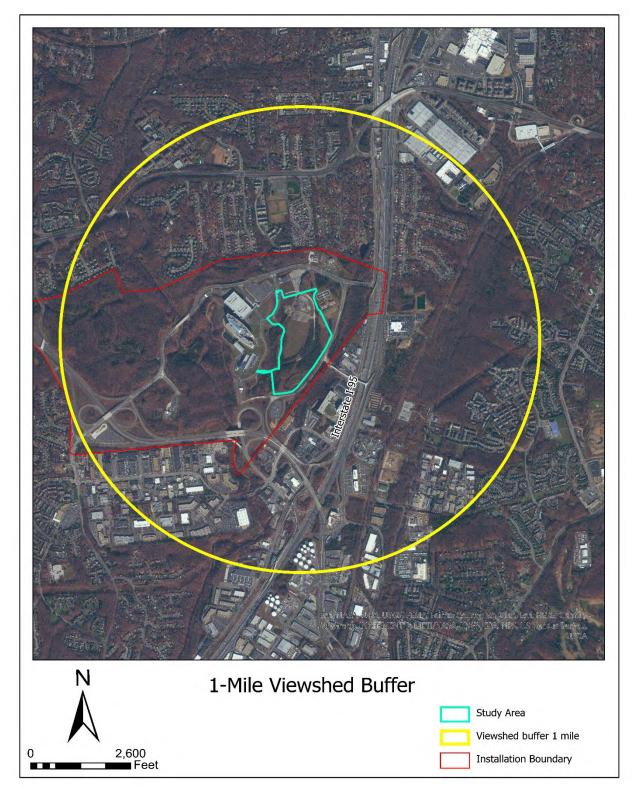


Figure 3-9: 1- Mile Viewshed Buffer

3.10.2 Environmental Consequences

3.10.2.1 Thresholds of Significance

Significant impacts to cultural resources could occur if possible resources that have not been previously documented are not properly identified, consultation pursuant to Section 106 is not completed, and/or if impacts to viewsheds within the APE buffer are not appropriately considered and addressed.

3.10.2.2 Impacts of the Proposed Action

No effects on cultural resources are anticipated from the Proposed Action. The project site has been highly disturbed as a result of testing activities and supporting infrastructure since its inception as a testing ground in the 1940's, with subsequent ground disturbance from removal of older buildings and infrastructure, testing for contamination and munitions, and use of the project site as a staging area for the BRAC construction of NGA in the late 2000's. As noted in Section 3.10.1, no eligible archaeological or architectural resources exist within the APE for the Proposed Action on FBNA. In terms of potential effects to viewsheds of historic districts in the project vicinity, the project is consistent with the campus-style environment Fort Belvoir is striving to establish on FBNA. The administrative building will be designed in accordance with applicable installation design guidelines, including the Fort Belvoir Master Plan, and will be no higher than the adjacent NGA facility. Although situated in a relatively high point in comparison to surrounding areas, the site is surrounded by stands of second-growth pines and hardwood forest that provide a visual screen for off-site properties.

In accordance with Section 106 of the NHPA, consultation has been initiated with the VDHR and Fort Belvoir is seeking concurrence from the SHPO, represented in Virginia by the VDHR, on the determination of "no historic properties affected." A record of this consultation is included in Appendix A: Agency Coordination.

Additionally, should cultural artifacts be inadvertently discovered during construction or operation of the Proposed Action, the inadvertent discovery plan described in Fort Belvoir's Integrated Cultural Resources Management Plan would be implemented to ensure notifications are made to appropriate personnel and the VDHR.

3.10.2.3 Impacts of the No Action Alternative

No effects on cultural resources are anticipated from the No Action Alternative.

3.11 SOCIOECONOMICS, ENVIRONMENTAL JUSTICE, and PROTECTION OF CHILDREN

3.11.1 Socioeconomics

Socioeconomic factors are defined by the interaction or combination of social and economic factors. The relevant factors related to the Proposed Action include population and housing, economic development, and quality of life/health and safety issues.

The Region of Influence (ROI) for socioeconomic characteristics encompasses Fairfax County, Virginia. This ROI includes the installation and the immediately surrounding communities that have direct and indirect socioeconomic relationships with the installation, as many DIA staff live in this county and many military personnel may frequent commercial establishments in this county.

3.11.2 Environmental Justice

Environmental justice addresses the race, ethnicity, and poverty status of populations within the ROI. On February 11, 1994, President Clinton issued EO 12898, *Federal Actions to Address Environmental Justice in Minority Populations and Low-Income Populations* to focus the attention of federal agencies on the human health and environmental conditions in minority and low-income communities. Potential environmental justice considerations are determined by comparing demographic and economic characteristics (minority population composition and poverty rates) within the study area to the same characteristics in the surrounding region. Environmental justice analyses are performed to identify potential disproportionate adverse effects from proposed actions and to identify alternatives that might mitigate these effects (U.S.EPA, 2016).

The term minority refers to people who classified themselves as American Indian or Alaskan Native; Asian or Pacific Islander; African Americans or Black, not of Hispanic origin; or Hispanic.

Minority populations are defined as areas where racial minorities comprise 50 percent or more of the total population. Because CEQ guidance does not establish a threshold for low- income communities, a low-income population is one with at least 25 percent or greater of its population living in poverty for the purposes of this EA.

Demographics

Fairfax County comprises an area of 391 square miles, and the estimated 2019 population was 1,147,532, a 6.1 percent increase from the population of 1,081,726 in 2010 (U.S. Census, 2021). In 2019, 35.3 percent of Fairfax County's population was composed of minorities. Fairfax County is not considered a minority community because the percentage of minorities living in the county is less than 50 percent of the total population. The median household income from 2015 to 2019 (in 2019 dollars) was \$124,831. There were approximately 6 percent of persons living in poverty in Fairfax County. Fairfax County is not considered a low-income community since low-income people and families do not comprise 25 percent or more of the total population (U.S. Census 2021).

Fort Belvoir is approximately 8,000 acres in size and has an approximate working population of 40,000 people (NCPC, 2017). FBNA is 804.07 acres in size and supports approximately 8,600 employees, most of whom are government civilians, military members, and contractors employed by the NGA Campus East (NCE), whose headquarters were completed as part of the 2005 BRAC in September 2011. NCE is the third largest federal facility in Washington, D.C. area, at approximately 2.77 million square feet (https://www.nga.mil/history/).

Approximately 7,500 residents live on Fort Belvoir (2,100 housing units, located on Main Post) (NCPC, 2017).

3.11.3 Protection of Children

On April 21, 1997, President Clinton issued EO 13045, *Protection of Children from Environmental Health Risks and Safety Risks*, directing each federal agency to ensure that its policies, programs, activities, and standards address disproportionate environmental health or safety risks to children that may result from the agency's actions. EO 13045 recognizes that a growing body of scientific knowledge demonstrates that children may suffer disproportionately from environmental health and safety risks due to still developing neurological, immunological, physiological, and behavioral systems. Examples of risks to children include increased traffic volumes and industrial- or production-oriented activities that would generate substances or pollutants that children could come into contact with and ingest.

Typically, children have only been present on FBNA at the Fort Belvoir North CDC #1 and #2, located to the east of the project area. These facilities were constructed in approximately 2013-2014 and provide child care services primarily for the existing NGA facility. The Army has taken precautions for their safety by a number of means, including limiting access to certain areas, the use of fencing, and providing adult supervision.

3.11.2 Environmental Consequences

3.11.2.1 Thresholds of Significance

Socioeconomics

The Proposed Action Alternative and No Action Alternative were evaluated against the following significance criteria to determine if they would result in a significant impact on the socioeconomic environment:

- Alternative would substantially change local population growth rates or employment opportunities.
- Alternative would create a demand for housing, schools, public facilities, or recreational opportunities that exceeds existing supply.
- Alternative would increase risks to public health or safety, including safety of children.

Environmental Justice

The concept of environmental justice is based on the premise that no segment of the population should bear a disproportionate share of adverse human health or environmental effects of a proposed federal action. Historically, low-income and minority communities have been disproportionately affected by negative environmental effects, receiving few of the benefits of economic growth and development while absorbing much of the societal cost.

The Proposed Action Alternative and No Action Alternative were evaluated against the following significance criteria to determine if they would result in a significant impact on environmental justice populations:

• Alternative would cause socioeconomic impacts that disproportionately affect low-income or minority populations.

Protection of Children

Because children may suffer disproportionately from environmental health risks and safety risks, EO 13045 *Protection of Children from Environmental Health Risks and Safety Risks* was issued in 1997 to prioritize the identification and assessment of environmental health risks and safety risks that may affect children and to ensure federal agencies' policies, programs, activities, and standards address environmental and safety risks to children.

The Proposed Action Alternative and No Action Alternative were evaluated against the following significance criteria to determine if they would result in a significant impact on the protection of children:

• Alternative would increase risks to the safety of children.

3.11.2.2 Impacts of the Proposed Action

Socioeconomics

Under the Proposed Action, less-than-significant, long-term beneficial effects would be expected to occur to socioeconomics. The construction and renovation expenditures would result in beneficial increases in the ROI business sales volume, income, and employment. Although the Proposed Action's expenditures would be quite substantial, Fort Belvoir is in such an economically large and robust region that the magnitude of the expenditures relative to the regional demographic and economic forces would be considered minor. Because construction projects are, by nature, temporary, the economic stimulus from construction of the Proposed Action would diminish over time as the project reached completion.

Long-term beneficials impacts would be seen due to the transfer of employees from the current DIA HQ Annex in Washington, D.C. to Fort Belvoir. The new employees would bring an increase in spending throughout the area in a number of areas including housing, employment, and

commercial businesses. Taxes revenues would also increase as a result of employees who relocate. However, these impacts are minor, as some of the Fort Belvoir employees live within an hour of the Proposed Action area and would not relocate. The benefits are also negated by an increase in demand for public services at and surrounding Fort Belvoir such as police, schooling, and firefighter services.

Environmental Justice

Under the Proposed Action, no effects would be anticipated on environmental justice. The ROI for the Proposed Action is not considered to be a minority or low-income community. In addition, the Proposed Action would not be an action that has the potential to substantially affect human health or the environment by excluding persons, denying persons benefits, or subjecting persons to discrimination because of their race, color, national origin, or income level.

Protection of Children

Under the Proposed Action, no adverse or disproportionate effects would be anticipated to occur to children. The CDC are to the east of the site and with proper precautions, would not allow children near the construction site. Post-construction, there would be no environmental risks for children near or in the HQs.

3.11.2.3 Impacts of the No Action Alternative

Socioeconomics

Under the No Action Alternative, no changes would be expected to occur to socioeconomics. Fairfax County would see no changes in employment or need for public services.

Environmental Justice

Under the No Action Alternative, no effects would be anticipated on environmental justice. No changes to minority or low-income communities would occur.

Protection of Children

Under the No Action Alternative, children would not be affected. No changes would occur on-site that had the potential to disproportionately affect children.

4.0 CONCLUSIONS

This EA has been prepared to analyze the potential environmental, cultural, and socioeconomic effects associated with the proposed construction and operation of a new DIA HQs Annex on FBNA. The purpose of this project is to build and operate an administrative building with an

associated parking structure at Fort Belvoir to consolidate administrative facilities for approximately 650 personnel from DIA HQs to address safety, security, and operational concerns specific to the administrative functions of the agency.

The analysis within this EA concluded that there would be: no impacts to topography, groundwater, wetlands, coastal zones, environmental justice, protection of children, cultural resources, air quality, and utilities; short-term minor adverse impacts to surface water, wildlife resources, vegetation, noise and transportation; long-term minor beneficial impacts to topography and soils, and hazardous materials and munitions; and short-term minor beneficial impacts to socioeconomics.

Table 4-1 summarizes the potential consequences that the Proposed Action and the No-Action
 Alternative would have on environmental resources.

Based on the evaluation of the environmental consequences accomplished by this EA, the Proposed Action would have no significant impact on the environment, and the preparation of an EIS is not warranted. The preparation of a FNSI will be appropriate.

Table 4-1: Summary of Potential Environmental Consequences on Environmental Resources

Resource	Proposed Action	No Action Alternative	Permits and Best Management and Mitigation Measures
Geology, topography, and soils	Less-than-significant, short- term adverse effects to soils. Clearing, grubbing and grading would temporarily increase erosion and the potential for sediments to be transported off-site; however, the finished building would be beneficial in reducing accelerated rates of run-off from adversely affecting downstream receiving waters as a result of properly designed stormwater management	Less-than- significant adverse impacts to soils.	-Obtain ground disturbance permits from Fort Belvoir DPW -Follow ESC Plan (to be included in the project civil design plan following review by Fort Belvoir DPW and approval by VDEQ) -Follow SWPPP -Obtain Construction General Permit from VDEQ.
Water resources (Surface water, RPAs, floodplains, groundwater, stormwater, Coastal Zone)	Less-than-significant, short- term adverse effects. This stage of construction exposes soils and increases the potential for erosion and discharge of sediment-laden stormwater to downstream receiving waters; however, appropriate erosion and sediment control measures would be implemented, pursuant to the construction SWPPP and the VSMP Construction General Permit, and would minimize any detrimental effects. Construction of permanent stormwater management features will handle stormwater generated from the development and be designed to maintain pre-development levels of off-site discharge.	Less-than- significant adverse impacts to surface waters. Existing conditions would continue.	 -Obtain CGP -Follow ESC and SWPPP as referenced above. -Design and construction would be performed in accordance with Virginia CZMA policies. -Obtain permit for impacts to wetlands/streams pursuant to Section 401/404 of the CWA prior to disturbance to these resources - All temporarily disturbed areas would be graded and re-vegetated upon completion of construction -Employ erosion and sediment control measures during construction, to include silt fencing and sediment traps.

			 Provide spill kits on site in the event of an accidental release of petroleum products from construction equipment. Provide appropriate secondary containment for on-site generators.
Biological resources (Vegetation, wildlife, RTE species, PIF)	Less-than-significant, short- term adverse effects to vegetation, wildlife, and RTE. The Proposed Action would remove existing vegetation, disturbing habitat areas and causing fauna that use the area to relocate. The vegetation/tree removal would be offset with replantings, and the construction area stabilized and revegetated with native plants.	No effects.	 -Replanting to offset removal of existing trees within the site would be performed in accordance with Fort Belvoir's Tree Removal and Protection Policy. -Consultation regarding listed species would be conducted pursuant to Section 7 of the ESA. -Surveys for the presence of the wood turtle and small-whorled pogonia would be conducted prior to site clearing. - Perimeter controls would be installed during the winter months to exclude the endangered wood turtle from proposed areas of construction activity, as necessary. - To minimize impacts to birds, construction activities would avoid cutting and removal of vegetation from April 1 to July 15. - To protect nesting bat species, no trees over 3 inches in diameter would be removed within the project study area between April 15 and September 15.

Hazardous	Less-than-significant	No effects.	-Clean-up of groundwater
Waste Materials	beneficial effects to hazardous	no effects.	contamination at the site
and Munitions	waste and munitions.		would be conducted in
	Remediation of the		accordance with CERCLA
	contaminated groundwater at		and Fort Belvoir's IRP,
	the site will be completed in		using steps and
	accordance with the final CC-		methodology outlined in
	MPS-2009 Feasibility Study,		the final Feasibility Study.
	and the munitions survey		-Munitions clearance
	would ensure the Proposed		would be conducted
	Action area is cleared from		pursuant to U.S. Army
	munitions., alleviating safety		Garrison (USAG), Fort
	concerns related to possible		Belvoir, Policy
	munitions remaining on the		Memorandum #28 (USAG,
	surface or buried near the		2014).
	surface.		-Land use controls, likely
			to result in the requirement
			for a vapor intrusion
			barrier for the
			administrative building,
			would continue to be in
			effect for this site.
			-Ongoing remedial actions
			would be enhanced through
			the re-establishment of an
			effective groundwater
			monitoring well system
			that would be able to more
			accurately characterize the
			-
			contamination plume. -Soils excavated or
			otherwise disturbed during
			the project's construction
			phase would be tested in
			accordance with
			established Fort Belvoir
			policies and procedures.
			-The construction
			contractor would be
			required to prepare and
		2.2. 00	adhere to a SPCC plan.
Utilities	Less-than-significant, long-	No effects	Any required ground
(Electric,	term adverse effects to electric,		disturbance associated with
Wastewater, and	wastewater, and natural gas.		the extension of existing
Natural Gas)	The operation of the building		utilities for connection to

7 III Space	effects		
Noise Air Space	Less-than-significant, short- term adverse effects during the construction period would occur as a result of the various types of heavy equipment needed. BMPs (listed in this section) would be employed to minimize the adverse effects from construction noise. Operation of the completed facility would be expected to result in a negligible increase in ambient noise from climate control (heating/cooling) infrastructure supporting the building and additional commuting vehicles.	No effects	-The Fairfax County noise ordinance limits construction noise above 60 dBA for residential areas during weekdays. -Noise levels must not exceed National Institute for Occupational Safety and Health (NIOSH) or Occupational Safety and Health Administration (OSHA) guidance for workers. -To minimize the potential adverse impact from these noises, construction vehicles would be equipped with noise- dampening equipment including mufflers which would be operated according to the manufacturers' instructions. -Construction vehicles and equipment would be turned off when not in use for more than five minutes. -Construction would take place during daylight hours on weekdays, unless there is a specific action that would require working outside of this normal timeframe, such as mobilizing oversized materials or equipment to the site.
	would increase demand, but the existing utility systems have been constructed in consideration of long-term build-out of FBNA.		the Proposed Action would adhere to the required sediment and erosion control permits.

Air Quality	Less-than-significant, short- and long-term adverse effects. During construction engine emissions and potential fugitive dust emissions would have adverse effects; however, these impacts would be minimized through BMPs. Long-term operation of the facility would result in de minimis emissions.	No effects	-Comply with VDEQ's Fort Belvoir - North Area synthetic minor New Source Review (mNSR) air permit (Registration No. 73630) -BMPs include: covering truck beds while in transit to reduce fugitive emissions; spraying water on any unpaved roads or stockpiles to limit fugitive emissions; using ultra-low sulfur diesel as a fuel source where appropriate to minimize oxides of sulfur emissions; using clean diesel in construction equipment and vehicles though the implementation of add-on control
Traffic	these impacts would be minimized through BMPs. Long-term operation of the facility would result in de minimis emissions.	No effects	-BMPs include: covering truck beds while in transit to reduce fugitive emissions; spraying water on any unpaved roads or stockpiles to limit fugitive emissions; using ultra-low sulfur diesel as a fuel source where appropriate to minimize oxides of sulfur emissions; using clean diesel in construction equipment and vehicles though the implementation
	construction worker commutes and delivery/pickup of construction materials/debris. Less-than-significant long- term effects of increased		

	personnel commuting to/from FBNA.		
Cultural and Historic Resources	No effects. No sites eligible for listing on the NRHP are located within the study area.	No effects	-Consultation in accordance with Section 106 of the NHPA required. -Inadvertent discovery of cultural resources would be managed according to procedures documented in Fort Belvoir's ICRMP.
Socioeconomics, Environmental Justice, and Protection of Children	Less-than-significant, short- term beneficial effects to socioeconomics due to the potential employment of local construction workers and purchasing of materials from local vendors.	No effects	The Proposed Action would be initiated only after this environmental review has been completed and the appropriate permits are acquired. It is anticipated that the permitting process would result in assurance of safety and protection of the public, including children. -Proper precautions including the placement of fencing, signage, and other types of barriers would be used to prevent potential harm to all civilians, including children.

5.0 ACRONYMS

ACP	Access Control Point
ADNL/dBA	A-weighted day night-levels
ADP	Area Development Plan
АНА	Activity Hazard Analysis
AIRFA	American Indian Religious Freedom Act
APE	Area of Potential Effect
ARPA	Archaeological Resource Protection Act
AT/FP	anti-terrorism/force protection
AQCR	Air Quality Control Regions
BMP	best management practices
BO	Biological Opinion
BRAC	Base realignment and closure
CAA	Clean Air Act
CBPO	Chesapeake Bay Preservation Ordinance
CDC	Child Development Center
CDNL/dBC	C-weighted decibels day night-levels
CEQ	Council of Environmental Quality
CERCLA	Comprehensive Environmental Response, Compensation, and
	Liability Act
CFR	Code of Federal Regulations
CH ₄	methane
CO	carbon monoxide
CO_2	carbon dioxide
CO ₂ e	equivalent emissions of CO ₂
COC	chemicals of concern
CRMP	Coastal Resources Management Program
CWA	Clean Water Act
CZMA	Coastal Zone Management Act
DA	Department of the Army
DAAF	Davison Army Airfield
dB	Decibels
dBP	peak sound level
DEQ	Department of Environmental Quality
DERP	Defense Environmental Restoration Program
DIA	Defense Intelligence Agency
DIAC	Defense Intelligence Analysis Center
DMM	discarded military munitions
DNL	day-night levels
DoD	Department of Defense
DPW	Department of Public Works

DVP	Dominion Virginia Power
DWR	Department of Wildlife Resources
EA	Environmental Assessment
EI	Environmental Investigation
EIS	Environmental Impact Statement
EISA	Energy Independence and Security Act
ENRD	Environmental and Natural Resources Division
EO	Executive Orders
EPA	U.S. Environmental Protection Agency
EPG	Engineering Proving Ground
ESA	Endangered Species Act
FBNA	Fort Belvoir North Area
FFS	Focused Feasibility Study
FNSI	Finding of No Significant Impact
FS	feasibility study
FUDS	Formerly Used Defense Sites
GCR	General Conformity Rule
GHGs	greenhouse gases
GIS	Geographic Information System
GSA	General Services Administration
GSF	gross square foot
GWP	global warming potential
HAP	Hazardous Air Pollutant
HDPE	high-density polyethylene
HFCs	hydrofluorocarbons
HHRA	Human Health Risk Assessment
HQ	Headquarters
IC	Institutional controls
Infosec	information security
INRMP	Integrated Natural Resources Management Plan
IPaC	Information for Planning and Conservation
IRP	Installation Restoration Program
ISW RO	Industrial Stormwater Outfall
kV	kilovolt
LEED	Leadership in Energy and Environmental Design
LID	low impact development
LOD	limits of disturbance
LOS	Level of Service

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LUC	land use controls
LUCIP	Land Use Control Implementation Plan
LUPZ	Land Use Planning Zone
MC	Munitions constituents
MDW	Military District of Washington
MEC	munitions and explosives of concern
MGD	million gallons per day
MMRP	Military Munitions Response Program
MS4	Municipal Separate Storm Sewer System
MWCOG	Metropolitan Washington Council of Governments
NAAQS NAGPRA NCE NCP NCR NEPA NHPA NGA NIOSH NIOSH NLEB NOI NO $_{\rm X}$ N2O NPDES NRCS NRHP NRO NSF NSR	National Ambient Air Quality Standards Native American Graves Protection and Repatriation Act NGA Campus East National Contingency Plan National Capital Region National Environmental Policy Act National Historic Preservation Act National Geospatial-Intelligence Agency National Institute for Occupational Safety and Health northern long-eared bat Notice of Intent nitrogen dioxides nitrous oxide National Pollutant Discharge Elimination System Natural Resources Conservation Service National Register of Historic Properties Northern Regional Office net square foot noise-sensitive receptors
O ₃	ozone
OHWM	Ordinary High-Water Mark
OMB	Office of Management and Budget
OSAA	Operational Support Airlift Agency
OSACOM	Operational Support Airlift Command
OSHA	Occupational Safety and Health Administration
PFCs	perfluorocarbons
PFO	palustrine forested
PIF	Partners in Flight
PP	proposed plan
PM	particulate matter
PRG	Preliminary Remediation Goals

U.S. Army Corps of Engineers June 2021

PSA	Petroleum Storage Areas
RBC	Risk-Based Concentrations
RD	remedial design
RIP	Restoration in place
ROD	Record of Decision
RPA	Riparian Buffer Area
RTE	Rare, Threatened and Endangered
SARA	Superfund Amendments and Reauthorization Act
SF_6	sulfur hexafluoride
SHPO	State Historic Preservation Office
SIP	state implementation plan
SO_2	sulfur dioxide
SOC	Species of Concern
SPCC	Spill Prevention, Control, and Countermeasure
SSHP	Site Safety and Health Plan
SWPP	Stormwater Pollution Prevention Plan
TEMPEST	Telecommunications Electronics Materials Protected from
	Emanating Spurious Transmissions
TIS	Traffic Impact Study
TMC	Turning Movement Counts
TMDL	Total Maximum Daily Load
TPH	Total Petroleum Hydrocarbons
TSS	Total Suspended Solids
UAG	U.S. Army Garrison
UP	Utilities Privatization
USACE	U.S. Army Corps of Engineers
USC	U.S. Code
USDA	United States Department of Agriculture
USFWS	U.S. Fish and Wildlife Service
UST	underground fuel storage tank
UXO	unexploded ordnance
VAC	Virginia Administrative Code
VADEQ	Virginia's Department of Environmental Quality
VDHR	Virginia Department of Historic Resources
VDWR	Virginia Department of Wildlife Resources
VI	vapor intrusion
VISL	vapor intrusion screening level
VOC	volatile organic compounds
VSMP	Virginia Stormwater Management Program

6.0 LIST OF PREPARERS

US Army Corps of Engineers, Baltimore District		
Name	Project Responsibility	Organization
Connie Ramsey	Project Manager	Planning Division
Lauren Joyal	Biologist	Planning Division
Heather Cisar	NEPA Program Manager	Planning Division

7.0 REFERENCES

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APPENDIX A – AGENCY COORDINATION

Draft EA Construction of DIA HQ Annex Fort Belvoir, Virginia U.S. Army Corps of Engineers June 2021

NOTICE OF AVAILABILITY FOR THE ENVIRONMENTAL ASSESSMENT AND DRAFT FINDING OF NO SIGNIFICANT IMPACT FOR THE PROPOSED DEFENSE INTELLIGENCE AGENCY (DIA) HEADQUARTERS (HQ) ANNEX FORT BELVOIR, VIRGINIA

The U.S. Army Garrison Fort Belvoir hereby gives Notice of the Availability (NOA) for the Environmental Assessment (EA) and Draft Finding of No Significant Impact (FNSI) for the proposed construction of the DIA HQ Annex within the vicinity of the National Geospatial-Intelligence Agency (NGA) complex on Fort Belvoir's North Area, Fairfax County, Virginia. The purpose of this project is to build and operate an approximately 77,000 net square foot/116,080 gross square foot administrative building with an associated parking structure on Fort Belvoir to consolidate administrative facilities for approximately 650 personnel from DIA HQ to address safety, security, and operational concerns specific to the administrative functions of the agency. The proposed HQ Annex building would include a multi-story administrative building with offices, cubicles/workstations, publications rooms, conference rooms, break rooms, server rooms, a multi-purpose auditorium, a café/cafeteria, a gym/fitness center, a utility plant, visitor control center, visitor parking, a secured employee parking structure, and a perimeter security fence.

The EA has been prepared in accordance with the regulations for implementing the procedural provisions of the National Environmental Policy Act (NEPA), (Public Law 91-190, 42 USC 4321-4347 January 1, 1970), amendments, and the Army's Implementing Regulations (32 CFR Part 651, Environmental Analysis of Army Actions). The EA is available to view in printed form at the Lorton Branch, Kingstowne Branch, and Sherwood Regional Branch of the Fairfax County Public Library system, or to view/download electronically at https://home.army.mil/belvoir/index.php/about/Garrison/directorate-publicworks/environmental-division. Click the "Programs and Documents" tab, then "National Environmental Policy Act (NEPA) Program." Information about the EA and links to download the various documents are provided under the "Open for Public/Agency Review & Comment" heading.

Comments or questions on the EA and Draft FNSI may be directed in writing to: Environmental Division, Directorate of Public Works, Building 1442, 9430 Jackson Loop, Fort Belvoir, VA 22060, or by email to: <u>usarmy.belvoir.imcom-atlantic.mbx.enrd@mail.mil</u>. Comments must be received no later than 30 days after publication of this NOA.

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Chris Daniel Program Analyst Advisory Council on Historic Preservation 401 F Street NW, Suite 308 Washington , D.C. 20001

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Chairwoman Katherine Ward Mount Vernon Council of Citizen's Associations P.O. Box 203 Mount Vernon, Virginia 22121

Chairman David Dale Mount Vernon Council of Citizen's Associations P.O. Box 203 Mount Vernon , VA 22121

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Mr. Marcel Acosta Executive Director National Capital Planning Commission 401 Ninth Street NW Suite 500, North Lobby Washington, DC 20004

Mr. Kevin Casalenuovo Park Manager Pohick Bay Regional Park 6501 Pohick Bay Drive Lorton, VA 22079

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Mr. Marc Holma Architectural Historian Virginia Department of Historic Resources 2801 Kensington Avenue Richmond, Virginia 23221



United States Department of the Interior

FISH AND WILDLIFE SERVICE Virginia Ecological Services Field Office 6669 Short Lane Gloucester, VA 23061-4410 Phone: (804) 693-6694 Fax: (804) 693-9032 http://www.fws.gov/northeast/virginiafield/



May 07, 2021

In Reply Refer To: Consultation Code: 05E2VA00-2021-SLI-3582 Event Code: 05E2VA00-2021-E-10392 Project Name: DIA HQ Annex

Subject: List of threatened and endangered species that may occur in your proposed project location or may be affected by your proposed project

To Whom It May Concern:

The enclosed species list identifies threatened, endangered, proposed and candidate species, as well as proposed and final designated critical habitat, that may occur within the boundary of your proposed project and/or may be affected by your proposed project. The species list fulfills the requirements of the U.S. Fish and Wildlife Service (Service) under section 7(c) of the Endangered Species Act (Act) of 1973, as amended (16 U.S.C. 1531 *et seq.*). Any activity proposed on National Wildlife Refuge lands must undergo a 'Compatibility Determination' conducted by the Refuge. Please contact the individual Refuges to discuss any questions or concerns.

New information based on updated surveys, changes in the abundance and distribution of species, changed habitat conditions, or other factors could change this list. Please feel free to contact us if you need more current information or assistance regarding the potential impacts to federally proposed, listed, and candidate species and federally designated and proposed critical habitat. Please note that under 50 CFR 402.12(e) of the regulations implementing section 7 of the Act, the accuracy of this species list should be verified after 90 days. This verification can be completed formally or informally as desired. The Service recommends that verification be completed by visiting the ECOS-IPaC website at regular intervals during project planning and implementation for updates to species lists and information. An updated list may be requested through the ECOS-IPaC system by completing the same process used to receive the enclosed list.

The purpose of the Act is to provide a means whereby threatened and endangered species and the ecosystems upon which they depend may be conserved. Under sections 7(a)(1) and 7(a)(2) of the Act and its implementing regulations (50 CFR 402 *et seq.*), Federal agencies are required to utilize their authorities to carry out programs for the conservation of threatened and endangered

species and to determine whether projects may affect threatened and endangered species and/or designated critical habitat.

A Biological Assessment is required for construction projects (or other undertakings having similar physical impacts) that are major Federal actions significantly affecting the quality of the human environment as defined in the National Environmental Policy Act (42 U.S.C. 4332(2) (c)). For projects other than major construction activities, the Service suggests that a biological evaluation similar to a Biological Assessment be prepared to determine whether the project may affect listed or proposed species and/or designated or proposed critical habitat. Recommended contents of a Biological Assessment are described at 50 CFR 402.12.

If a Federal agency determines, based on the Biological Assessment or biological evaluation, that listed species and/or designated critical habitat may be affected by the proposed project, the agency is required to consult with the Service pursuant to 50 CFR 402. In addition, the Service recommends that candidate species, proposed species and proposed critical habitat be addressed within the consultation. More information on the regulations and procedures for section 7 consultation, including the role of permit or license applicants, can be found in the "Endangered Species Consultation Handbook" at:

http://www.fws.gov/endangered/esa-library/pdf/TOC-GLOS.PDF

Please be aware that bald and golden eagles are protected under the Bald and Golden Eagle Protection Act (16 U.S.C. 668 *et seq*.), and projects affecting these species may require development of an eagle conservation plan

(http://www.fws.gov/windenergy/eagle_guidance.html). Additionally, wind energy projects should follow the wind energy guidelines (http://www.fws.gov/windenergy/) for minimizing impacts to migratory birds and bats.

Guidance for minimizing impacts to migratory birds for projects including communications towers (e.g., cellular, digital television, radio, and emergency broadcast) can be found at: http://www.fws.gov/migratorybirds/CurrentBirdIssues/Hazards/towers/towers.htm; http://www.towerkill.com; and htt www.fws.gov/migratorybirds/CurrentBirdIssues/Hazards/towers/comtow.html.

http://

We appreciate your concern for threatened and endangered species. The Service encourages Federal agencies to include conservation of threatened and endangered species into their project planning to further the purposes of the Act. Please include the Consultation Tracking Number in the header of this letter with any request for consultation or correspondence about your project that you submit to our office.

Attachment(s):

- Official Species List
- USFWS National Wildlife Refuges and Fish Hatcheries

Official Species List

This list is provided pursuant to Section 7 of the Endangered Species Act, and fulfills the requirement for Federal agencies to "request of the Secretary of the Interior information whether any species which is listed or proposed to be listed may be present in the area of a proposed action".

This species list is provided by:

Virginia Ecological Services Field Office 6669 Short Lane Gloucester, VA 23061-4410 (804) 693-6694

Project Summary

Approximate location of the project can be viewed in Google Maps: <u>https://www.google.com/maps/@38.7523379,-77.19079239440862,14z</u>



Counties: Fairfax County, Virginia

Endangered Species Act Species

There is a total of 2 threatened, endangered, or candidate species on this species list.

Species on this list should be considered in an effects analysis for your project and could include species that exist in another geographic area. For example, certain fish may appear on the species list because a project could affect downstream species.

IPaC does not display listed species or critical habitats under the sole jurisdiction of NOAA Fisheries¹, as USFWS does not have the authority to speak on behalf of NOAA and the Department of Commerce.

See the "Critical habitats" section below for those critical habitats that lie wholly or partially within your project area under this office's jurisdiction. Please contact the designated FWS office if you have questions.

1. <u>NOAA Fisheries</u>, also known as the National Marine Fisheries Service (NMFS), is an office of the National Oceanic and Atmospheric Administration within the Department of Commerce.

Mammals

NAMESTATUSNorthern Long-eared Bat Myotis septentrionalis
No critical habitat has been designated for this species.
Species profile: https://ecos.fws.gov/ecp/species/9045ThreatenedFlowering Plants
NAMESTATUSSmall Whorled Pogonia Isotria medeoloides
No critical habitat has been designated for this species.
Species profile: https://ecos.fws.gov/ecp/species/1890Threatened

Critical habitats

THERE ARE NO CRITICAL HABITATS WITHIN YOUR PROJECT AREA UNDER THIS OFFICE'S JURISDICTION.

3

USFWS National Wildlife Refuge Lands And Fish Hatcheries

Any activity proposed on lands managed by the <u>National Wildlife Refuge</u> system must undergo a 'Compatibility Determination' conducted by the Refuge. Please contact the individual Refuges to discuss any questions or concerns.

THERE ARE NO REFUGE LANDS OR FISH HATCHERIES WITHIN YOUR PROJECT AREA.



United States Department of the Interior

FISH AND WILDLIFE SERVICE Virginia Ecological Services Field Office 6669 Short Lane Gloucester, VA 23061-4410 Phone: (804) 693-6694 Fax: (804) 693-9032 http://www.fws.gov/northeast/virginiafield/



June 23, 2021

In Reply Refer To: Consultation code: 05E2VA00-2021-TA-3582 Event Code: 05E2VA00-2021-E-12582 Project Name: DIA HQ Annex

Subject: Verification letter for the 'DIA HQ Annex' project under the January 5, 2016, Programmatic Biological Opinion on Final 4(d) Rule for the Northern Long-eared Bat and Activities Excepted from Take Prohibitions.

Dear Connie Ramsey:

The U.S. Fish and Wildlife Service (Service) received on June 23, 2021 your effects determination for the 'DIA HQ Annex' (the Action) using the northern long-eared bat (*Myotis septentrionalis*) key within the Information for Planning and Consultation (IPaC) system. This IPaC key assists users in determining whether a Federal action is consistent with the activities analyzed in the Service's January 5, 2016, Programmatic Biological Opinion (PBO). The PBO addresses activities excepted from "take"^[1] prohibitions applicable to the northern long-eared bat under the Endangered Species Act of 1973 (ESA) (87 Stat.884, as amended; 16 U.S.C. 1531 et seq.).

Based upon your IPaC submission, the Action is consistent with activities analyzed in the PBO. The Action may affect the northern long-eared bat; however, any take that may occur as a result of the Action is not prohibited under the ESA Section 4(d) rule adopted for this species at 50 CFR §17.40(o). Unless the Service advises you within 30 days of the date of this letter that your IPaC-assisted determination was incorrect, this letter verifies that the PBO satisfies and concludes your responsibilities for this Action under ESA Section 7(a)(2) with respect to the northern long-eared bat.

Please report to our office any changes to the information about the Action that you submitted in IPaC, the results of any bat surveys conducted in the Action area, and any dead, injured, or sick northern long-eared bats that are found during Action implementation. If the Action is not completed within one year of the date of this letter, you must update and resubmit the information required in the IPaC key.

This IPaC-assisted determination allows you to rely on the PBO for compliance with ESA Section 7(a)(2) <u>only</u> for the northern long-eared bat. It **does not** apply to the following ESA-protected species that also may occur in the Action area:

• Small Whorled Pogonia *Isotria medeoloides* Threatened

If the Action may affect other federally listed species besides the northern long-eared bat, a proposed species, and/or designated critical habitat, additional consultation between you and this Service office is required. If the Action may disturb bald or golden eagles, additional coordination with the Service under the Bald and Golden Eagle Protection Act is recommended.

^[1]Take means to harass, harm, pursue, hunt, shoot, wound, kill, trap, capture, or collect, or to attempt to engage in any such conduct [ESA Section 3(19)].

Action Description

You provided to IPaC the following name and description for the subject Action.

1. Name

DIA HQ Annex

2. Description

The following description was provided for the project 'DIA HQ Annex':

Administrative building with parking and security fence on FBNA.

Approximate location of the project can be viewed in Google Maps: <u>https://www.google.com/</u> <u>maps/@38.7523379,-77.19079239440862,14z</u>



Determination Key Result

This Federal Action may affect the northern long-eared bat in a manner consistent with the description of activities addressed by the Service's PBO dated January 5, 2016. Any taking that may occur incidental to this Action is not prohibited under the final 4(d) rule at 50 CFR §17.40(o). Therefore, the PBO satisfies your responsibilities for this Action under ESA Section 7(a)(2) relative to the northern long-eared bat.

Determination Key Description: Northern Long-eared Bat 4(d) Rule

This key was last updated in IPaC on May 15, 2017. Keys are subject to periodic revision.

This key is intended for actions that may affect the threatened northern long-eared bat.

The purpose of the key for Federal actions is to assist determinations as to whether proposed actions are consistent with those analyzed in the Service's PBO dated January 5, 2016.

Federal actions that may cause prohibited take of northern long-eared bats, affect ESA-listed species other than the northern long-eared bat, or affect any designated critical habitat, require ESA Section 7(a)(2) consultation in addition to the use of this key. Federal actions that may

affect species proposed for listing or critical habitat proposed for designation may require a conference under ESA Section 7(a)(4).

Determination Key Result

This project may affect the threatened Northern long-eared bat; therefore, consultation with the Service pursuant to Section 7(a)(2) of the Endangered Species Act of 1973 (87 Stat.884, as amended; 16 U.S.C. 1531 et seq.) is required. However, based on the information you provided, this project may rely on the Service's January 5, 2016, *Programmatic Biological Opinion on Final 4(d) Rule for the Northern Long-Eared Bat and Activities Excepted from Take Prohibitions* to fulfill its Section 7(a)(2) consultation obligation.

Qualification Interview

- 1. Is the action authorized, funded, or being carried out by a Federal agency? *Yes*
- 2. Have you determined that the proposed action will have "no effect" on the northern longeared bat? (If you are unsure select "No")

No

3. Will your activity purposefully Take northern long-eared bats?

No

4. [Semantic] Is the project action area located wholly outside the White-nose Syndrome Zone?

Automatically answered
No

5. Have you contacted the appropriate agency to determine if your project is near a known hibernaculum or maternity roost tree?

Location information for northern long-eared bat hibernacula is generally kept in state Natural Heritage Inventory databases – the availability of this data varies state-by-state. Many states provide online access to their data, either directly by providing maps or by providing the opportunity to make a data request. In some cases, to protect those resources, access to the information may be limited. A web page with links to state Natural Heritage Inventory databases and other sources of information on the locations of northern longeared bat roost trees and hibernacula is available at <u>www.fws.gov/midwest/endangered/</u> <u>mammals/nleb/nhisites.html.</u>

Yes

6. Will the action affect a cave or mine where northern long-eared bats are known to hibernate (i.e., hibernaculum) or could it alter the entrance or the environment (physical or other alteration) of a hibernaculum?

No

7. Will the action involve Tree Removal?

Yes

- 8. Will the action only remove hazardous trees for the protection of human life or property? *No*
- 9. Will the action remove trees within 0.25 miles of a known northern long-eared bat hibernaculum at any time of year?

No

10. Will the action remove a known occupied northern long-eared bat maternity roost tree or any trees within 150 feet of a known occupied maternity roost tree from June 1 through July 31?

No

Project Questionnaire

If the project includes forest conversion, report the appropriate acreages below. Otherwise, type '0' in questions 1-3.

1. Estimated total acres of forest conversion:

0

2. If known, estimated acres of forest conversion from April 1 to October 31

0

3. If known, estimated acres of forest conversion from June 1 to July 31

0

If the project includes timber harvest, report the appropriate acreages below. Otherwise, type '0' in questions 4-6.

4. Estimated total acres of timber harvest

0

5. If known, estimated acres of timber harvest from April 1 to October 31

0

6. If known, estimated acres of timber harvest from June 1 to July 31

0

If the project includes prescribed fire, report the appropriate acreages below. Otherwise, type '0' in questions 7-9.

7. Estimated total acres of prescribed fire

0

8. If known, estimated acres of prescribed fire from April 1 to October 31

0

9. If known, estimated acres of prescribed fire from June 1 to July 31

0

If the project includes new wind turbines, report the megawatts of wind capacity below. Otherwise, type '0' in question 10.

10. What is the estimated wind capacity (in megawatts) of the new turbine(s)?

0

APPENDIX B – AIR QUALITY RECORD OF NON-APPLICABILITY

GENERAL CONFORMITY – RECORD OF NON-APPLICABILITY

Project/Action Name:	DIA HQ Annex Facility
Project/Action Point of Contact:	Connie Ramsey (410) 209-7589 Baltimore District, Corps of Engineers
Begin Date (Anticipated): May 2024	End Date (Anticipated): February 2026

The Proposed Action involves the construction of the headquarters annex building within Fort Belvoir's North Area (FBNA), in the vicinity of the National Geospatial-Intelligence Agency (NGA) complex. The proposed headquarters annex building would be approximately 116,080 SF and would include a multi-story administrative building with offices, cubicles/workstations, publications rooms, conference rooms, break rooms, server rooms, a multi-purpose auditorium, café/cafeteria and gym/fitness center, and a parking structure, all on a 12.5-acre site, to support stationing of approximately 650 personnel.

Emissions for Building Construction:

Volatile Organic Compounds (VOC)	3.51 tons per year $(tpy)^{(1)}$
Nitrogen Oxides (NO _x)	34.11 tpy
Sulfur Oxides (SO _x)	2.64 tpy
Carbon Monoxide (CO)	18.61 tpy
Particulate Matter Less than 2.5 μ m (PM _{2.5})	34.77 tpy

⁽¹⁾ Values were obtained by dividing the calculated total emissions by 2, assuming a construction window of approximately 2 years, to obtain the tons per year (tpy) value.

Emissions for Building Operation (Generator):	
Volatile Organic Compounds (VOC)	0.22 tons per year $(tpy)^{(2)}$
Nitrogen Oxides (NO _x)	8.05 tpy
Sulfur Oxides (SO _x)	0.004 tpy
Carbon Monoxide (CO)	1.84 tpy
Particulate Matter Less than 2.5 µm (PM _{2.5})	0.24 tpy

⁽²⁾ Calculations performed using a conservative estimate of 500 hours of run-time per year at maximum output.

General Conformity under the Clean Air Act, Section 176 has been evaluated for the project described above according to the requirements of 40 CFR 93, Subpart B. The requirements of this rule are not applicable to this project/action because the highest annual emissions from this project/action have been estimated to be under the applicability thresholds as below:

Conformity Threshold Rate

VOC	50 tpy
NO _x	100 tpy
SO _x	100 tpy
CO	100 tpy

100 tpy

Supporting documentation and emissions estimates are attached.

Harback W

Wilamena Harback Chief, Environmental Division

Date dozi

Joshna P. SeGraves Colonel, US Army Commanding

1 3 JUL 2021

Date

RECORD OF NON-APPLICABILITY (RONA) SUPPORTING DOCUMENTATION For DIA HQ Annex Facility

The purpose of this documentation is to support General Conformity applicability determinations under the Clean Air Act, Section 176 for the Defense Intelligence Agency (DIA) Headquarters (HQ) Annex located on Fort Belvoir, Fairfax County, Virginia. This document provides an estimate of worst-case emissions from the proposed construction and operation of a 6-story administration building with an associated parking garage. The emission estimates for which this documentation was developed were based on the following assumptions:

Project Characteristics and Area Disturbed

- Construction and operation of a 6-story approximately116,080 SF Command and Control Facility (C2F) building and an approximately 135,000 SF parking garage to accommodate 650 personnel.
- A total of approximately 12.5 acres will be cleared and grubbed.
- The LOD will be cleared of all vegetation, topsoil, and unsuitable material in order to prepare the site for construction. Topsoil will be reserved for use in final grading of the site.
- As construction activities will occur throughout the project to varying degrees, a project duration of approximately 510 days (2 years) was used.

Contractor and Equipment Assumptions

- Assumed sixty contractor staff would be on-site for 510 working days to complete this work. Approximately 60% would commute to the site each day in a light duty diesel truck, with a round trip of 30 miles.
- Assumed 6 heavy duty diesel trucks would come to the site (again, 30 mile roundtrip) each construction day, to mobilize and demobilize the equipment.
- Assumed durations of operation for heavy equipment are explicitly identified in the enclosed spreadsheet where air emissions are quantified for this project. This includes the following:
 - Estimated equipment to be used includes skid steer (bobcat), cement mixers, plate compactors, lifts, excavators, backhoes, asphalt pavers, paving equipment, graders, scrapers, cranes, and dumpers/tenders. To develop a conservative estimate, it was assumed that 9 skid steers would be used 8 hours a day for one year, and one scraper and one crane would be used 8 hours a day for one year.

Also, it was assumed 6 each of the remaining equipment would be used for 8 hours a day for two years.

Project Duration

- Assumed to be 510 working days, or two years, which will dictate contractor travel to the site, and the number of 8-hour days over which fugitive dust emissions will be generated as a result of the work performed.
- Operational emissions will result from the project (i.e., permanent air emissions sources from the generator).

Emissions

The emission calculations to quantify these values are presented in the following tables, and were performed using methodology and information provided in the *Air Emissions Guide for Air Force Mobile Sources*, U.S. Air Force Installations, 2020, Air Emissions Guide to Air Force Transitory Sources, 2016, and Air Emissions Factor Guide to Air Force Stationary Sources, 2020.

Emissions for Building Construction:	
Volatile Organic Compounds (VOC)	3.51 tons per year $(tpy)^{(1)}$
Nitrogen Oxides (NO _x)	34.11 tpy
Sulfur Oxides (SO _x)	2.64 tpy
Carbon Monoxide (CO)	18.61 tpy
Particulate Matter Less than 2.5 µm (PM _{2.5})	34.77 tpy

⁽¹⁾ Values were obtained by dividing the total calculated construction emissions by 2, assuming a construction window of approximately 2 years, to obtain the tons per year (tpy) value.

Emissions for Building Operation (Generator):	
Volatile Organic Compounds (VOC)	0.22 tons per year $(tpy)^{(2)}$
Nitrogen Oxides (NO _x)	8.05 tpy
Sulfur Oxides (SO _x)	0.004 tpy
Carbon Monoxide (CO)	1.84 tpy
Particulate Matter Less than 2.5 μ m (PM _{2.5})	0.24 tpy

⁽²⁾ Calculations performed using a conservative estimate of 500 hours of run-time per year at maximum output.

Conformity Threshold Rate

VOC	50 tpy
NO _x	100 tpy
SO _x	100 tpy
СО	100 tpy
PM _{2.5}	100 tpy

 $PM_{2.5}$ is some fraction of PM_{10} and to be conservative, it was assumed that PM_{10} is equal to $PM_{2.5}$ where a $PM_{2.5}$ emission factor was not available. Therefore, if the predicted PM_{10} emissions do not exceed regulatory thresholds, then neither will $PM_{2.5}$. Fugitive dust emissions are presented as PM_{10} in the emission calculations.

Diesel	Average	Loading	Emissions Factors (lbs/hr) ³					GHG ³	
Equipment	Rated HP ¹	Factors ²	СО	NOx	voc	PM ₁₀	PM _{2.5}	SOx	CO _{2e}
Asphalt Pavers	91	59%	0.26	0.58	0.05	0.05	0.05	0.05	65.20
Plate Compactors	8	43%	0.03	0.05	0.01	0.01	0.01	0.00	4.47
Concrete Pavers	130	59%	0.37	0.82	0.07	0.07	0.06	0.06	93.14
Rollers	99	59%	0.34	0.65	0.06	0.06	0.06	0.05	72.07
Scrapers	311	59%	0.86	2.01	0.12	0.12	0.12	0.15	217.15
Paving Equipment	99	59%	0.37	0.68	0.07	0.06	0.06	0.05	71.68
Signal Boards	6	43%	0.02	0.03	0.01	0.00	0.00	0.00	3.34
Trenchers	60	59%	0.28	0.42	0.05	0.05	0.05	0.03	45.08
Bore/Drill Rigs	209	43%	0.49	1.38	0.12	0.10	0.09	0.08	106.96
Excavators	183	59%	0.40	1.08	0.08	0.08	0.07	0.09	129.00
Concrete/Indust. Saw	56	59%	0.29	0.39	0.05	0.05	0.05	0.03	43.11
Cement Mixers	11	43%	0.03	0.07	0.01	0.01	0.01	0.00	5.89
Cranes	194	43%	0.25	1.01	0.07	0.05	0.05	0.07	139.10
Graders	172	59%	0.34	1.02	0.08	0.07	0.07	0.08	120.29
Off-Highway Trucks	489	59%	1.06	3.25	0.18	0.16	0.16	0.24	341.43
Crushing/Proc Equip.	127	43%	0.23	0.69	0.05	0.04	0.04	0.05	126.38
Rough Terrain Lifts	93	59%	0.40	0.64	0.07	0.07	0.06	0.05	85.99
Rubber Tired Loaders	158	59%	0.45	1.10	0.08	0.08	0.07	0.08	143.93
Tractor/Loader/Backhoe	77	21%	0.24	0.25	0.06	0.04	0.04	0.02	37.09
Crawler Tractors/Dozer	157	59%	0.42	1.03	0.07	0.07	0.07	0.08	110.23
Skid Steer Loader	42	21%	0.17	0.14	0.04	0.03	0.03	0.01	13.62
Off-Highway Tractor	214	59%	0.77	1.64	0.12	0.11	0.10	0.10	258.30
Dumpers/Tenders	23	21%	0.09	0.08	0.02	0.02	0.01	0.01	11.46
Forklifts	83	59%	0.32	0.49	0.04	0.04	0.05	0.04	61.49
Other Const. Equip.	161	59%	0.61	1.24	0.09	0.09	0.08	0.08	112.59

Construction Equipment Air Quality Emissions Factors

Note: Emissions Factors in Ib/1000 HP-hr from Table 4-1 of the Air Emissions Guide for Air Force Mobile Sources, U.S. Air Force Installations, 2020, converted to Ibs/hr using the conversion equation: Average Rated HP X Loading Factors X Emission Factors (Ib/1000 HP-hr) /1,000

Fugitive Dust from Site Preparation for DIA HQ Annex Facility

Description:	
Total disturbed area (square feet):	544,500
Total disturbed area (acres):	12.5
Assumed number of 8-hr work days:	255

Equation for Fugitive Dust Emissions (PM₁₀)

E_{PM10} (lb/yr) = 20 (lb/acre day) * GA (acres) * WD (days)

Where:

20 = factor converting acre-day to lb
GA = grading area (acres)
WD = work days

Calculation

E _{PM10} =	63,750	lb/yr
	3.19E+01	tpy

Assumptions

1. Construction and operation of an approximately 116,080 square foot operations building. The limits of disturbance (LOD) will be minimized to reduce erosion and sediment control requirements. A total of approximately 12.5 acres will be cleared and grubbed. The LOD will be cleared of all vegetation, topsoil, and unsuitable material in order to install the perimeter trail. Topsoil will be reserved for use in final grading of the site.

2. It was assumed that the majority of the site preparation work would be completed within the first 6 months of construction, approximately 127.5 hours which were rounded up for a conservative estimate.

3. It was conservatively assumed that $PM_{10} = PM_{2.5}$.

Source of Equation

Air Emissions Guide to Air Force Transitory Sources, July 2016, Section 4, Equation 4.4.

Personal Vehicle Emissions for DIA HQ Annex Facility

	Number of	Calendar	Emissions Factors (grams/mile)						
Personal Vehicles	Vehicles	Years	СО	NOx	VOC	PM ₁₀ ¹	PM _{2.5} ¹	SOx	CO _{2e}
Heavy Duty Diesel Trucks	6	2024 & 25	1.628	4.498	0.412	0.146	0.134	0.013	1483.312
Light Duty Diesel Trucks	36	2024 & 25	4.046	0.336	0.217	0.007	0.006	0.004	425.412

	Number of	Number of		Emissions (lbs/year)						
Personal Vehicles	Days	Vehicles	Miles/Day	СО	NOx	VOC	PM ₁₀ ¹	PM _{2.5} ¹	SOx	CO _{2e}
Heavy Duty Diesel Trucks	255	6	30	164.74	455.16	41.69	14.774	13.560	1.316	150100.4
Light Duty Diesel Trucks	255	36	30	2456.55	204.00	131.75	4.25	3.643	2.429	258291.6

Assumptions:

- Up to 60 contractors on-site on any one day, approximately 60% driving light duty diesel trucks.

- Assume 6 heavy duty trucks for material and equipment hauling for the duration of the project.

- The project duration is approximately 510 days, which is two years of work. The value of 510/2 (=255) is used to obtain lbs/year.

- Average round trip is 30 miles/day.

Source: Emissions factors and methodology from Air Emissions Factor Guide to Air Force Mobile Sources, June 2020, Section 5, Table 5-20.

Note: ¹ PM_{10/2.5} factors derived from combining PM combustion and fugitive emission factors on paved surfaces (EF Combustion + EF Fugitive). The PM₁₀ and PM_{2.5} fugitive emission factors for diesel trucks (both light and heavy duty) are 0.058 and 0.014 grams/mile, respectively. The calendar year 2021 combustion emission factors (grams/mile) from the Air Force guidance, Table 5-20 (On-Road Vehicle Emission Factors - 2021) are being used in the emissions calculation. The fugitive emission factors will remain unchanged.

Operational Emissions (Generator) – DIA HQ Annex Facility

Pollutant	Emissions Factor Ib/hp-hr (2)	PTE Rates ⁽⁴⁾		
		(lb/yr)	(tpy)	
DIA HQ Annex Facility Emergency Generator ⁽¹⁾				
PM/PM ₁₀ /PM _{2.5} ⁽³⁾	7.00E-04	4.69E+02	0.235	
NO _X	2.40E-02	1.61E+04	8.046	
VOC	6.42E-04	4.30E+02	0.215	
СО	5.50E-03	3.69E+03	1.844	
SO ₂	1.21E-05	8.11E+00	0.004	
CO ₂ e	1.16E+00	7.78E+05	388.890	

Construction	Usage	e Emissions (lbs)						
Equipment	(hrs)	СО	NOx	VOC	PM ₁₀	PM _{2.5}	SOx	CO _{2e} (tpy)
Asphalt Pavers (Paving)	12240	3,128.11	7,044.82	591.45	578.31	552.02	552.02	399.02
Plate Compactors (Soil/Stone	122 10	0,120.11	7,011.02	001.10	070.01	002.02	002.02	000.02
Compaction)	12240	417.69	631.16	102.32	72.42	70.74	37.90	27.36
Concrete Pavers (Large	12210		001110	102.02			01.00	21100
Concrete Placement)		0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rollers (Soil/Stone/Paving		0.00	0.00	0.00	0.00	0.00	0.00	0.00
Compaction)	12240	4,132.34	7,928.67	722.09	707.79	693.49	614.85	441.07
Scrapers (Soil relocation)	2,336	2,014.57	4,706.39	282.90	291.47	282.90	351.48	253.63
Paving Equipment	_,	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Signal Boards		0.00	0.00	0.00	0.00	0.00	0.00	0.00
Trenchers (Pipe/Utility								
Trenching)		0.00	0.00	0.00	0.00	0.00	0.00	0.00
Bore/Drill Rigs		0.00	0.00	0.00	0.00	0.00	0.00	0.00
Excavators (Dig Holes)	12240	4,955.82	13,255.17	991.16	938.30	898.66	1,110.10	789.47
Concrete/Indust. (Saw Line		.,					.,	
Cutting)		0.00	0.00	0.00	0.00	0.00	0.00	0.00
Cement Mixers (Mixes		0.00	0.00	0.00	0.00	0.00	0.00	0.00
Concrete Ingredients)	12240	415.11	914.17	104.79	78.16	75.26	49.79	36.05
Cranes	2,336	588.50	2,350.12	163.69	124.72	120.82	159.79	162.47
Graders (Push soils to make	2,000	000.00	2,000.12	100.00		120.02	100110	102.11
flat)	12240	4,136.24	12,483.26	931.59	844.64	819.80	1,018.53	736.17
Off-Highway Trucks (Huge		.,	,		01.001	0.000	.,0.000	
Dump Truck)		0.00	0.00	0.00	0.00	0.00	0.00	0.00
Crushing/Proc. Equipment		0.00	0.00	0.00	0.00	0.00	0.00	0.00
(Stone Crusher or Washer)		0.000	0.000	0.000	0.000	0.000	0.000	0.00
Rough Terrain Lifts (Either a								
man lift or material lift)	8160	3268.496	5243.026	550.719	541.764	523.855	394.010	350.84
Rubber-Tired Loaders (Dump								
Truck Loader)		0.000	0.000	0.000	0.000	0.000	0.000	0.00
Tractor/Loader/Backhoe	12240	2897.561	3089.544	676.889	467.093	449.280	199.900	226.99
Crawler Tractors/Rubber Tired								
Dozer (Farm Tractor and Dirt								
Pusher)		0.000	0.000	0.000	0.000	0.000	0.000	0.00
Skid Steer Loader (Big Tired								
Fork Lift/Bobcat)	21024	3630.752	2968.761	899.344	576.693	560.004	196.558	143.17
Off-Highway Tractor (Huge								
Equipment/Think Quarry								
Equipment)		0.000	0.000	0.000	0.000	0.000	0.000	0.00
Dumpers/Tenders (Concrete								
Delivery Vehicle)	12240	1107.894	971.328	296.187	183.861	177.358	61.484	70.14
Forklifts	12240	3896.053	5975.946	539.454	539.454	557.435	527.466	376.32
Other Construction Equipment		0.000	0.000	0.000	0.000	0.000	0.000	0.00
Site Preparation		[
(Tree/Overgrowth Removers)	-	-	-	-	63750.00	63750.00		
POVs - Contractors	-	2621.296	659.169	173.444	19.024	17.203	3.744	204.20
Total - Construction Phase (to	ons per vear)	18.61	34.11	3.51	34.86	34.77	2.64	4216.89
Concretors (Operation Phase)		1 0 1 1	9.046	0.215	0.025	0.225	0.004	200.00

1.844

8.046

0.215

0.235

0.235

0.004

388.89

Total Air Emissions – DIA HQ Annex Facility

Generators (Operation Phase) (tpy)

Estimated equipment to be used includes skid steer (bobcat), cement mixers, plate compactors, lifts, excavators, backhoes, asphalt pavers, paving equipment, graders, scrapers, cranes, and dumpers/tenders. To develop a conservative estimate, it was assumed that 9 skid steers would be used 8 hours a day for one year. Also, it was assumed 6 each of the remaining equipment would be used for 8 hours a day for two years.

Source: The above estimates were calculated using the methodology and information provided in the Air Emissions Guide for Air Force Mobile Sources, U.S. Air Force Installations, 2020, Air Emissions Guide for Air Force Transitory Sources, 2016, and Air Emissions Factor Guide to Air Force Stationary Sources, 2020.

APPENDIX C – COASTAL ZONE FEDERAL CONSISTENCY DETERMINATION

APPENDIX C Determination of Consistency with Virginia's Coastal Resources Management Program

This document provides the Commonwealth of Virginia with the Fort Belvoir Consistency Determination under the Coastal Zone Management Act Section 307(c)(1) and 15 CFR Part 930, Subpart C, for the Defense Intelligence Agency (DIA) Headquarters Annex on Fort Belvoir North Area, Fort Belvoir, Virginia. The information in this Consistency Determination is provided pursuant to 15 CFR §930.39.

This document represents an analysis of project activities in light of established Virginia Coastal Resources Management Program (CRMP) Enforceable Policies and Programs. Furthermore, submission of this consistency determination reflects the commitment of the U.S. Department of the Army (Army) to comply with those Enforceable Policies and Programs. The Proposed Action would be implemented in a manner that is consistent with the Virginia CRMP. The Army has determined that the construction and operation of the DIA HQ Annex would have a negligible impact on any land and water uses or natural resources of the Commonwealth of Virginia's coastal zone.

C1 Description of Proposed Action

The Proposed Action involves the construction of the HQs Annex building within Fort Belvoir's North Area (FBNA), in the vicinity of the National Geospatial-Intelligence Agency (NGA) complex (see Figure 2-1). The proposed headquarters annex building would be approximately 116,080 SF and would include a multi-story administrative building with offices, cubicles/workstations, publications rooms, conference rooms, break rooms, server rooms, a multi-purpose auditorium, café/cafeteria and gym/fitness center, and a utility plant, visitor control center, visitor parking, a secured employee parking structure, and a perimeter security fence, all to support stationing of approximately 650 personnel.

C2 Assessment of Probable Effects

Fort Belvoir has prepared an Environmental Assessment (EA) to evaluate the potential environmental impacts from the DIA HQ Annex in accordance with the National Environmental Policy Act (NEPA) of 1969, as amended (42 U.S. Code 4321-4347), and 32 Code of Federal Regulations (CFR) Part 651, Environmental Analysis of Army Actions.

The Army intends to obtain all applicable permits required for implementation of the Proposed Action alternative. A review of the permits and/or approvals required under the enforceable policies is being conducted. The Army has evaluated the construction of the headquarter annex building for its foreseeable effects on the following enforceable policies:

Fisheries – The Proposed Action alternative has no foreseeable impacts on fish or shellfish resources and would not affect the promotion of, or access to, commercial or recreational fisheries.

The proposed site is located approximately 4.5 miles northwest of the Potomac River and approximately 0.25 miles east of Accotink Creek. The closest water features near the proposed site are an unnamed tributary to Accotink Creek and associated riparian wetlands. Compliance with the installation's Municipal Separate Storm Sewer System (MS4) Permit and the Virginia Erosion and Sediment Control regulations would minimize the risk of sediment being transported off the site to the Potomac River Fishery. Best management practices recommended by the Virginia Departments of Conservation and Recreation (DCR) and Forestry (DOF) would be employed when necessary.

Subaqueous Lands Management – The Virginia Marine Resources Commission (VMRC), pursuant to Virginia Administrative Code (VAC) Section 28.2-1204, has jurisdiction over encroachments in, on, or over any State-owned rivers, streams and creeks. The project would have no foreseeable impacts on subaqueous resources.

Tidal and Non-tidal Wetlands Management – The Proposed Action alternative would not affect any tidal wetlands. Potential impacts to approximately 0.02 acres of non-tidal wetlands within the southwest portion of the project area would be avoided, minimized and, if necessary, mitigated in accordance with applicable Virginia laws.

Dunes Management – The Proposed Action alternative would not affect any coastal primary sand dunes.

Non-Point Source Water Pollution Control – Typically, a Proposed Action that is greater than 2,500 square feet would require an erosion and sediment control (ESC) plan and a stormwater management plan to be developed. The ESC plan would include temporary erosion and sediment control measures. The ESC plan and stormwater management plan would be prepared utilizing the requirements for water quality and quantity found in the Virginia Technical Criteria Part IIB (9VAC25-870-62 through 9VAC25-870-92). The Proposed Action disturbance of soil is approximately 7 acres, therefore an ESC plan and stormwater management plan are required. A construction general permit in accordance with 9VAC25-830-130 would also be required. Minor short-term adverse impacts would occur from the Proposed Action on surface water with regard to water quality. Appropriate temporary erosion and sediment control measures and stormwater Best Management Practices (BMP) will be employed to minimize impacts to water quality from earth disturbance and potential erosion during construction.

Point Source Water Pollution Control – The Proposed Action would not result in point source water discharge.

Shoreline Sanitation – The Proposed Action is not located on or near a shoreline. The Proposed Action alternative would therefore have no impact on shoreline sanitation.

Air Pollution Control – The proposed site is located within an ozone (O_3) non-attainment area, triggering the need to analyze emissions and determine the applicability of General Conformity Rule under the Clean Air Act (CAA). A construction emissions estimate indicates that

construction and operation activity would not generate sufficient emissions to trigger a need for a full General Conformity Analysis.

The estimated emissions associated with the construction and operation of this project are very low. The temporary impacts to air quality would be minor, short-term impacts that are not regionally or locally significant.

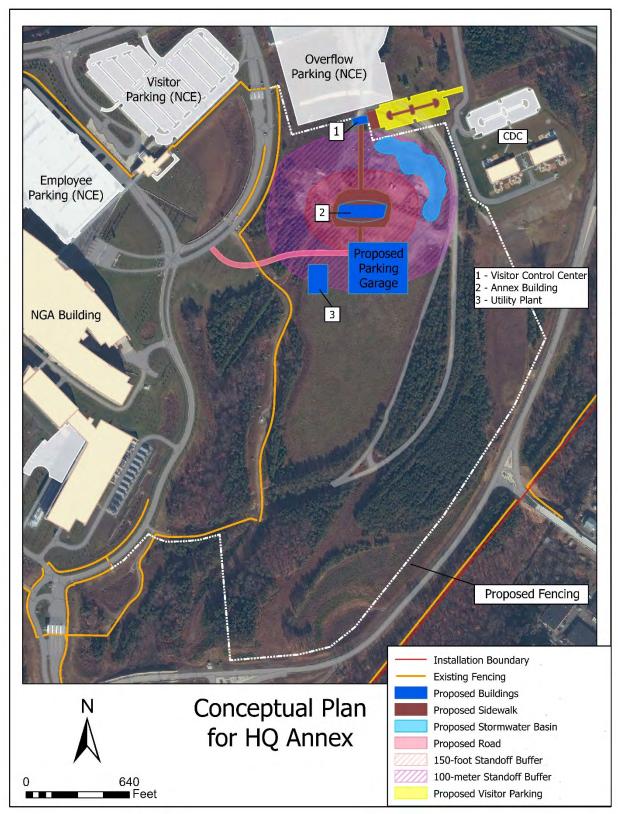
Coastal Lands Management –Resource Protection Areas (RPAs) are associated with Accotink Creek, its tributaries, and its associated tidal and non-tidal wetlands. Minor, short-term adverse impacts to the RPAs associated with an unnamed tributary to Accotink Creek and the adjacent riparian, non-tidal wetlands are anticipated in the project area (Figure 3-3). Avoidance and minimization of impacts to this area will be fully considered as the project design progresses. Any unavoidable impacts will be addressed through applicable permitting pursuant to Section 404 of the Clean Water Act and the Virginia Water Protection Permit Program (9 Virginia Administrative Code [VAC] 25-210-10 et seq.). Appropriate temporary erosion and sediment control measures and stormwater BMPs will be employed at the construction site to minimize downstream impacts to Accotink Creek from earth disturbance associated with construction activities.

C3 Summary of Findings

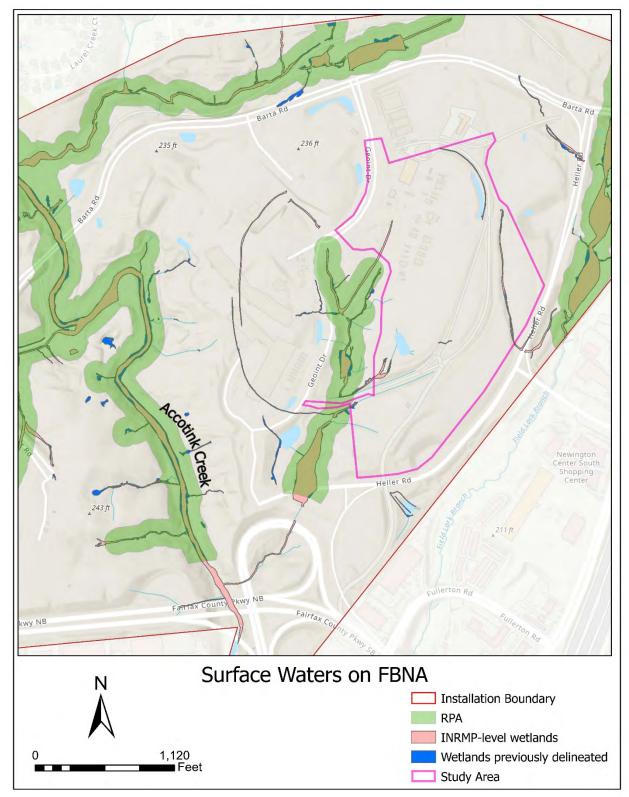
Based on the above analysis, which is elaborated on in the EA, Fort Belvoir personnel would: (1) ensure that the construction contractor uses and maintains appropriate temporary erosion and sediment controls; and (2) obtain the requisite permits and approvals. Fort Belvoir finds that the proposed HQs Annex construction is fully consistent to the maximum extent practicable with the federally approved enforceable provisions of Virginia CRMP, pursuant to the Coastal Zone Management Act of 1972, as amended and in accordance with 15 CFR 930.30.

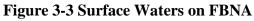
Pursuant to 15 CFR Part 930.41, the Virginia Coastal Resources Management Program has 60 days from receipt of this letter in which to concur with or object to this Consistency Determination, or to request an extension, in writing, under 15 CFR Part 930.41(b). Virginia's concurrence will be presumed if its response is not received by Fort Belvoir on the 60th day from receipt of this determination. The state's response should be sent to U.S. Army Garrison Fort Belvoir, 9430 Jackson Loop, Suite 200, Fort Belvoir, VA 22060-5116.

Joshua P. SeGraves Colonel, US Army Commanding









APPENDIX D – TRAFFIC IMPACT STUDY





US Army Corps of Engineers Baltimore District

Traffic Impact Study to Support National Environmental Policy Act Documentation for DIA HQ Annex

REVISED DRAFT

Fort Belvoir, Virginia

Contract No. W912DR-20-D-0010 Task Order W912DR21F0071



Traffic Impact Study to Support National Environmental Policy Act Documentation for DIA HQ Annex

Fort Belvoir, Virginia

Prepared for: US Army Corps of Engineers Baltimore District

Under contract with: U.S. Army Corps of Engineers

Prepared by: HDR Tehama JV 1600 Genessee St Ste 754 Kansas City, MO 64102-1064

Our Reference: Contract W912DR-20-D-0010 Task Order W912DR21F0071 Tehama Project F0133.01

Date: 27 May 2021

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Brad Loomis, PE, PTOE Project Manager



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APPENDICES

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- Appendix C Synchro Files
- Appendix D Gate SMART Analysis



ACRONYMS AND ABBREVIATIONS

ATR	Automated Traffic Recorder
DIA	Defense Intelligence Agency
EBL	Eastbound Left
EBR	Eastbound Right
EBT	Eastbound Thru
Ex	Existing
Ft	Foot
HCM	Highway Capacity Manual
HQ	Headquarters
LOS	Level of Service
NBL	Northbound Left
NBR	Northbound Right
NBT	Northbound Thru
NGA	National Geospatial-Intelligence Agency
PE	Professional Engineer
S	Seconds
SBL	Southbound Left
SBR	Southbound Right
SBT	Southbound Thru
TIS	Traffic Impact Study
ТМС	Turning Movement Count
USACE	United States Army Corps of Engineers
veh	Vehicle
v/c	volume to capacity
WBL	Westbound Left
WBR	Westbound Right
WBT	Westbound Thru



EXECUTIVE SUMMARY

This Traffic Impact Study (TIS) study presents the traffic operational analysis results at two (2) alternative locations in order to accommodate the proposed construction and operation of a new 155,000 square foot Defense Intelligence Agency (DIA) Headquarters (HQ) Annex building with an associated parking structure. Approximately 650 additional personnel will be employed at the new site. The traffic study focused on roadways providing access to the two alternative sites:

- Alternative 1 on Fort Belvoir North Area (FBNA)
- Alternative 2 within the 1400 Area East district of the Fort Belvoir Main Post.

Traffic data was collected at sixteen (16) locations to support the development of the traffic impact study (TIS). Both turning movement counts (TMCs) at the major intersections (10 locations) and automatic traffic recorders (ATRs) at select ramps and entrance/exit lanes (6 locations) were collected. In addition, security gate entry data was requested for the time period prior to decreased site reporting conditions (early 2020) and January of 2021. This gate entry data was used to estimate and generate Adjusted No-Build volumes for pre-COVID conditions.

Level of Service Standards

Level of service is a qualitative measure describing operational traffic conditions, and the perception of these conditions by drivers or passengers. These conditions include factors such as speed, delay, travel time, freedom to maneuver, traffic interruptions, comfort, convenience, and safety. Levels of service are given letter designations from A to F, with LOS A representing the best operating conditions (free flow, little delay) and LOS F, the worst (congestion, long delays). Generally, LOS A and B are considered high level of service, LOS C and D are considered moderate, and LOS E and F are considered low. In general, the standards are LOS D in urban areas and LOS C in rural areas.

Table ES 1: Existing (adjusted) Intersection Operational Analysis – FBNA										
	O an aliand	AM	PM	AM	PM					
Intersection	Signalized (Y/N)	Delay	(s/veh)	LOS						
Barta Road / FBNA Facilities Access	Y	1.7	1.1	A	A					
West Gate Entrance	Ν	-	-	A	A					
Barta Road / Parking Garage Exit	Y	0.0	10.4	A	В					
Barta Road / Main Guest Access	Ν	-	-	A	A					
Barta Road / GEOINT Drive	Y	5.5	13.3	A	В					
Barta Road / Heller Road	Y	9.8	0.6	A	A					
Barta Road / Backlick Road	Y	7.9	20.1	A	С					
Heller Road / HOV Entrance Ramp	Ν	-	-	A	A					
95 Exit Ramp / Heller Road	Ν	-	-	A	A					
South Gate Entrance	Ν	-	-	A	A					

The results of the operational analysis using Synchro are provided below.



Table ES 2: Existing (adjusted) Intersection Operational Analysis – Fort Belvoir									
	Signalized	AM	PM	AM	PM				
Intersection	(Y/N)	Delay (s/veh)		LC	DS				
Richmond Highway (Hwy 1) / Pohick Road	Y	25.1	36.4	С	D				
Gunston Road / 1st Street	Y	11.0	33.6	В	С				
Gunston Road / 3rd Street	Y	22.2	14.1	С	В				
Pohick Road / Theote Road	Y	11.4	12.2	В	В				
Gunston Road / Pohick Road / 12th Street	Y	19.9	24.5	В	С				
Richmond Highway (Hwy 1) / Belvoir Road	Y	42.8	78.7	D	Е				
Belvoir Road / DeWitt Loop (N)	N	19.4	4.3	С	А				
Gunston Road / Meade Road	Y	3.0	55.6	А	Е				

As shown in the table above, all intersections are operating at LOS C or better.

As shown in the table above, all intersections are operating at LOS D or better.

The Build scenario with 650 additional personnel was analyzed using current roadway geometrics and intersection control. No background growth was assumed for the construction year. This 'build' scenario was compared to adjusted existing 2021 volumes. A sensitivity analysis was performed using 1,000 new personnel (in lieu of 650) to determine any effects possible with increased volumes over the anticipated traffic generated from the annex construction.

Trip generation was based on one additional employee resulting in one AM trip and one PM trip and assumed all occur during the peak hours. This approach assumes no carpool trips for estimated added employees. Site entry distribution was based on traffic counts performed March 2021.

Field personnel observed the Meade and Belvoir gates opened at varying times. For modeling purposes, the analyses assumed AM entering traffic through the Belvoir Road gate and the Pohick Road gate. It also assumes that PM exiting traffic leaves the site back through the Pohick Road gate and the Meade gate. Individual analyses at critical intersections were performed to determine capacity of alternate entrance and exit routes.



The following potential operational impacts were identified.

Table ES 3: Build Condition (2021 adjusted) Intersection Operational Analysis –Alternative 1										
	þé	650	Added	Persor	nnel	1000 Added Personnel				
Intersection	Signalized (Y/N)	AM	PM	AM	PM	AM	PM	AM	PM	
	Sig (Delay	(s/veh)	LOS		Delay (s/veh)		LOS		
Barta Road / FBNA Facilities Access	Y	2.0	1.3	A	А	2.2	1.5	A	A	
West Gate Entrance	Ν	-	-	А	А	-	-	А	А	
Barta Road / Parking Garage Exit	Y	0.1	10.0	A	А	0.1	10.0	А	A	
Barta Road / Main Guest Access	Ν		-	А	A	-	-	А	А	
Barta Road / GEOINT Drive	Y	8.7	21.5	A	С	11.1	67.2	В	Е	
Barta Road / Heller Road	Y	11.5	3.1	В	А	12.2	2.9	В	А	
Barta Road / Backlick Road	Y	8.0	21.5	A	С	20.4	20.9	С	С	
Heller Road / HOV Entrance Ramp	N	-	-	А	A	-	-	A	A	
95 Exit Ramp / Heller Road	Z		-	A	A	-	-	A	A	
South Gate Entrance	Z		-	А	A	-	-	A	А	

• Alternative 1 FBNA, Adjusted Build

- Build with 650 Additional Personnel All intersection (AM and PM) operate at LOS B or better with the exception of the intersection of Barta Road /Geoint Drive (LOS C during the PM peak hour) and Barta Road / Backlick Road (LOS C during the PM peak hour).
- Build with 1000 Additional Personnel All intersection (AM and PM) operate at LOS C or better with the exception of the intersection of Barta Road /Geoint Drive (LOS E during the PM peak hour).



Table ES 4: Build Condition (2021 adjusted) Intersection Operational Analysis – Alternative 2										
	þe	650	Added P	Person	nel	1000 Added Personnel				
Intersection	Signalized (Y/N)	AM	PM	AM	PM	AM	PM	AM	PM	
	Sigi ()	Delay (s/veh)		LOS		Delay (s/veh)		LOS		
Richmond Parkway (US 1) / Pohick Road	Y	36.2	41.1	D	D	51.1	46.2	D	D	
Gunston Road / 1 st Street	Y	11.3	101.5	В	F	11.2	155.7	В	F	
Gunston Road / 3 rd Street	Y	11.9	18.4	В	В	9.8	22.6	A	С	
Pohick road / Thoete Road	Y	18.8	11.5	В	В	32.9	11.8	С	В	
Gunston Road / Pohick Road	Y	23.3	44.0	С	D	36.7	71.9	D	E	
Richmond Parkway (US 1) / Belvoir Road	Y	63.5	124.5	E	F	82.6	153.2	F	F	
Belvoir Road / DeWitt Loop (roundabout)	N	50.3 (98.5 SB RT)	4.3	F	A	87.1 (178.4 SB RT)	4.3	F	A	
Gunston Road / Meade Road	Y	9.4	112.4	А	F	12.4	167.5	В	F	

Alternative 2 Fort Belvoir, Adjusted Build

- Build with 650 Additional Personnel Several intersections fail in the AM and PM peak hour. Belvoir Road / DeWitt Loop Roundabout (LOS F during the AM peak hour). Gunston Road / 1st Street and Richmond Parkway (US 1) / Belvoir Road (LOS F during the PM peak hour).
- Build with 1000 Additional Personnel Several intersections fail in the AM and PM peak hour. Belvoir Road / DeWitt Loop Roundabout and Richmond Parkway (US 1) / Belvoir Road (LOS F during the AM peak hour). Gunston Road / 1st Street and Richmond Parkway (US 1) / Belvoir Road (LOS F during the PM peak hour).

The Alternative 1 FBNA location is separated from heavy traffic and does not share significant intersections with arterials like Alternative 2 Fort Belvoir does with Richmond Parkway (US 1). In addition, the roundabout



at Belvoir Road / DeWitt Loop is currently near capacity for a dual-lane roundabout and will exceed capacity with the additional volumes.

Based on the traffic operational results of both alternates, this study concludes that Alternative 1 FBNA can accommodate the existing site traffic and the anticipated additional traffic generated by the annex. There also appears to be excess capacity if additional site traffic generators are proposed.

Entrance Control Facility Impacts

Each gate was reviewed relative to impacts from the proposed added volumes. Gate SMART Evaluator -Quick Calculator was used to determine potential staffing and lane needs. Based on 650 added vehicles to the AM peak hour at each gate, the following possible impacts were determined. All gates have excess number of receiving lanes.

Alternative 1

- All Gates
 - o No additional manpower or lanes required.
 - Minor additional vehicle queueing.

Alternative 2

- Meade Gate
 - No additional manpower or lanes required.
- Belvoir Gate
 - o No additional lanes required.
 - 2 additional staff required for Handheld Tandem processing, or 1 additional staff for No Arms AIE processing.
 - o Minor additional vehicle queueing.
 - o Minor additional delays.
- Pohick Gate
 - No additional lanes required.
 - o 1 additional staff required for Handheld Single processing.
 - Minor additional vehicle queueing.
 - Minor additional delays.

Site Parking

Proposed improvements would include a parking facility at either alternative location. The facility would need to accommodate a minimum of 650 spaces with appropriate number of accessible parking stalls. Limited area is available at Alternative 2 and may require a multi-story parking structure. Additionally, space available between the structure and roadway system is limited and may create turning queues into the new facility. Alternative 1 has adequate available space for additional surface lots and new HQ Annex. Available space is present to create new access roads into the new facility.

Indirect Effects

Increased vehicle traffic may affect some intersections outside of the study area. The project traffic traveling through those intersections is expected to result in a small (less than 1 percent) increase in traffic at those



intersections. The project trips associated with this project are not expected to affect the level of service of those intersections significantly.

Proposed Design Features Intended to Reduce Impacts

From the analyses results, possible roadway and intersection improvements were identified to mitigate operational impacts that were degraded to LOS E or LOS F. Potential mitigation is discussed below.

Alternative 1 – 1000 Additional Personnel

- PM NB Geoint Drive to both EB & WB Barta Road
 - Mitigation Signal optimization

Alternative 2 – 650 Additional Personnel

- AM WB Richmond Parkway to SB Belvoir Road (dual-lane left turn)
 - Mitigation Signal optimization and construct an additional SB merge lane for EB Richmond Parkway right turns.
- PM SB Meade Road to EB / WB Richmond Parkway
 - Mitigation Provide an additional 200-250 foot left and right turn lane to provide for dual lefts and a designated right turn lane.
- AM SB Belvoir Road to WB DeWitt Loop (roundabout)
 - Mitigation DETERMINATION IN PROGRESS
- PM NB Gunston Road to EB Meade Road
 - Mitigation Signal optimization and construct signalized NB dual right turn lanes onto Meade Road.

Alternative 2 – 1000 Additional Personnel

- AM WB Richmond Parkway to SB Belvoir Road (dual-lane left turn)
 - Mitigation Signal optimization and construct an additional SB merge lane for EB Richmond Parkway right turns.
- PM SB Meade Road to EB / WB Richmond Parkway
 - Mitigation Provide an additional 200-250 foot left and right turn lane to provide for dual lefts and a designated right turn lane.
- AM SB Belvoir Road to WB DeWitt Loop (roundabout)
 - Mitigation DETERMINATION IN PROGRESS
- PM NB Gunston Road to EB Meade Road
 - Mitigation Signal optimization and construct signalized NB dual right turn lanes onto Meade Road.

This study recommends that Alternative 1 (FBNA) be selected, as there is more available capacity to accommodate increased traffic volumes.



1 INTRODUCTION

1.1 Introduction

Tehama HDR JV was retained by US Army Corps of Engineers (USACE) to evaluate the potential traffic impacts resulting from the proposed construction and operation of a new approximately 155,000 square foot Defense Intelligence Agency (DIA) Headquarters (HQ) Annex building with an associated parking structure to accommodate approximately 650 personnel at Fort Belvoir, Virginia. The traffic study will focus on roadways providing access to the two alternative sites.

Various Measures of Effectiveness (MOEs), such as intersection delay and Level of Service (LOS) will be presented in this study. The analysis results will be determined using the definitions and methodology outlined in the 6th edition of the Highway Capacity Manual (HCM). The Synchro 11 software module will be used to evaluate the signalized and unsignalized intersections.

1.2 Analyses Years

The traffic analyses were performed during morning (AM) and afternoon (PM) weekday peak hours for the following analysis years:

- Existing Year (2021, As Counted)
- Existing Year (2021) Adjusted
 - Adjusted volumes are based on total inbound base gate counts from January 2020 (pre COVID) and January 2021. Volumes were increased by 40% to account for the 35-40% reduction in overall base traffic experienced. Volumes along Richmond Parkway (US 1) were not inflated.
- Build Condition
 - o Build with 650 additional personnel reporting to new annex with adjusted 2021 traffic.
 - Build with 1000 additional personnel reporting to new annex (sensitivity analysis) with adjusted 2021 traffic.

1.3 Study Area / Project

The study area consists of two (2) separate locations within Ft. Belvoir, Virginia. The Regions of Influence (study areas) are shown on Figure 1-1 and are as described below. **Alternative 1** - Fort Belvoir North Area (FBNA) in the vicinity of the National Geospatial-Intelligence Agency (NGA)



• Alternative 2 - southeast corner of 1st Street and Gunston Road within the 1400 Area East district of Main Post

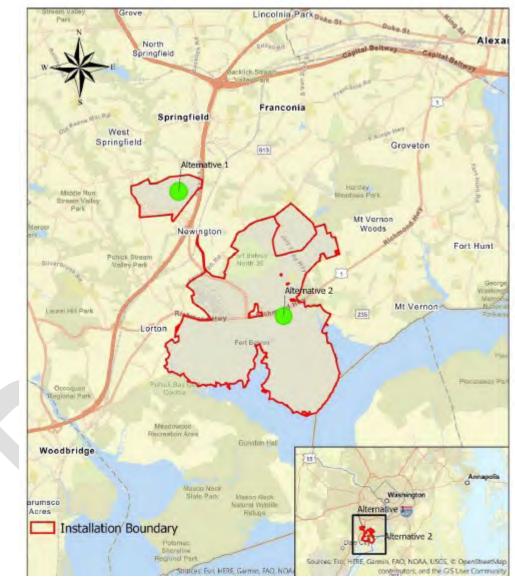


Figure 1-1: Regions of Influence – Alternatives 1 and 2



2 DATA COLLECTION

2.1 Traffic Volume Collection

Traffic data was collected at sixteen (16) locations to support the development of the traffic impact study (TIS). Both turning movement counts (TMCs) at the major intersections (10 locations) and automatic traffic recorders counts (ATRs) at select ramps/gates (6 locations) were collected. The turning movement counts were completed using JAMAR boards. These are industry standard counting equipment which are versatile in acquiring data at signalized, unsignalized and roundabout intersections. PICO tubes were used for the volume data at ATR identified locations. The tubes allowed the acquisition of 24-hour counts which helped identify peak hours.

Turning Movement Counts (TMCs) and roadway volume counts were conducted at the locations shown in Figure 2-1 and Figure 2-2. The locations for the roadways and intersection counts are listed below in Table 2-1 and Table 2-2. Figure 2-3 and Figure 2-4 present diagrams of the volumes counted at specific intersections within the study areas (refer to Appendix B for the original count data). The counts were collected during the AM and PM peak hours over a three-day period of a typical Tuesday, Wednesday, and Thursday. During project discussions, NGA noted that focus may be given to certain times based on employee work schedules. Based on this input, it was assumed the AM peak occurs between 6-9 AM and the PM peak occurs between 3-6 PM. The turning movement counts were collected in 15-minute periods and include classification of passenger vehicles, trucks (vehicles with 3 or more axles), and bicycles/pedestrians. This information was input into the existing conditions model.

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Figure 2-1: Count Locations for Existing Conditions – Alternative 1 (FBNA)

Table 2-1 : Traffic Volume Count Locations – Alternative 1						
Count ID	Intersection	Count Date	Туре			
1	Barta Road with Geoint Drive	2021-03-23	TMC (JAMAR)			
2	Barta Road with Heller Road	2021-03-23	TMC (JAMAR)			
3	Barta Road with Backlick Road	2021-03-23	TMC (JAMAR)			
4	Barta Road / Fairfax County Parkway (VA 286) NB Ramps	2021-03-24	TMC (JAMAR)			
5	Barta Road / Fairfax County Parkway (VA 286) SB Ramps	2021-03-24	TMC (JAMAR)			
6	Heller Road with I-95 NB/I-95 SB Express Lane	2021-03-23	ATR (Pico)			
7	Heller Road with I-95 SB	2021-03-23	ATR (Pico)			
8a	Heller Road with NGA South Gate (inbound)	2021-03-23	ATR (Pico)			
8b	Heller Road with NGA South Gate (outbound)	2021-03-24	ATR (Pico)			
9	Barta Road at NGA West Gate Entry	2021-03-24	ATR (Pico)			
10	Barta Road at NGA West Gate Exit	2021-03-24	ATR (Pico)			
11	GEOINT Drive Visitor Parking Lot Access Lane	2021-03-24	ATR (Pico)			

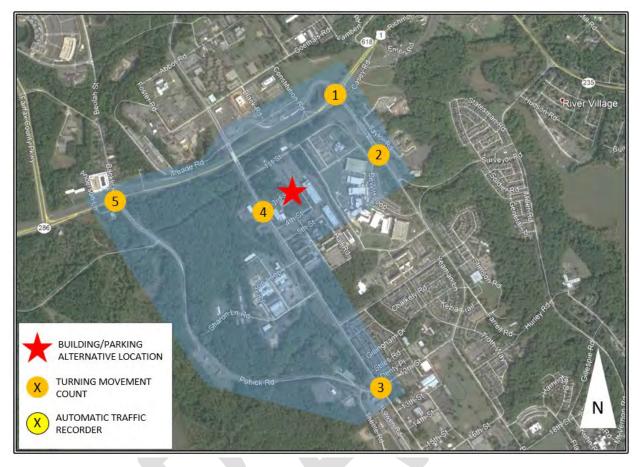


Figure 2-2: Count Locations for Existing Conditions – Alternative 2 (Fort Belvoir)

	Table 2-2 : Traffic Volume Count Locations – Alternative 2							
Count ID	Intersection	Count Date	Direction					
1	US 1 with Meade Road/Belvoir Road	2016-02-24	TMC (JAMAR)					
2	Belvoir Road with Dewitt Loop/Taylor Road	2016-02-25	TMC (JAMAR)					
3	Gunston Road with Pohick Road/12th Street	2016-02-25	TMC (JAMAR).					
4	Gunston Road with 3rd Street	2016-02-25	TMC (JAMAR)					
5	US 1 with Pohick Road	2016-02-24	TMC (JAMAR)					

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24-Hour Counts: 24-Hour Counts were taken on either Tuesday, Wednesday, or Thursday at 6 primary locations (6 - 11) identified in Figure 2-1: The average daily traffic (ADT) measured in vehicles per day (vpd) is shown in Table 2-3.

	Table 2-3 : 24-Hour Tube (ATR) Count ADT (2021)						
Count ID	Roadway	Description	Direction	ADT (vpd)			
6	HOV Entrance Lane	Traffic From Heller Road to I-95	EB	4697			
7a	I-95 Exit Ramp	Exit Ramp to Heller Road (RT)	EB	2234			
7b	I-95 Exit Ramp	Exit Ramp to Heller Road (LT)	WB	1792			
8a	Heller Road	South Gate (Outbound)	SB	188			
8b	Heller Road	South Gate (Inbound)	NB	2632			
9	West Gate	West Gate Entrance Traffic	ЕВ	5788			
10	Exit Gate (Onto Barta Road)	Parking Garage Exit	NB	4180			
11	GEOINT Drive	Visitor Parking Lot Access Lane	SB	1344			

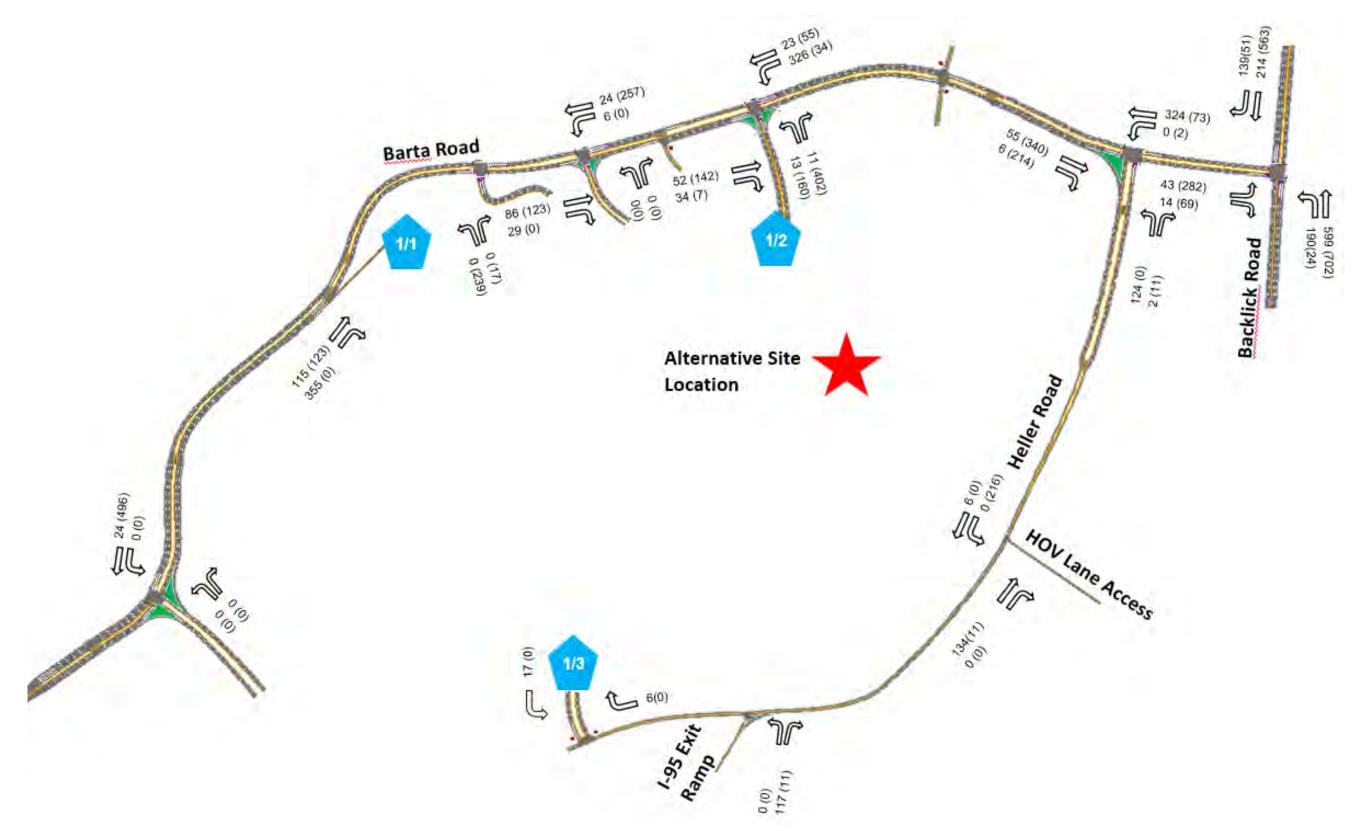


2.2 Existing Year (2021) Traffic Volumes

A review of the traffic count data indicates that the weekday morning and afternoon peak hours are not consistent among the study intersections in both Alternative Locations. The respective peak hour for each intersection is shown in Table 2-4.

Table 2-4 : Peak Hours for Existing Counts (2021)					
		Peak Hour			
Count ID	ID Location		РМ		
Alternative ²	1 – FBNA				
1	Barta Road with Geoint Drive	6:45–7:45	4:30–5:30		
2	Barta Road with Heller Road	7:15-8:15	3:45-4:45		
3	Barta Road with Backlick Road	7:00-8:00	4:00-5:00		
4-5	Barta Road with Fairfax County Parkway (VA 286) NB Ramps (WB Barta Road)	6:45–7:45	3:45-4:45		
6	Heller Road with I-95 NB/I-95 SB Express Lane	12:00-1:00	5:45-6:45		
7	Heller Road with I-95 SB	7:45-8:45	3:00-4:00		
8	Heller Road with NGA South Gate (inbound)	7:30-8:30	8:45-9:45		
9	Barta Road at NGA West Gate Entry	9:30-10:30	-		
10	Barta Road at NGA West Gate Exit	-	5:45-6:45		
11	GEOINT Drive Visitor Parking Lot Access Lane	7:15-8:15	2:45-3:45		
Alternative 2	2 – Fort Belvoir				
1	US 1 with Meade Road/Belvoir Road	7:30-8:30	3:30-4:30		
2	Belvoir Road with Dewitt Loop/Taylor Road	7:00-8:00	3:45-4:45		
3	Gunston Road with Pohick Road/12th Street	7:45-8:45	3:30-4:30		
4	Gunston Road with 3rd Street	7:30-8:30	3:45-4:45		
5	US 1 with Pohick Road	7:15-8:15	3:30-4:30		

Figures 2-3 through **Figure 2-6** show the Existing (2021) morning (AM) and afternoon (PM) peak hour traffic volumes for both Alternatives, both *as counted* and *adjusted*.





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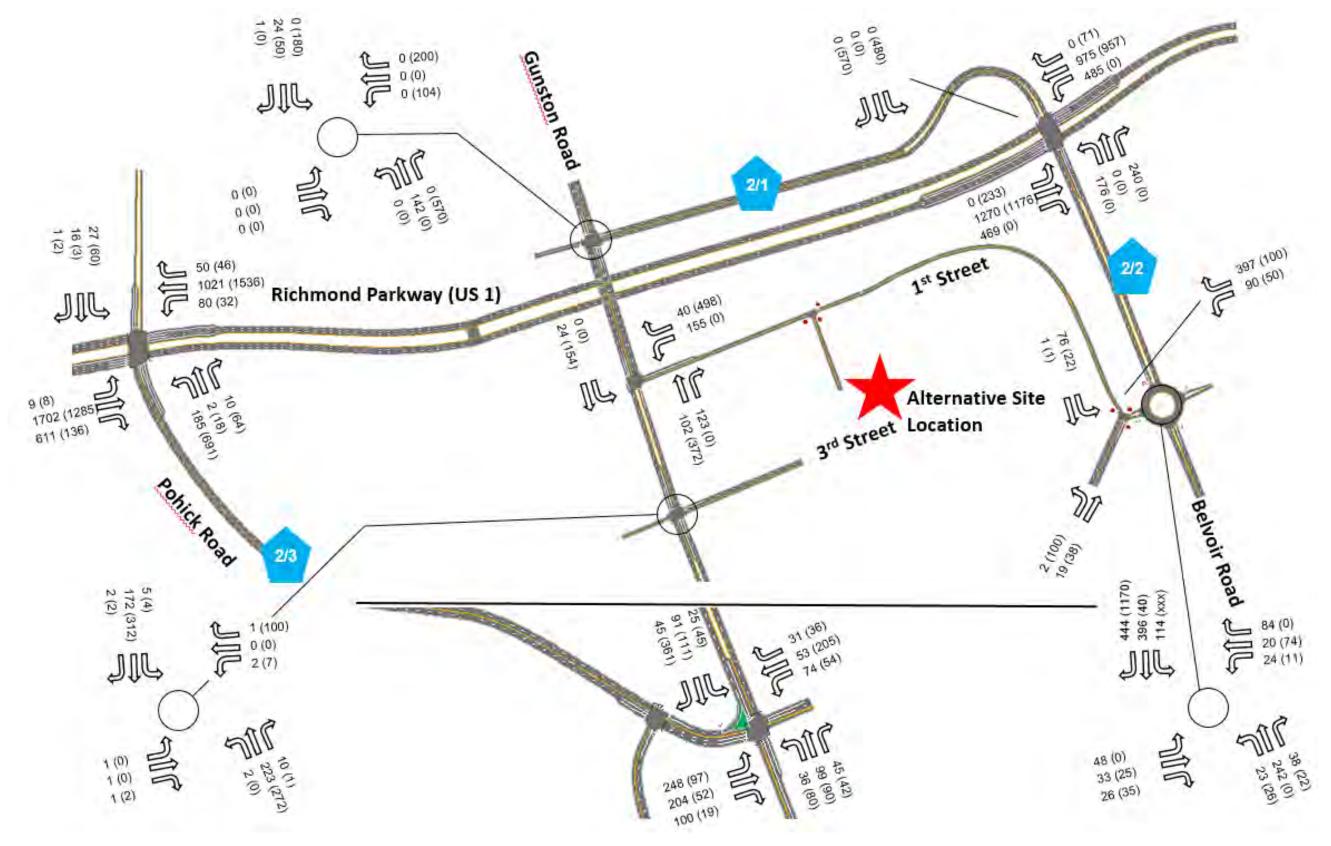


Figure 2-5: AM (PM) Peak Hour Turning Movement Volumes for Existing Conditions (2021, as counted) – Alternative 2 (Fort Belvoir)







Figure 2-6: AM (PM) Peak Hour Turning Movement Volumes for Existing Conditions (2021, adjusted) – Alternative 2 (Fort Belvoir)





2.3 Traffic Signal Timing Data

Signal timing was not provided by the agencies. Timing was observed during traffic counts and noted. Total cycle length, protected / permissive movements, and phase lengths were collected and modelled within Synchro 11. Where timing and cycle length information was not recorded in the field, Synchro "optimized" conditions were used in the model. See Appendix A for field notes taken.



3 OPERATIONAL ANALYSES

3.1 Methodology

This study includes the operational analysis of the existing year 2021 conditions, future 2021 conditions with 650 new staff, and sensitivity analysis scenarios. The future year analyses were performed for only the Build condition. The operating condition of the study intersections were evaluated using the Synchro/SimTraffic micro- simulation software.

Different Measures of Effectiveness (MOEs) were evaluated while performing the operational condition. The intersection delay and Level of Service (LOS) were evaluated and presented in this study for the existing, future year build traffic conditions.

The Synchro 11 traffic simulation software program was used to perform intersection and arterial operational analyses. This software provides industry standard analysis for signalized and roundabout intersections. The study area consists of both unsignalized and signalized intersections. The analysis methodologies are described in the following sections.

3.2 Description of Level of Service Grades (LOS)

Based on delay or density values, a "grade" or level of service (LOS) ranging from LOS A, the best, to LOS F, the worst are assigned. The Highway Capacity Manual (HCM) describes service as the following:

LOS A - free flow

Traffic flows at or above the posted speed limit and motorists have complete mobility between lanes. The average spacing between vehicles is about 550 ft (167 m) or 27 car lengths. Motorists have a high level of physical and psychological comfort. The effects of incidents or point breakdowns are easily absorbed. LOS A generally occurs late at night in urban areas and frequently in rural areas.

LOS B - reasonably free flow

LOS A speeds are maintained, maneuverability within the traffic stream is slightly restricted. The lowest average vehicle spacing is about 330 ft(100 m) or 16 car lengths. Motorists still have a high level of physical and psychological comfort.

LOS C - stable flow, at or near free flow



Ability to maneuver through lanes is noticeably restricted and lane changes require more driver awareness. Minimum vehicle spacing is about 220 ft (67 m) or 11 car lengths. Most experienced drivers are comfortable, roads remain safely below but efficiently close to capacity, and posted speed is maintained. Minor incidents may still have no effect but localized service will have noticeable effects and traffic delays will form behind the incident. This is the target LOS for some urban and most rural highways.

LOS D - approaching unstable flow

Speeds slightly decrease as traffic volume slightly increase. Freedom to maneuver within the traffic stream is much more limited and driver comfort levels decrease. Vehicles are spaced about 160 ft (50m) or 8 car lengths. Minor incidents are expected to create delays. Examples are a busy shopping corridor in the middle of a weekday, or a functional urban highway during commuting hours. It is a common goal for urban streets during peak hours, as attaining LOS C would require prohibitive cost and societal impact in bypass roads and lane additions.

LOS E - unstable flow, operating at capacity

Flow becomes irregular and speed varies rapidly because there are virtually no usable gaps to maneuver in the traffic stream and speeds rarely reach the posted limit. Vehicle spacing is about 6 car lengths, but speeds are still at or above 50 mi/h(80 km/h). Any disruption to traffic flow, such as merging ramp traffic or lane changes, will create a shock wave affecting traffic upstream. Any incident will create serious delays. Drivers' level of comfort become poor. This is a common standard in larger urban areas, where some roadway congestion is inevitable.

LOS F - forced or breakdown flow

Every vehicle moves in lockstep with the vehicle in front of it, with frequent slowing required. Travel time cannot be predicted, with generally more demand than capacity. A road in a constant traffic jam is at this LOS, because LOS is an average or typical service rather than a constant state. For example, a highway might be at LOS D for the AM peak hour, but have traffic consistent with LOS C some days, LOS E or F others, and come to a halt once every few weeks.

Figure 3-1 shows the roadway traffic condition corresponding to the LOS letter grades. The goal of this study is to ensure study intersections would operate at an acceptable LOS D or better in the future build year.

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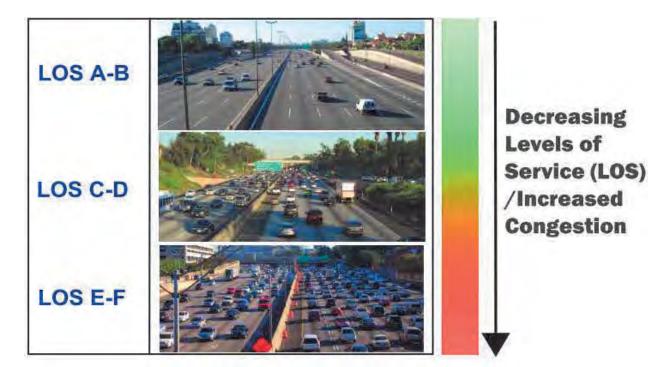


Figure 3-1: Level of Service (LOS) Conditions

3.3 Analysis Methodology for STOP and Roundabout Controlled Intersections

The capacity analysis procedures provide an 'approach delay' for the stop sign controlled approaches to the unsignalized intersections. The intersection LOS "grades" for two-way stop-controlled intersections are as follows in Table 3-1:

Table 3-1: STOP Controlled Intersection Level of Service (LOS) Criteria				
Level of Service (LOS) Average Control Delay (sec/veh)				
A	< 10			
В	10 to 15			
С	15 to 25			
D	25 to 35			
E	35 to 50			
F	> 50			



3.4 Analysis Methodology for SIGNAL Controlled Intersections

At a signalized intersection, the total delay is dependent upon a number of factors, including when a driver approaches the intersection, the driver's position in the queue and the traffic signal cycle length and green times. The control delay for a signalized intersection is determined for each lane group and aggregated for each approach and for the intersection as a whole.

Table 3-2 below presents the LOS criteria for signalized intersections (based on Highway Capacity Manual), which is directly related to the overall intersection control delay value. The intersection LOS for signalized intersections are as follows:

Table 3-2 : SIGNAL Controlled Intersection Level of Service (LOS) Criteria				
Level of Service (LOS) Average Control Delay (sec/ve				
А	< 10			
В	10 to 20			
С	20 to 35			
D	35 to 55			
E 55 to 80				
F	> 80			

Source: Highway Capacity Manual

The operational analyses at each study area intersection, for each individual alternative, were evaluated based on these signalized intersection delay thresholds.



4 EXISTING CONDITIONS

4.1 Existing Geometric Configuration and Intersections

The study areas have been defined to include site access points for both alternatives.



Figure 4-1: Analyzed Intersections within the Alternative 1 Study Area (FBNA)



Figure 4-2: Analyzed Intersections within the Alternative 2 Study Area (Fort Belvoir)



Figure 4-3 presents the lane configurations for intersections within the study area under existing conditions for Alternative 1, Fort Belvoir North Area. Figure 4-4 presents the lane configurations for intersections within the study area under existing conditions for Alternative 2, Fort Belvoir.

Existing conditions in this report refer to the current conditions as of March 2021. A site visit was conducted in March 2021 to document the lane configurations in place at that time.

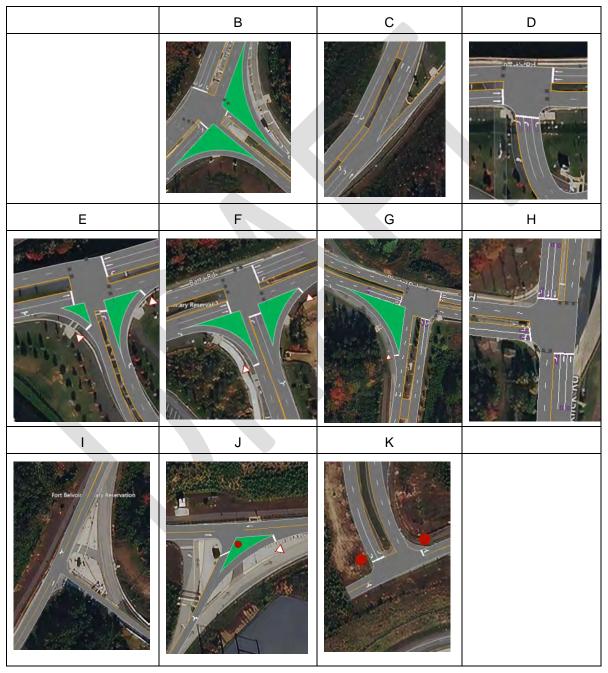


Figure 4-3: Existing Lane Configurations, Fort Belvoir North Area

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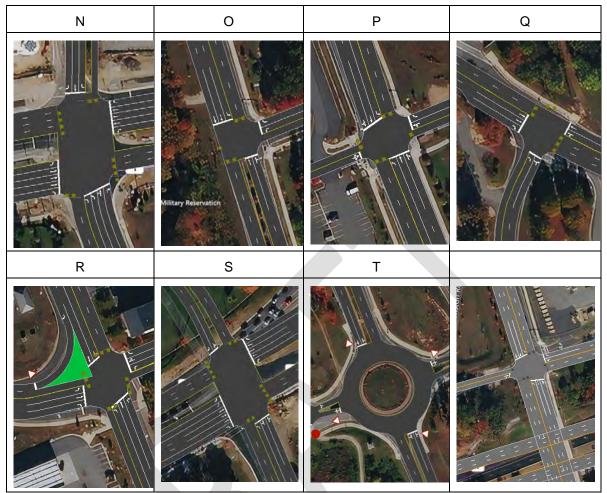


Figure 4-4: Existing Lane Configurations, Fort Belvoir

4.2 Existing Gate Access4.2.1 Existing Gate Access Location

During field visits and data acquisition tasks, it was observed that gate accessibility varied during the week. Alternative 1 (FBNA) has three (3) main points of access and Alternative 2 (Fort Belvoir) has three (3) main points of access. Figure 4-5 and Figure 4-6 depict gated access locations.

Alternative 1 FBNA

Alternative 2 Fort Belvoir

- West Gate (1/1)
- North Gate (GEOINT Drive) (1/2)
- South Gate (Heller Road) (1/3)
- Lieber Gate (Meade Road) (2/1)
- Pence Gate (Belvoir Drive) (2/2)
- Tulley Gate (Pohick Road) (2/3)

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Figure 4-5: Gate Access Locations Alternative 1 Study Area (FBNA)



Figure 4-6: Gate Access Locations Alternative 2 Study Area (Fort Belvoir)



4.2.2 Existing Gate Access Volumes

Estimated percentages of entering and exiting traffic was calculated using the March 2021 field counts. Table 4-1 and Table 4-2 summarize entering and exiting vehicle percentages for each location during peak hours. It was noted that the Alternative 1 - South Gate traffic occurred during off peak times. However, the West Gate off Barta Road does not have direct access to the Alternative 1 site location. Therefore, this study assumes that the South Gate will provide an alternative access point. Based on field observations in March 2021, Alternative 2 - Meade Road Gate (Lieber) was used for exiting (PM) traffic only and the Belvoir Road Gate (Tulley) was utilized only for entering (AM) traffic during field counts. Estimated peak hour traffic will be routed through alternate gates (Meade vs. Belvoir) for optional gate access conditions. The percentage shown below will be used to distribute expected new trips generated by the new facility for normal conditions.

Table 4-1: Modeled Gate Access Volume Splits (%) – Alternative 1							
Access ID	Description	АМ	РМ				
1/1	West Gate / Parking Garage Exit (Barta Road)	0%	0%				
1/2	North Gate (GEOINT Drive)	70%	70%				
1/3	South Gate (Heller Road)	30%	30%				

Table 4-2: Modeled Gate Access Volumes (%) – Alternative 2							
Access Description AM PM							
	Belvoir Gate (Enter) / Meade Gat	te (Exit)					
2/1	Meade Road Gate (Lieber)	0%	57%				
2/2	Belvoir Road Gate (Pence)	60%	0%				
2/3	Pohick Road Gate (Tulley)	40%	43%				



4.3 Existing Operational Analysis

The existing peak hour traffic volume (AM peak and PM peak hours) (Figures 2-3 and 2-4) and the existing lane-use configuration (Figures 4-3 and 4-4) were used in performing the existing (2021) operational analysis. *The existing (2021) peak hour volumes were adjusted assuming 60% of personnel reported during March 2021 counts from pre-COVID conditions. January 2020 and January 2021 total inbound gate counts were used for volume adjustment.* Gate counts were totaled from Inbound Tulley, Pence, Kingman, and Farrar gates for a similar Monday through Friday time period. See table for calculation summary.

Table 4-3: Adjusted Volume Calculation Methodology						
		202	20			
Gate Inbound	Monday	Tuesday	Wednesday	Thursday	Friday	Total
Tulley	8251	8315	8957	5324	7570	
Pence	7170	15403	15173	14781	14326	
Kingman	4338	5478	4773	4929	5909	
Farrar	1313	1415	1767	2079	1810	
Subtotal	21072	30611	30670	27113	29615	139081
		202	21			
Tulley	11810	11156	11555	12278	5714	
Pence	6839	6642	6930	6815	1637	
Kingman	2567	1957	5538	4530	1392	
Farrar	1486	1263	1264	1449	138	
Subtotal	22702	21018	25287	25072	8881	102960
2020 volume / 2021 volume						135%
Assume 2020 v	volumes 40	% higher th	an 2021 counte	ed volumes		140%

4.3.1 Existing (2021) Intersection Operational Analysis

The AM and PM peak hour intersection operational analyses results were evaluated using the Synchro 11 model. They are presented in **Tables 4-3 through 4-6**. The existing year Synchro output files are included in **Appendix C**.

Due to the nature of the anticipated additional trips, the weekday AM and PM peak periods were the focus of this study. Total volume counts system-wide were calculated from the intersection (TMC) and ATR data. The following peak hours identified and compared to Table 2-4.



Alternative 1 - FBNA

- AM peak period: 7:45am-8:45am;
- PM peak period: 4:00pm-5:00pm.

Table 4-4: Existing (2021, as counted) Intersection Operational Analysis – Alternative 1							
		Circolized	AM	PM	AM	PM	
Intersection ID	Intersection	Intersection Signalized (Y/N)		Delay (s/veh)		os	
В	Barta Road / FBNA Facilities Access	Y	0.9	1.0	А	А	
С	West Gate Entrance	N	-	-	А	А	
D	Barta Road / Parking Garage Exit	Y	0.0	10.0	А	А	
E	Barta Road / Main Guest Access	N	-	-	А	А	
F	Barta Road / GEOINT Drive	Y	3.2	8.9	А	А	
G	Barta Road / Heller Road	Y	8.8	1.0	А	А	
Н	Barta Road / Backlick Road	Y	6.8	11.4	А	В	
I	Heller Road / HOV Entrance Ramp	N	-	-	А	А	
J	95 Exit Ramp / Heller Road	Ν	-	-	А	А	
к	South Gate Entrance	Ν	-	-	А	А	

Table 4-5: Existing (adjusted) Intersection	Operational Analysis – Alternative 1
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Interception		Signalized	AM	PM	AM	PM
Intersection	Intersection Intersection		Delay (s/veh)		LOS	
В	Barta Road / FBNA Facilities Access	Y	1.7	1.1	А	А
С	West Gate Entrance	N	-	-	А	А
D	Barta Road / Parking Garage Exit	Y	0.0	10.4	А	В
E	Barta Road / Main Guest Access	N	-	-	А	А
F	Barta Road / GEOINT Drive	Y	5.5	13.3	А	В
G	Barta Road / Heller Road	Y	9.8	0.6	А	А
Н	Barta Road / Backlick Road	Y	7.9	20.1	А	С
I	Heller Road / HOV Entrance Ramp	N	-	-	А	А
J	95 Exit Ramp / Heller Road	N	-	-	А	А
К	South Gate Entrance	N	-	-	А	А



Alternative 2 – Fort Belvoir

- AM peak period: 7:30am-8:30am
- PM peak period: 3:45-4:45pm.

Table	Table 4-6: Existing (2021, as counted) Intersection Operational Analysis – Alternative 2										
Intersection	. <u>.</u> .	Signalized	AM	PM	AM	PM					
ID	Intersection	(Y/N)	Delay	(s/veh)	LOS						
N	Richmond Highway (Hwy 1) / Pohick Road	Y	20.1	29.5	С	С					
0	Gunston Road / 1st Street	Y	9.2	10.5	А	В					
Р	Gunston Road / 3rd Street	Y	22.8	13.7	С	В					
Q	Pohick Road / Theote Road	Y	9.1	9.2	А	А					
R	Gunston Road / Pohick Road / 12th Street	Y	15.8	14.8	В	В					
S	Richmond Highway (Hwy 1) / Belvoir Road	Y	31.5	44.5	С	D					
Т	Belvoir Road / DeWitt Loop (N)	Ν	8.0	3.8	А	А					
U	Gunston Road / Meade Road	Y	2.8	23.7	А	С					
Tab	le 4-7: Existing (2021, adjusted) Intersection Ope	erational Ana	lysis – J	Alternat	ive 2						
Intersection		Signalized	AM	PM	AM	PM					
ID	Intersection	(Y/N)	Delay (s/veh)		LC	DS					
N	Richmond Highway (Hwy 1) / Pohick Road	Y	25.1	36.4	С	D					
0	Gunston Road / 1st Street	Y	11.0	33.6	В	С					
Р	Gunston Road / 3rd Street	Y	22.2	14.1	С	В					
Q	Pohick Road / Theote Road	Y	11.4	12.2	В	В					
R	Gunston Road / Pohick Road / 12th Street	Y	19.9	24.5	В	С					
S	Richmond Highway (Hwy 1) / Belvoir Road	Y	42.8	78.7	D	Е					
Т	Belvoir Road / DeWitt Loop (N)	N	19.4	4.3	С	А					
U	Gunston Road / Meade Road	Y	3.0	55.6	А	Е					



4.3.2 Existing (2021) Entry Control Facility Analyses

In addition to analyzing intersections within each alternative location, entry control facility (ECF) gate capacities were calculated to determine existing manpower and entry lane needs. Adjusted peak hour volumes at each gate were utilized for calculations. The Quick Calculation method of Gate SMART Evaluator, provided through the SDDCTEA website, was used to determine these needs. Since this study is only determining the direct impacts the Annex will have on the existing site facilities, the analyses calculate only the current need and no future growth based on the *adjusted* existing volumes. The following table summarizes the existing needs. Demand volumes are 15-minute counts. The peak hour volumes were divided by 4 to calculate the inputs. Each gate's current configuration is either 3 or 4 processing lanes and is shown in the table. See Appendix D for calculation tables.

		7	able 4-8	3: Existing	g Gate Needs – J	Alterna	tive 1					
		Existing				Manual		Har	ndheld	A	IE	
Gate ID	Location	(Adjusted Demand Volume 15- minutes (veh)	Future Growth (%)	Percent Deployed (%)		Single	Tandem	Single	Tandem	No Arms	Arms	
					Existing Lanes			4				
	West Gate /			0	Required Lanes	1	1	1	1	1	1	
	Parking				Traffic Queue (Veh)	1	1	1	1	1	1	
1/1 Garage E (Barta Road)	`	Exit 148	0		Delay / Veh (seconds)	19	15	22	17	17	20	
	Road)				Total Manpower Needed	1	2	1	2	1	1	
		150	0	0	Existing Lanes	3						
	North Gate				Required Lanes	1	1	1	1	1	1	
					Traffic Queue (Veh)	1	1	1	1	1	1	
1/2	(GEOINT Drive)				Delay / Veh (seconds)	19	15	22	17	17	20	
					Total Manpower Needed	1	2	1	2	1	1	
					Existing Lanes			4		-		
					Required Lanes	1	1	1	1	1	1	
1/0	South Gate				Traffic Queue (Veh)	0	0	0	0	0	0	
1/3	(Heller Road)		0	0	Delay / Veh (seconds)	16	13	17	15	15	17	
					Total Manpower Needed	1	2	1	2	1	1	

Summary – Alternative 1

Currently, from the calculations, either 1 lane or 2 lanes of the existing lanes is utilized depending on the method of entrant processing.



Table 4-9: Existing Gate Needs – Alternative 2											
		Existing				Ma	nual	Har	ndheld	AIE	
Gate ID	Location	(Adjusted Demand Volume 15- minutes (veh)	Future Growth (%)	Percent Deployed (%)		Single	Tandem	Single	Tandem	No Arms	Arms
	Meade Road Gate	0		0	Existing Lanes			3			
					Required Lanes	1	1	1	1	1	1
			0		Traffic Queue (Veh)	0	0	0	0	0	0
2/1					Delay / Veh (seconds)	16	13	17	15	15	16
					Total Manpower Needed	1	2	1	2	1	1
	Belvoir Road Gate	398		0	Existing Lanes			3			
					Required Lanes	2	1	2	1	1	2
					Traffic Queue (Veh)	3	5	4	9	9	4
2/2			0		Delay / Veh (seconds)	18	27	21	59	59	20
					Total Manpower Needed	2	2	2	2	1	2
					Existing Lanes			3			
					Required Lanes	1	1	1	1	1	1
	Pohick Road				Traffic Queue (Veh)	4	2	7	3	3	5
2/3	Gate	288	0	0	Delay / Veh (seconds)	31	18	54	23	23	39
					Total Manpower Needed	1	2	1	2	1	1

Summary – Alternative 2

Currently, from the calculations, either 1 lane or 2 lanes of the existing lanes is utilized depending on the method of entrant processing.



5 BUILD CONDITIONS

5.1 **Proposed Site Development**

Two alternative locations have been selected to accommodate the proposed construction and operation of a new 155,000 square foot Defense Intelligence Agency (DIA) Headquarters (HQ) Annex building with an associated parking structure. Approximately 650 additional personnel will be employed at the new site. No changes to existing roadways have been identified for either locations. New infrastructure improvements are assumed to be limited to the building, parking structure, access lanes, and associated site improvements.

5.2 Geometric Configuration

No changes in roadway geometrics has been assumed for this study.

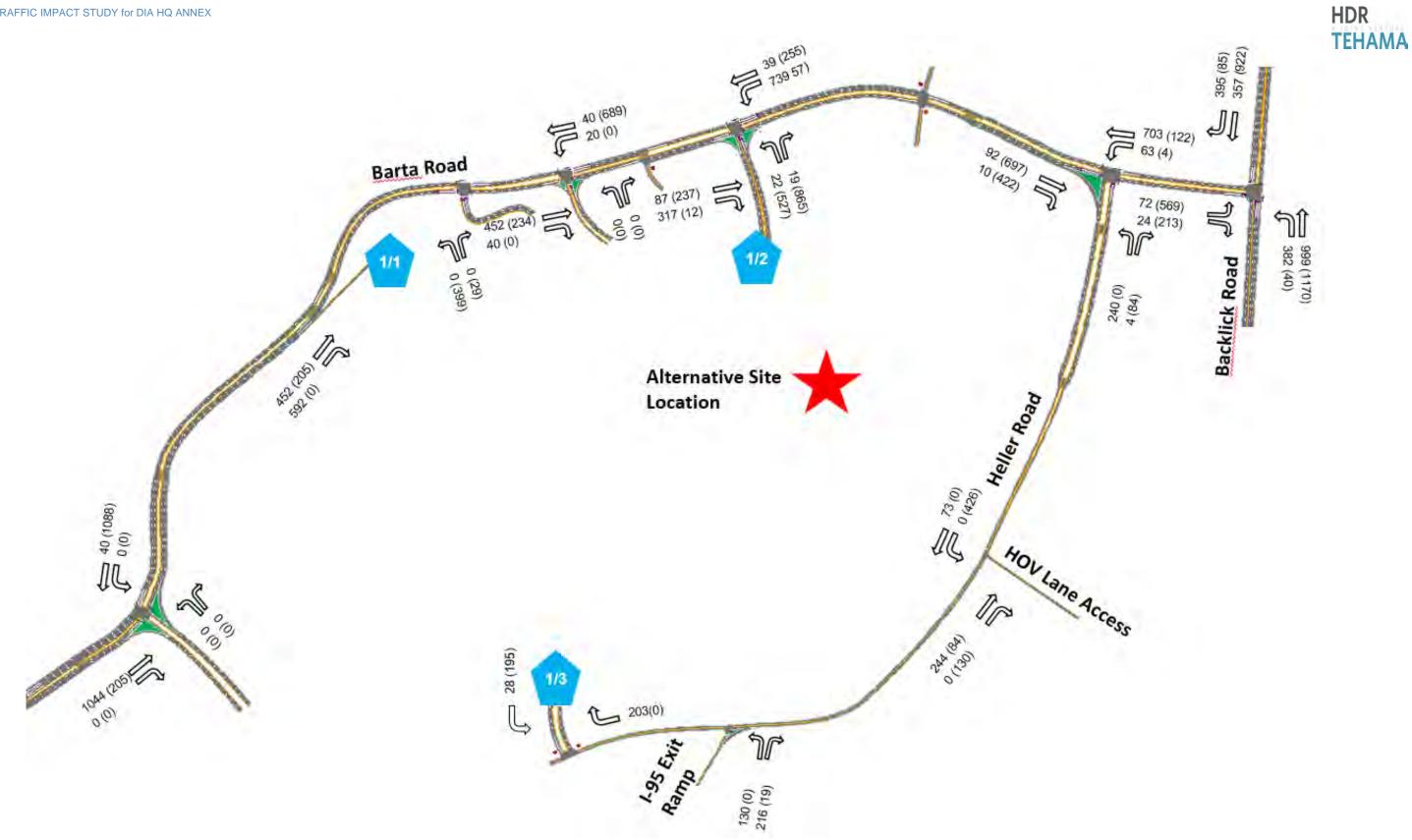
5.3 Trip generation

The annex construction is estimated to generate 650 additional staff positions. The analysis assumes that each additional staff member generates one (1) additional AM and PM peak hour trip for both 650 additional staff and 1000 additional staff scenarios. The distribution between site access points was determined utilizing the March 2021 count data.

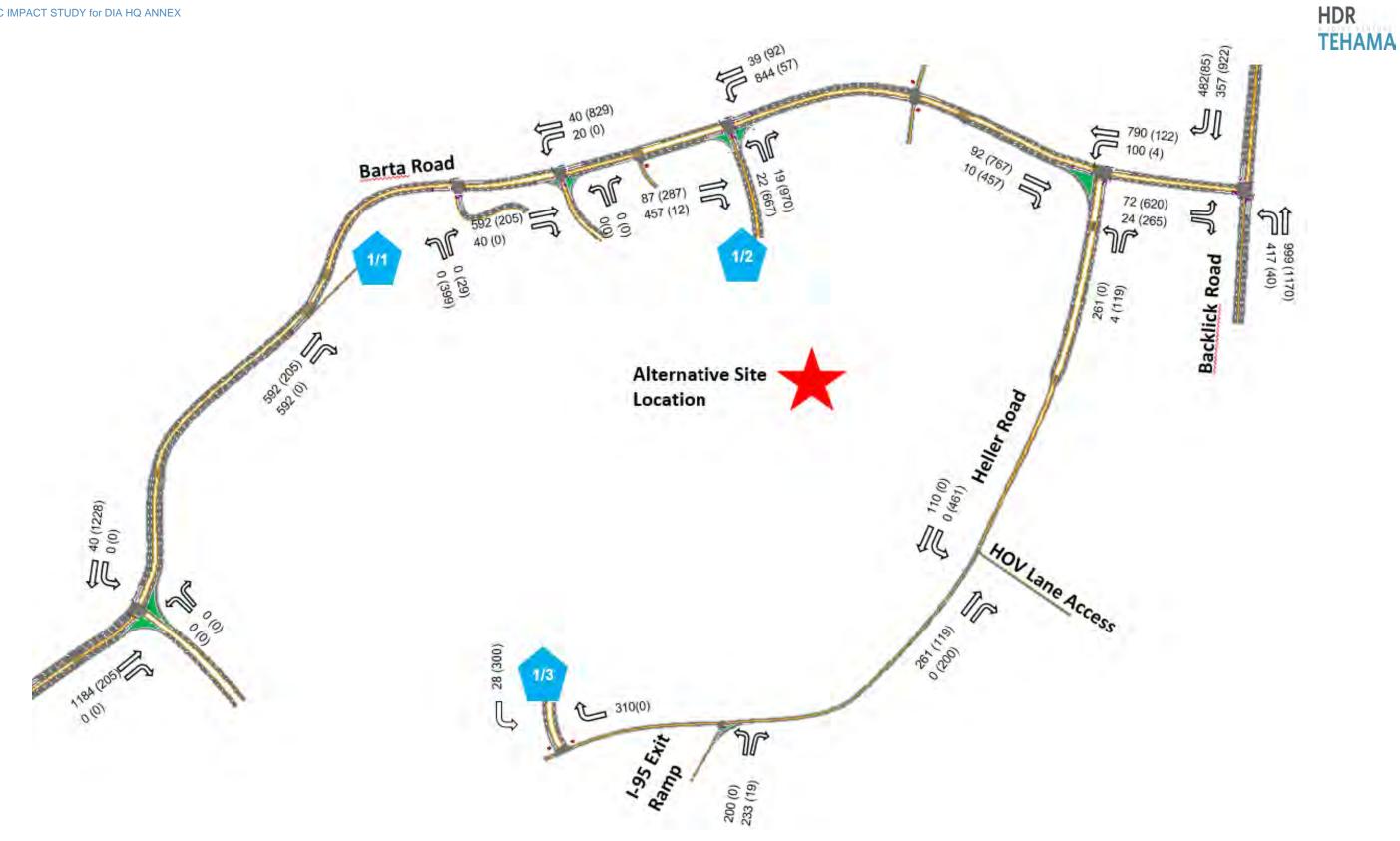
	Table 5-1: Trip Generation										
Scenario	Scenario Description	Trips									
		АМ	РМ								
1	650 Additional Staff	650	650								
2	1000 Additional Staff	1000	1000								

Table 4-1 and Table 4-2 show the percentage and volumes assigned to each site access point.

Figure 5-1 through Figure 5-4 show the total intersection volumes used for the Build condition. No background growth was used for the two alternative sites.









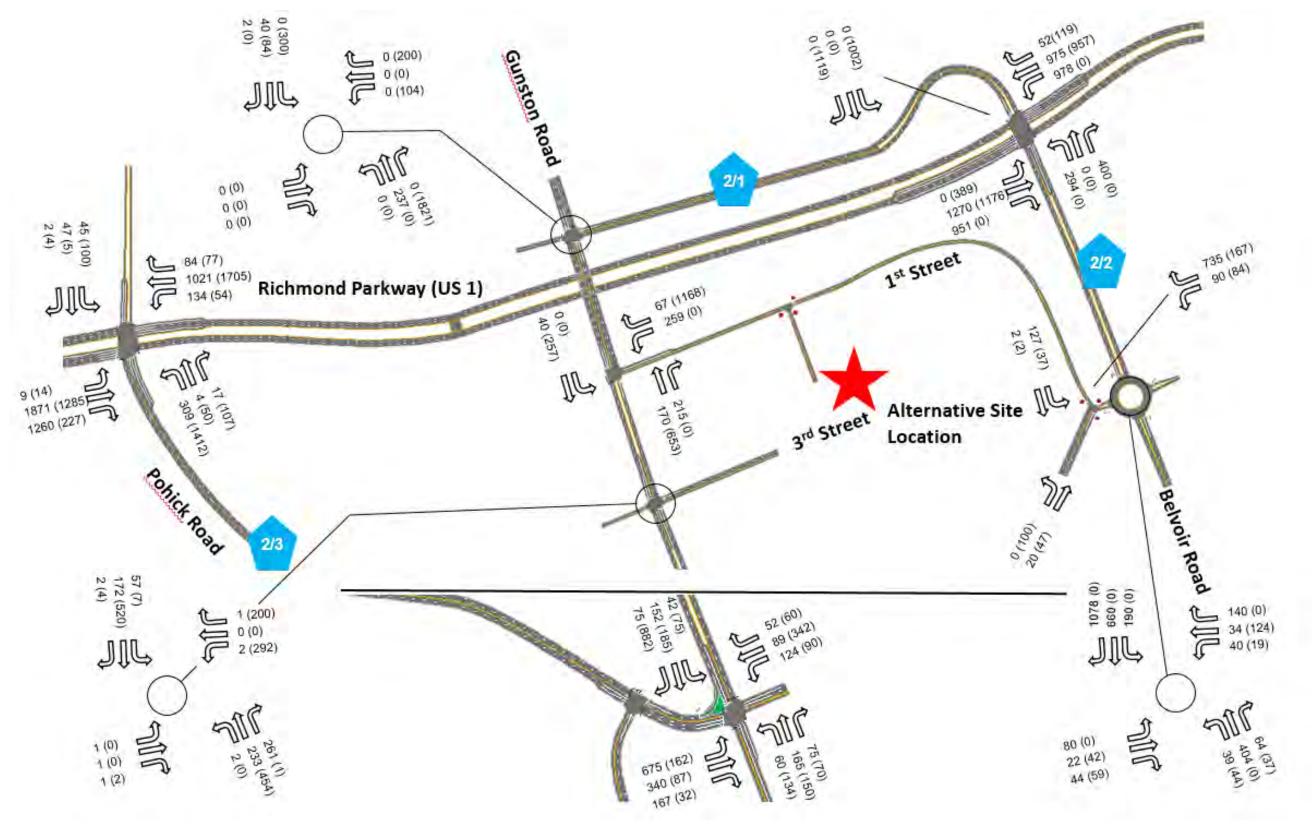


Figure 5-3: Volumes for Build Conditions (650 Additional Personnel) – Alternative 2



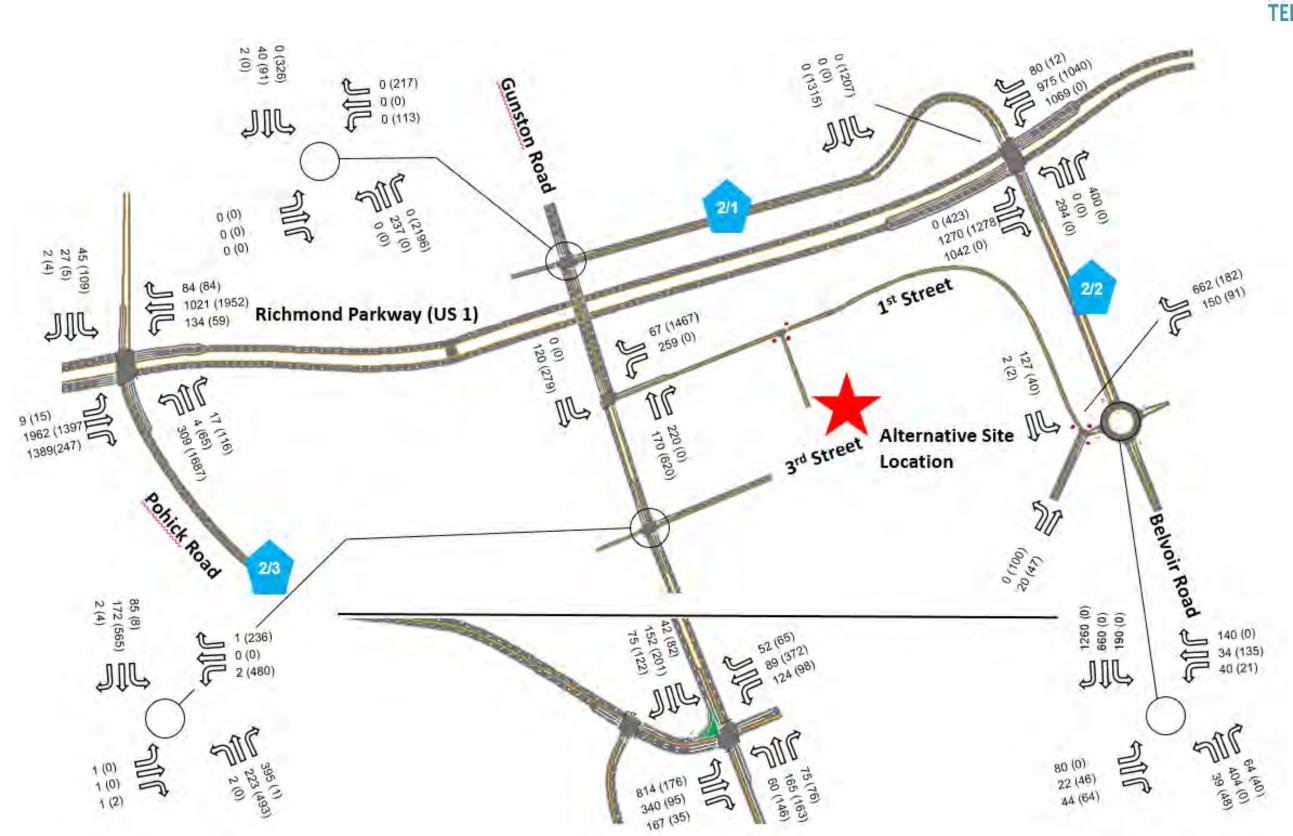


Figure 5-4: Volumes for Build Conditions (1000 Additional Personnel) – Alternative 2

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5.4 General Traffic Operations

Synchro traffic analysis models were created for each of the AM and PM peak periods to analyze traffic operations under existing and full-build conditions. The performance results of these models area presented in this section. Full Synchro reports are provided in Appendix C.

5.4.1 Intersections Analysis

Table 5-1 presents the general traffic operations summary for all scenarios analyzed for the in **Alternative 1**. Table 5-2 presents the general traffic operations summary for all scenarios analyzed for the intersections in **Alternative 2**.

	Table 5-2: Build Condition	n (2021 ac	ljusted) l	ntersect	ion Ope	rational	Analysis	– Altern	ative 1		
		pe	650 Added Personnel				1000 Added Personnel				
Int.	Intersection	Signalized (Y/N)	AM	PM	AM	PM	AM	PM	AM	PM	
ID))	Delay	(s/veh)	L	OS	Delay	(s/veh)	LOS		
В	Barta Road / FBNA Facilities Access	Y	2.0	1.3	A	А	2.2	1.5	A	A	
С	West Gate Entrance	N	-	-	А	Α	-	-	А	А	
D	Barta Road / Parking Garage Exit	Y	0.1	10.0	А	A	0.1	10.0	A	A	
E	Barta Road / Main Guest Access	N	-	-	А	A	-	-	A	A	
F	Barta Road / GEOINT Drive	Y	8.7	21.5	А	С	11.1	67.2	В	E	
G	Barta Road / Heller Road	Y	11.5	3.1	В	А	12.2	2.9	В	А	
н	Barta Road / Backlick Road	Y	8.0	21.5	А	С	20.4	20.9	С	С	
I	Heller Road / HOV Entrance Ramp	Ν	-	-	А	A	-	-	А	А	
J	95 Exit Ramp / Heller N Road		-	-	A	А	-	-	A	А	
к	South Gate Entrance	Ν	-	-	A	А	-	-	A	А	

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Alternative 1 FBNA, Adjusted Build

- All intersection (AM and PM) operate at LOS B or better with the exception of the intersections of:
 - Barta Road /Geoint Drive (LOS C during the PM peak hour) Exiting traffic from Geoint Drive creates queues while waiting to turn on to Barta Road.
 - Barta Road / Backlick Road (LOS C during the PM peak hour) Barta Road EB left turns queue and saturate the lanes waiting for Backlick thru movements to clear.

	Table 5-3: Build Condition (2021 adjusted) Intersection Operational Analysis – Alternative 2											
		pe	650	Added F	erson	nel	1000 Added Personnel					
Int.	Intersection	Intersection	Intersection	Signalized (Y/N)	AM	PM	AM	PM	AM	PM	AM	PM
ID		Sig.	Delay	(s/veh)	L	SS	Delay (s	s/veh)	L	os		
N	Richmond Parkway (US 1) / Pohick Road	Y	36.2	41.1	D	D	51.1	46.2	D	D		
0	Gunston Road / 1 st Street	Y	11.3	101.5	В	F	11.2	155.7	В	F		
Р	Gunston Road / 3 rd Street	Y	11.9	18.4	В	В	9.8	22.6	A	С		
Q	Pohick road / Thoete Road	Y	18.8	11.5	В	В	32.9	11.8	С	В		
R	Gunston Road / Pohick Road	Y	23.3	44.0	С	D	36.7	71.9	D	Е		
S	Richmond Parkway (US 1) / Belvoir Road	Y	63.5	124.5	Е	F	82.6	153.2	F	F		
Т	Belvoir Road / DeWitt Loop (roundabout)	Ν	50.3 (98.5 SB RT)	4.3	F	A	87.1 (178.4 SB RT)	4.3	F	A		
U	Gunston Road / Meade Road	Y	9.4	112.4	A	F	12.4	167.5	В	F		



Alternative 2 Fort Belvoir, Adjusted Build

- Several intersections fail in the AM and PM peak hour.
 - Belvoir Road / DeWitt Loop Roundabout (LOS F during the AM peak hour) Additional Belvoir Gate traffic causes the SB right turn movement to fail.
 - Gunston Road / 1st Street and Richmond Parkway (US 1) / Belvoir Road (LOS F during the PM peak hour) – Large NB exiting volumes on Gunston Road are required to use a single right-turn lane onto Meade Road.
- Already heavy entering and exiting traffic at the Richmond Parkway / Meade Road / Belvoir Road intersection during the AM and PM peak hours are added to with the planned additional traffic. Traffic operations show less than desired performance given the limited intersection options along the busy arterial of Richmond Parkway.



5.4.2 Proposed Entry Control Facility Analyses

The ECFs were analyzed for the 650 additional staff build condition. The gate capacities were calculated to determine future build year manpower and entry lane needs. Adjusted peak hour volumes and the proposed 650 vehicles were utilized for calculations. The Quick Calculation method of Gate SMART Evaluator, provided through the SDDCTEA website, was used to determine these needs. The analyses calculate only the current build need and no future growth. The following tables show the build needs for the existing adjusted volumes plus the additional 650 personnel. Demand volumes are 15-minute counts. Each gate's current configuration is either 3 or 4 processing lanes and is shown in the table. See Appendix D for calculation tables.

	Table 5-4: Existing Gate Build Needs – Alternative 1												
		Existing (Adjusted)				Manual		Har	ndheld	AIE			
Gate ID	Location	Demand Volume w/ 650 Additional Staff 15-minutes (veh)	Future Growth (%)	Percent Deployed (%)		Single	Tandem	Single	Tandem	No Arms	Arms		
					Existing Lanes	4							
	West Gate				Required Lanes	1	1	1	1	1	1		
1/1	/ Parking Garage	148	0	0	Traffic Queue (Veh)	1	1	1	1	1	1		
	Exit (Barta Road)	140	0		Delay / Veh (seconds)	19	15	22	17	17	20		
					Total Manpower Needed	1	2	1	2	1	1		
	(0	0	Existing Lanes	nes 3							
	North Gate (GEOINT Drive)				Required Lanes	1	1	1	1	1	1		
1/2					Traffic Queue (Veh)	3	2	5	2	2	4		
					Delay / Veh (seconds)	27	17	40	21	21	32		
					Total Manpower Needed	1	2	1	2	1	1		
					Existing Lanes		-	4	-	-	-		
					Required Lanes	1	1	1	1	1	1		
1/3	Gate	South Gate (Heller Road)	0	0	Traffic Queue (Veh)	0	0	0	0	0	0		
./0					Delay / Veh (seconds)	16	14	18	15	15	17		
					Total Manpower Needed	1	2	1	2	1	1		

Alternative 1

- All Gates
 - No additional manpower or lanes required.
 - Minor additional vehicle queueing.



	Table 5-5: Existing Gate Build Needs – Alternative 2											
Gate ID	Location	Existing (Adjusted) Demand Volume w/ 650 Additional Staff 15-minutes (veh)	Future Growth (%)	Percent Deployed (%)		Ma Single	nual Tandem	Har Single	ndheld Tandem	A No Arms	IE Arms	
					Existing Lanes		-	3				
	Meade Road Gate				Required Lanes	1	1	1	1	1	1	
1/1		13		0	Traffic Queue (Veh)	0	0	0	0	0	0	
1/ 1			0		Delay / Veh (seconds)	16	13	17	15	15	17	
					Total Manpower Needed	1	2	1	2	1	1	
	Belvoir	ə ⁴⁸²	0	0	Existing Lanes	3						
					Required Lanes	2	1	2	2	2	2	
1/2					Traffic Queue (Veh)	5	11	7	4	4	6	
1/2	Road Gate				Delay / Veh (seconds)	20	66	25	17	17	22	
					Total Manpower Needed	2	2	2	4	2	2	
					Existing Lanes		-	3			-	
					Required Lanes	1	1	2	1	1	1	
4/2	Pohick				Traffic Queue (Veh)	9	3	3	5	5	11	
1/3	Road Gate	ate 349	0	0	Delay / Veh (seconds)	63	21	20	33	33	110	
					Total Manpower Needed	1	2	2	2	1	1	

Alternative 2

- Meade Gate
 - No additional manpower or lanes required.
- Belvoir Gate
 - o No additional lanes required.
 - 2 additional staff required for Handheld Tandem processing, or 1 additional staff for No Arms AIE processing.
 - Minor additional vehicle queueing.
 - o Minor additional delays.
- Pohick Gate
 - No additional lanes required.
 - o 1 additional staff required for Handheld Single processing.
 - o Minor additional vehicle queueing.
 - o Minor additional delays.

5.5 Transit Operations

No transit lines are present within either alternate location.



5.5.1 Existing Bus Routes

There are three bus transit routes that pass near Fort Belvoir and FBNA:

- Route 171
- Route 335
- REX (Richmond Highway Express)

Routes 171 and 335 are operated by the Fairfax Connector, and the REX is operated by Washington Metropolitan Area Transit Authority. Ongoing studies and public outreach are being completed for possible expansion of transit facilities (*Springfield to Quantico Enhanced Public Transportation Feasibility Study*). For the purpose of this study no analysis of bus route capacity or level of service was deemed relevant.

5.6 Pedestrian and Bicycle Operations

For the signalized intersections within the study area there are no intersections that are expected to experience lane changes. Pedestrian and bicycle volumes were witnessed to be extremely low. Signals within the alternative sites allow for pedestrian movements. There are dedicated bicycle facilities (NB / SB bike lanes) along Gunston Road and Belvoir Road at Alternative 2. Separated pedestrian facilities are present along Barta Road from 286 to Backlick Road at Alternate 1.



6 CONCLUSIONS AND RECOMMENDATIONS

Based on the analysis completed in the above sections, the following conclusions can be made:

Traffic Operations

- Existing Conditions
 - The analysis indicates that all signalized intersections are operating at acceptable levels overall (LOS D or better) at both alternate locations.
 - For the unsignalized intersections, the analysis indicates that the majority of the intersections are operating well.
 - o Alternative 2
 - The intersection of Richmond Parkway and Belvoir Road operates at capacity (v/c ratio at 1.02, WB left) currently.
- Build Scenarios
 - o FBNA Alternative 1, Scenario 1 (650 Additional Personnel)
 - Intersection F (Barta Road/Geoint Drive) The additional left turning volumes into Geoint Drive (AM) and the increased left/right turning volumes exiting Geoint Drive (PM) decrease the level of service due to added delay. Intersection AM peak LOS drops from LOS A to LOS B. Intersection PM peak LOS drops from LOS B to LOS C. The following are critical movement:
 - AM WB Barta Road to SB Geoint Drive
 - Mitigation Signal optimization.
 - PM NB Geoint Drive to both EB & WB Barta Road
 - Mitigation Signal optimization
 - Intersection H (Barta Road/Backlick Drive) The additional AM left turns from the south leg of Backlick Road exceed the capacity of the single turn lane and signal timing plan. Intersection PM peak LOS drops from LOS A to LOS C.
 - PM EB Barta Road to NB Backlick Road
 - Mitigation Signal optimization.

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- FBNA Alternative 1, Scenario 2 (1000 Additional Personnel)
 - The results discussed above in Scenario 1 are compounded with the additional 350 personnel.
 - Intersection F (Barta Road/Geoint Drive) The additional left turning volumes into Geoint Drive (AM) and the increased left/right turning volumes exiting Geoint Drive (PM) decrease the level of service due to added delay. Intersection AM peak LOS drops from LOS A to LOS C. Intersection PM peak LOS drops from LOS B to LOS E. The following are critical movements:
 - AM WB Barta Road to SB Geoint Drive
 - o Mitigation Signal optimization
 - PM NB Geoint Drive to both EB & WB Barta Road
 - o Mitigation Signal optimization
 - Intersection H (Barta Road/Backlick Drive) The additional AM left turns from the south leg of Backlick Road exceed the capacity of the single turn lane and signal timing plan. Intersection AM peak LOS drops from LOS A to LOS D. The following are critical movements:
 - PM EB Barta Road to NB Backlick Road
 - Mitigation Signal optimization
- Fort Belvoir Alternative 2, Scenario 1 (650 Additional Personnel)
 - Intersection S (Belvoir Road / Richmond Parkway) The additional left turning volumes into Belvoir Road (AM) and the increased left/right turning volumes exiting Meade Road (PM) decrease the level of service due to added delay. Intersection AM peak LOS drops from LOS D to LOS E. Intersection PM peak LOS drops from LOS E to LOS F. The following are critical movements:
 - AM WB Richmond Parkway to SB Belvoir Road (dual-lane left turn)
 - Mitigation Signal optimization and construct an additional SB merge lane for EB Richmond Parkway right turns.
 - PM SB Meade Road to EB / WB Richmond Parkway
 - Mitigation Provide an additional 200-250 foot left and right turn lane to provide for dual lefts and a designated right turn lane.
 - Intersection T (Belvoir Road / DeWitt Loop roundabout t) The adjusted existing volumes entering the existing roundabout approach capacity for a two-lane roundabout. SB Belvoir Road to WB DeWitt Loop exceeds lane capacity. The roundabout AM peak hour LOS decreases from LOS C to LOS F. The following are critical movements:



- AM SB Belvoir Road to WB DeWitt Loop (roundabout) o Mitigation – DETERMINATION IN PROGRESS
- Intersection U (Gunston Road / Meade Road) Similarly to the Belvoir Road roundabout, the Gunston Road / Meade Road intersection operates poorly in the PM peak hour. The exiting base traffic flows through a single lane right-turn movement. Intersection PM peak LOS drops from LOS E to LOS F. The following are critical movements:
 - PM NB Gunston Road to EB Meade Road
 - Mitigation Signal optimization and construct signalized NB dual right turn lanes onto Meade Road.
- Fort Belvoir Alternative 2, Scenario 2 (1000 Additional Personnel)
 - The results discussed above in Scenario 1 are compounded with the additional 350 personnel.
 - Intersection S (Belvoir Road / Richmond Parkway) The additional left turning volumes into Belvoir Road (AM) and the increased left/right turning volumes exiting Meade Road (PM) decrease the level of service due to added delay. Intersection AM peak LOS drops from LOS D to LOS F. Intersection PM peak LOS drops from LOS E to LOS F. The following are critical movements:
 - AM WB Richmond Parkway to SB Belvoir Road (dual-lane left turn)
 - Mitigation Signal optimization and construct an additional SB merge lane for EB Richmond Parkway right turns.
 - PM SB Meade Road to EB / WB Richmond Parkway
 - Mitigation Provide an additional 200-250 foot left and right turn lane to provide for dual lefts and a designated right turn lane.
 - Intersection T (Belvoir Road / DeWitt Loop roundabout t) The adjusted existing volumes entering the existing roundabout approach capacity for a two-lane roundabout. SB Belvoir Road to WB DeWitt Loop exceeds lane capacity. The roundabout AM peak hour LOS decreases from LOS C to LOS F. The following are critical movements:
 - AM SB Belvoir Road to WB DeWitt Loop (roundabout)
 - Mitigation DETERMINATION IN PROGRESS
 - Intersection U (Gunston Road / Meade Road) Similarly to the Belvoir Road roundabout, the Gunston Road / Meade Road intersection operates poorly in the PM peak hour. The exiting base traffic flows through a single lane right-turn movement. Intersection PM peak LOS drops from LOS E to LOS F. The following are critical movements:
 - PM NB Gunston Road to EB Meade Road
 - Mitigation Signal optimization and construct signalized NB dual right turn lanes onto Meade Road.

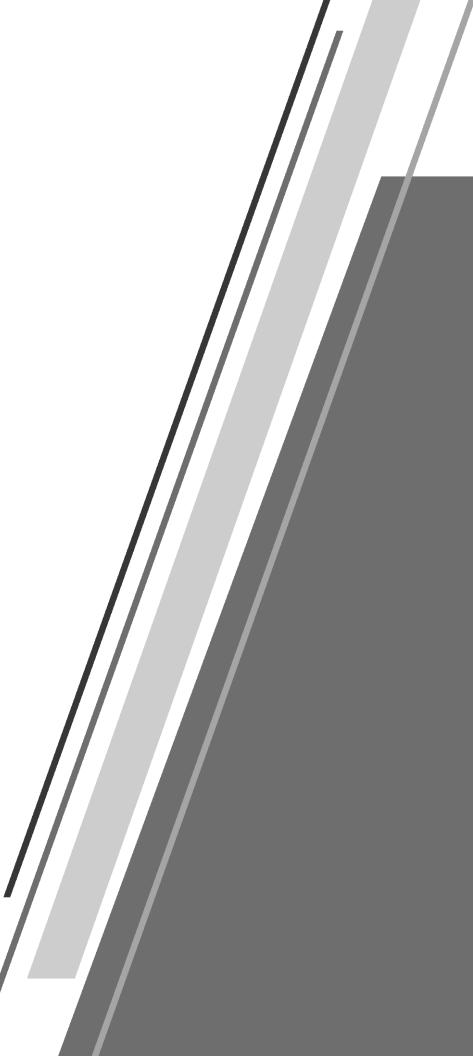


- Pedestrian and Bicycle Operations
 - Pedestrians are provided shared phasing with appropriate traffic phases. No impacts are expected at either site.

This study recommends that Alternative 1 (FBNA) be selected, as there is more available capacity to accommodate increased traffic volumes.

APPENDIX A

Signal Timing



Traffic Impact Study for DIA HQ Annex

Signalized Intersections Phasing and Timing

Alternative 1 – Location 1: Barta Road with Geoint Drive

- Notes taken right after AM time slot on Wednesday, April 7TH
- 35 MPH for EB/WB legs (Barta Rd), 25 MPH for SB leg (Geoint Dr)
- Phase 2 -> EB, Phase 6 -> WB, Phase 4 -> SB
- Protected left hand turn phase for EBL (Phase 5)
- 20 sec EBL (Phase 5), 15 sec EB/WB (Phases 2 and 6), 15 sec SBL/SBR (Phase 4)
- 50 sec total cycle
- Median separated WBR lane (yield), Median separated SBR lane (yield)

Alternative 1 – Location 2: Barta Road with Heller Road

- Notes taken right during AM time slot on Wednesday, April 7TH
- 25 MPH for all legs
- Phase 2 -> EB, Phase 6 -> WB, Phase 4 -> SB
- Phases 2 and 6 are continuous until Phase 4 is called when a vehicle arrives
- 15 sec SBL/SBR (Phase 4), Continuous EB/WB (Phases 2 and 6)
- All right turn on reds are legal
- WBR had extended median a few hundred feet down the road closer to NGA buildings

Alternative 1 – Location 3: Barta Road with Backlick Road

- Notes taken shortly after AM time slot on Wednesday, April 7TH
- 25 MPH for WB leg (Barta Rd), 45 MPH for NB/SB leg (Backlick Rd)
- Phase 2 -> SB, Phase 6 -> NB, Phase 4 -> WB, Phase 5 -> SBL (protected)
- Protected left turn phase for SBL (Phase 5)
- NBR arrow on when Phase 4 is on
- Phases 2 and 6 are continuous (with flashing yellow arrow for Phase 5) until Phase 4 is called when a vehicle arrives
- 10-15 sec WBL/WBR (Phase 4) , 15 sec SBL (Phase 5)
- SBL must yield on a flashing yellow arrow (NBR has right of way over SBL except during Phase 5)
- Phase 2 and Phase 5 run concurrently
- WB has two lanes -> one left only lane and one dual left-right turn lane
- All right turns on red are legal
- Not sure what the total cycle length adds up to -> I would guess on the lower side 60 sec maybe

Alternative 2 – Location 1: US 1 with Meade Road/ Belvoir Road

- Notes taken at approximately 11:45 AM 12:00 PM Wednesday, April 7TH
- 45 MPH for EB/WB legs (US 1), 25 MPH for NB leg (Belvoir Rd)
- Meade Road was closed during the traffic counts so the intersection acted as a threelegged intersection
- Phase 2 -> EB, Phase 6 -> WB, Phase 4 -> NB, Phase 5 -> EBL (Protected), Phase 1 -> WBL (Protected), Phase 8 -> SB (Closed), Phase 3 -> SBL (Closed)
- EBR and WBR both have right turn arrows
- All right turns on red are legal
- Total phase time = 150 sec
- 125 sec total EB/WB (Phases 1, 2, 5 and 6), 25 sec EBL/WBL (Phases 1 and 2), 25 sec NBL/NBR (Phase 4)
- WBL/WBT would start at same time if no vehicles going EBL (Blocked off since closed gate/road)
- 55 sec crossing for pedestrians across US 1 (across WB leg)
- 30 sec crossing for pedestrians across Meade Rd/Belvoir Rd

Alternative 2 – Location 5: US 1 with Pohick Road/Backlick Road

- Notes take at approximately 11:15 AM 11:40 AM Wednesday, April 7TH
- 50 MPH for EB/WB legs (US 1), 25 MPH for NB/SB legs (Pohick Rd/ Backlick Rd)
- Phase 2 -> EB, Phase 6 -> WB, Phase 4 -> NB, Phase 8 -> SB, Phase 1 -> WBL (Protected), Phase 5 -> EBL (Protected)
- All turns on red are legal
- EBR and WBR have right turn arrows
- 20-25 sec WBL/EBL (Phases 1 and 5), 80-100 sec WB/EB total (Phases 2 and 6), 18-25 sec NB (Phase 4), 18 sec SB (Phase 8)
- Was hard to tell the timings of each leg as it was a busy intersection
- Total phase time = 145 160 sec
- SB and NB legs did not run concurrently! Phases 2 and 6 did, Phase 4, then Phase 8
- WBL starts with WBT if no vehicles in queue at EBL
- 30 sec crossing for pedestrians across Pohick Rd (across NB leg)
- 60 sec crossing for pedestrians across US 1 (across EB/WB legs)
- 17 sec crossing for pedestrians across Backlick Rd (across SB leg)

Alternative 2 – Location 3: Gunston Road with Pohick Road/12th Street

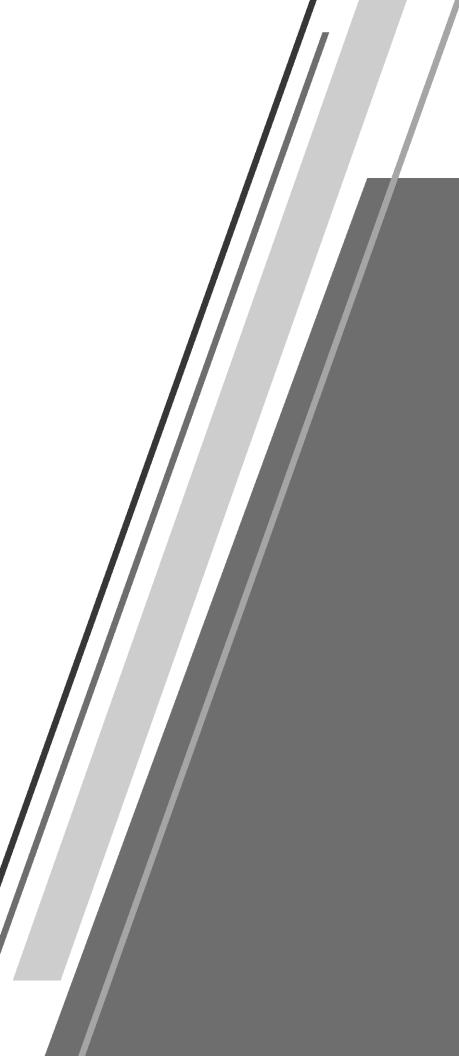
- Notes taken at approximately 12:15 PM 12:45 PM Wednesday, April 7TH
- 25 MPH for all legs
- Phase 2 -> NB, Phase 6 -> SB, Phase 4 -> EB, Phase 8 -> WB, Phase 5 -> NBL, Phase 1 -> SBL, Phase 7-> EBL, Phase 3 -> WBL
- All lefts are protected-permissive if possible
- 5-15 sec NBL (Phase 5), 5-15 sec SBL (Phase 1), 5-15 sec EBL (Phase 7), 5-15 sec NBL (Phase 3) when they are isolated
- 20 sec EBL/WBL together (Phases 1 and 7)
- 45 sec total NB/SB (Phases 2 and 6), 45 sec total EB/WB (Phases 4 and 8)
- Total cycle time = 90 sec
- 30 sec crossing pedestrians for each leg

Alternative 2 – Location 4: Gunston Road with 3rd Street

- Notes taken at approximately 12:45 PM 1:00 PM Wednesday, April 7th
- 25 MPH for all legs
- Phase 2 -> NB, Phase 6 -> SB, Phase 4 -> WB, Phase 8 -> EB, Phase 5 -> NBL, Phase 1 -> SBL, Phase 7 -> WBL
- NBL, SBL and WBL are all permitted left turns
- All turns on red are legal
- 15 sec EB/WB (Phases 4, 7 and 8)
- Phases 2 and 6 are continuous until Phase 4, 7 or 8 are called when a vehicle arrives
- EB and WB run concurrently
- Pedestrian crossing on NB, EB and WB legs

APPENDIX B

Traffic Data



r	Total Traffic	Total Traffic						Loor	ation					i
Time	(1 Hr)	(15 min)	1	2	3	4/5	6	7A	7B	8A	8B	9	10	11
12:00:00 AM	10	2				./ 0	2	0	0	0	0	0	0	0
12:15:00 AM	10	4					3	1	0	0	0	0	0	0
12:30:00 AM	9	3					3	0	0	0	0	0	0	0
12:45:00 AM	11	1					1	0	0	0	0	0	0	0
1:00:00 AM	11	2					2	0	0	0	0	0	0	0
1:15:00 AM	11	3					0	2	0	0	1	0	0	0
1:30:00 AM	11	5					2	1	0	0	2	0	0	0
1:45:00 AM	12	1					0	1	0	0	0	0	0	0
2:00:00 AM 2:15:00 AM	15 18	23					0	1	0	0	1	0	0	0
2:30:00 AM	36	6					0	5	0	0	1	0	0	0
2:45:00 AM	39	4					0	4	0	0	0	0	0	0
3:00:00 AM	40	5					0	5	0	0	Ő	0	0	0
3:15:00 AM	43	21					0	20	0	0	1	0	0	0
3:30:00 AM	27	9					0	5	0	3	1	0	0	0
3:45:00 AM	27	5					0	2	0	1	2	0	0	0
4:00:00 AM	34	8					0	7	0	1	0	0	0	0
4:15:00 AM	36	5					0	5	0	0	0	0	0	0
4:30:00 AM	44	9					0	9	0	0	0	0	0	0
4:45:00 AM	50	12					0	11	0	1	0	0	0	0
5:00:00 AM	276 705	10 13					0	9	0	0	0	0	0	0
5:15:00 AM 5:30:00 AM	1142	15					0	11 11	0	1	0	0	0	3
5:45:00 AM	1633	238			199	27	0	10	0	1	0	0	0	1
6:00:00 AM	1924	439	119	94	194	21	0	10	0	1	0	0	0	0
6:15:00 AM	2037	450	89	95	238	10	0	16	0	1	Ő	0	0	1
6:30:00 AM	2219	506	101	82	300	6	0	11	0	0	0	0	0	6
6:45:00 AM	2346	529	114	109	281	8	0	13	0	1	0	0	0	3
7:00:00 AM	2468	552	116	83	320	3	0	10	0	3	0	15	0	2
7:15:00 AM	2589	632	110	116	343	5	0	12	0	0	0	40	0	6
7:30:00 AM	2611	633	123	109	324	8	0	18	0	2	0	44	0	5
7:45:00 AM	2632	651	108	104	310	3	0	30	0	7	0	81	0	8
8:00:00 AM	2565	673	109	108 104	318 279	3 11	0	27	0	8	0	89	0	11
8:15:00 AM	2020	654	130	-	-	11 7	-	28	0	1	0	93	0	8
8:30:00 AM 8:45:00 AM	1501 992	654 584	<u>112</u> 111	116	292 284	0	0	32 14	0	1	0	92 88	0	2
9:00:00 AM	554	128	0	83 0	204	0	0	14	0	0	0	101	0	10
9:15:00 AM	566	135	0	0	0	0	0	14	0	1	0	101	0	10
9:30:00 AM	571	145	0	0	0	0	0	11	0	2	0	117	0	15
9:45:00 AM	552	146	0	0	0	0	0	10	0	0	0	124	0	12
10:00:00 AM	515	140	0	0	0	0	0	10	0	1	0	116	0	13
10:15:00 AM	441	140	0	0	0	0	0	11	0	1	0	112	0	16
10:30:00 AM	324	126	0	0	0	0	0	11	0	1	0	102	0	12
10:45:00 AM	219	109	0	0	0	0	0	12	0	1	0	81	0	15
11:00:00 AM	126	66	0	0	0	0	0	9	0	0	0	44	0	13
11:15:00 AM	69	23	0	0	0	0	0	10	0	1	0	0	0	12
11:30:00 AM	57	21	0	0	0	0	0	13	0	0	0	0	0	8 10
11:45:00 AM 12:00:00 PM	56 49	16 9	0	0	0	0	0	5 6	0	0	0	0	0	3
12:15:00 PM	49	 11	0	0	0	0	0	5	0	0	0	0	0	6
12:30:00 PM	43	20	0	0	0	0	0	8	0	2	0	0	0	10
12:45:00 PM	29	9	0	0	0	0	0	5	0	0	0	0	0	4
1:00:00 PM	25	7	0	0	0	0	0	2	0	0	0	0	0	5
1:15:00 PM	34	7	0	0	0	0	0	2	0	0	0	0	0	5
1:30:00 PM	46	6	0	0	0	0	0	3	0	0	0	0	0	3
1:45:00 PM	58	5	0	0	0	0	0	1	0	0	0	0	0	4
2:00:00 PM	578	16	0	0	0	0	4	4	0	0	0	0	0	8
2:15:00 PM	1447	19	0	0	0	0	10	5	0	0	0	0	0	4
2:30:00 PM 2:45:00 PM	2295 3106	18 525	0 181	0	0 329	0	11 10	4	0	0	0	0	0	3 4
3:00:00 PM	3106	525 885	181	176	329	123	10	6	0	0	0	0	0	6
3:15:00 PM	3679	867	213	153	365	85	42	3	0	0	0	0	0	6
3:30:00 PM	3817	829	178	146	365	120	12	2	0	0	0	0	0	6
3:45:00 PM	4034	884	221	132	347	134	35	10	0	0	0	0	1	4
4:00:00 PM	4149	1099	215	186	441	127	60	3	0	0	0	0	63	4
4:15:00 PM	4017	1005	175	186	387	129	64	4	0	0	0	0	59	1
4:30:00 PM	3992	1046	191	174	426	123	45	4	0	0	0	0	79	4
4:45:00 PM	3779	999	219	131	427	117	47	0	0	0	0	0	55	3
5:00:00 PM	3445	967	213	157	365	108	37	3	0	0	0	0	81	3
5:15:00 PM	2629	980	210	135	388	93	68	1	0	0	0	0	83	2
5:30:00 PM	1856	833	159	120	323	72	59	0	0	0	0	0	97	3
5:45:00 PM 6:00:00 PM	1198 688	665 151		103 0	320	68	65 54	0	0	0	0	0	109 95	0
6:15:00 PM	589	207		U			54 87	U	0	0	0	0	95 116	4
6:30:00 PM	413	175				-	71	-	3	0	0	0	98	4 3
6:45:00 PM	266	155					55		3	0	6	0	89	2
7:00:00 PM	143	52							Ŭ	0	27	0	20	5
7:15:00 PM	131	31					1			Ő	28	0	0	3
7:30:00 PM	138	28								0	26	0	0	2
7:45:00 PM	154	32								0	29	0	0	3
8:00:00 PM	172	40								0	39	0	0	1
8:15:00 PM	182	38								0	37	0	0	1
8:30:00 PM	205	44								0	44	0	0	0
8:45:00 PM	223	50								0	49	0	0	1
9:00:00 PM	210	50								0	50	0	0	0
9:15:00 PM	218	61				1	1	l	1	0	61	0	0	0

	Total Traffic	Total Traffic	Location											
Time	(1 Hr)	(15 min)	1	2	3	4/5	6	7A	7B	8A	8B	9	10	11
9:30:00 PM	199	62								0	62	0	0	0
9:45:00 PM	172	37								0	37	0	0	0
10:00:00 PM	144	58								0	58	0	0	0
10:15:00 PM	87	42								0	42	0	0	0
10:30:00 PM	50	35								0	35	0	0	0
10:45:00 PM	17	9								0	9	0	0	0
11:00:00 PM	10	1								0	1	0	0	0
11:15:00 PM		5								0	5	0	0	0
11:30:00 PM		2								0	2	0	0	0
11:45:00 PM		2								0	2	0	0	0

File Name: C:\Petra\Gray\2021_03_23_Alt 1_Loc 1_ Barta Road with Geoint.ppd

Start Date: 3/23/2021

Start Time: 6:00:00 AM

Site Code: 00000000

Comment 1: Default Comments

Comment 2: Change These in The Preferences Window

Comment 3: Select File/Preference in the Main Scree

Comment 4: Then Click the Comments Tab

		Barta Road			Geoint Drive			Barta Road			
		From East		F	From South			From West			
Start											
Time	Left	Thru	Peds	Left	Right	Peds	Thru	Right	Peds	Tot. Veh.	
06:00 AM	82	5	1	4	8	1	6	14	0	119	2
06:15 AM	71	6	0	0	6	1	2	4	0	89	1
06:30 AM	68	5	0	4	9	0	6	9	0	101	0
06:45 AM	90	5	0	2	3	0	7	7	0	114	0
07:00 AM	86	4	0	2	4	1	10	10	0	116	1
07:15 AM	87	2	0	7	2	2	9	3	0	110	2
07:30 AM	98	4	0	1	4	2	10	6	0	123	2
07:45 AM	79	4	0	0	3	1	15	7	0	108	1
08:00 AM	81	5	0	3	5	1	11	4	0	109	1
08:15 AM	93	6	0	5	1	1	13	12	0	130	1
08:30 AM	73	8	0	5	2	2	13	11	0	112	2
08:45 AM	71	7	0	4	7	1	8	14	0	111	1
09:00 AM	0	0	0	0	0	0	0	0	0	0	0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0
09:30 AM	0	0	0	0	0	0	0	0	0	0	0
09:45 AM	0	0	0	0	0	0	0	0	0	0	0
10:00 AM	0	0	0	0	0	0	0	0	0	0	0
10:15 AM	0	0	0	0	0	0	0	0	0	0	0
10:30 AM	0	0	0	0	0	0	0	0	0	0	0
10:45 AM	0	0	0	0	0	0	0	0	0	0	0
11:00 AM	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0
01:00 PM	0	0	0	0	0	0	0	0	0	0	0
01:15 PM	0	0	0	0	0	0	0	0	0	0	0
01:30 PM	0	0	0	0	0	0	0	0	0	0	0
01:45 PM	0	0	0	0	0	0	0	0	0	0	0
02:00 PM	0	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	0	0	0	0	0	0	0	0	0
02:30 PM	0	0	0	0	0	0	0	0	0	0	0
02:45 PM	11	8	0	28	96	0	34	4	0	181	0
03:00 PM	11	4	0	34	98	2	38	4	0	189	2
03:15 PM	12	5	0	32	119	3	40	5	0	213	3
03:30 PM	7	10	0	21	85	5	45	10	0	178	5
03:45 PM	7	13	0	27	112	0	56	6	0	221	0
04:00 PM	6	18	0	28	108	4	52	3	0	215	4
04:15 PM	6	16	0	26	91	5	33	3	0	175	5
04:30 PM	8	10	0	26	107	7	40	0	0	191	7
04:45 PM	14	11	0	80	96	6	17	1	0	219	6
05:00 PM	10	6	0	76	96	1	18	7	0	213	1
05:15 PM	9	12	0	82	91	4	15	1	0	210	4
05:30 PM	6	11	0	70	53	1	15	4	0	159	1
	-		-	-			-		-		

File Name: C:\Petra\Green\Download 2\Alt 1 1 1 c 2 - Barta Road with Heller Road_PM.ppd

Start Date: 4/7/2021

Start Time: 6:00:00 AM

Site Code: 00338202

Comment 1: Default Comments Comment 2: Change These in The Preferences Window

Comment 3: Select File/Preference in the Main Scree Comment 4: Then Click the Comments Tab

Con	iment 4:		K the Cor	nments	ар								-	
		Barta				Heller				Barta				
Chart		From	East			From	South			From	vvest			
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Tot. Veh.	Tot. Ped.
06:00 AM	0	65	0	0	10	0	3	0	0	14	2	0	94	0
06:15 AM	0	68	0	0	12	0	1	0	0	11	3	0	95	0
06:30 AM	0	63	0	0	6	0	0	0	0	12	1	0	82	0
06:45 AM	1	83	0	0	12	0	1	0	0	11	1	0	109	0
07:00 AM	0	73	0	0	4	0	0	0	0	6	0	0	83	0
07:15 AM	0	96	0	0	11	0	0	0	0	9	0	0		0
07:30 AM	1	76	0	0	14	0	1	0	0	16	1	0		0
07:45 AM	0	76	0	0	17	0	0	0	0	10	1	0		0
08:00 AM	1	80	0	0	12	0	3	0	0	12	0	0		0
08:15 AM	0	77	0	0	17	0	0	0	0	10	0	0		0
08:30 AM	2	76	0	0	18	0	1	0	0	18	1	0		0
08:45 AM	0	59	0	0	12	0	0	0	0	11	1	0		0
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0		0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0		0
09:30 AM	0	0	0	0	0	0	0	0	0	0	0	0		0
09:45 AM	0	0	0	0	0	0	0	0	0	0	0	0		0
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0		0
10:15 AM 10:30 AM	0	0	0	0	0	0	0	0	0	0	0	0 0		0 0
10:30 AM 10:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	-	0
	0	0	0	0	0	0	0	0	0	0	0	0		0
11:00 AM 11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0		0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0		0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0		0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0		0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0		0
12:30 PM	0	0	0	0	Ő	Ő	0	0	0	Ő	Ő	0		0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0		0
01:00 PM	0	0	0	0	0	0	0	0	0	0	0	0		0
01:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0		0
03:00 PM	5	9	0	0	2	0	2	1	0	96	62	0		1
03:15 PM	6	15	0	0	1	0	1	0	0	74	56	0		0
03:30 PM	4	16	0	0	1	0	2	1	0	71	52	1	146	2
03:45 PM	5	15	0	0	0	0	1	0	0	68	43	0		0
04:00 PM	2	21	0	0	1	0	5	0	0	84	73	1	186	1
04:15 PM	4	20	0	0	2	0	1	1	0	89	70	0		1
04:30 PM	3	17	0	0	2	0	3	0	0	80	69	0		0
04:45 PM	2	11	0	0	0	0	3	0	0	58	57	0		0
05:00 PM	3	16	0	0	2	0	3	2	0	79	54	0		2
05:15 PM	4	10	0	0	2	0	3	0	0	72	44	0		0
05:30 PM	4	15 9	0	0	2 0	0	4	0	0	48 45	47	0 1		0 2
05:45 PM 06:00 PM	1	9	0	0	0	0	2	1	0	45 0	46 0	1		2
	0	0	0	0	0	0	0	0	0	0	0	0	0	0

File Name: C:\Petra\Green\2021_03_23_Alt 1_Loc 3_Barta Road with Backlick Road.ppd

Start Date: 3/23/2021

Start Time: 5:45:00 AM

Site Code: 00000000

Comment 1: Default Comments

Comment 2: Change These in The Preferences Window

Comment 3: Select File/Preference in the Main Scree

Comment 4: Then Click the Comments Tab

	Bi	acklick Roa	ad		acklick Roa	d		Barta Road		ľ	
		From North			From South	-		From West			
Start											
Time	Thru	Right	Peds	Left	Thru	Peds	Left	Right	Peds	Tot. Veh.	Tot. Ped.
05:45 AM	27	29	0	55	78	0	9	1	0	199	0
06:00 AM	19	24	0	43	102	0	6	0	0	194	0
06:15 AM	34	24	0	38	132	0	5	5	0	238	0
06:30 AM	34	41	0	50	170	0	5	0	0	300	0
06:45 AM	54	32	0	47	139	0	8	1	0	281	0
07:00 AM	51	39	0	39	180	0	10	1	0	320	0
07:15 AM	70	43	0	58	156	0	10	6	0	343	0
07:30 AM	50	31	0	44	182	0	15	2	0	324	0
07:45 AM	46	38	0	45	170	0	8	3	0	310	0
08:00 AM	52	50	0	50	148	0	17	1	0	318	0
08:15 AM	51	29	0	53	138	0	5	3	0	279	0
08:30 AM	65	22	0	42	143	0	13	7	0	292	0
08:45 AM	58	25	0	45	140	0	10	6	0	284	0
09:00 AM	0	0	0	0	0	0	0	0	0	0	0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0
09:30 AM	0	0	0	0	0	0	0	0	0	0	0
09:45 AM	0	0	0	0	0	0	0	0	0	0	0
10:00 AM	0	0	0	0	0	0	0	0	0	0	0
10:15 AM	0	0	0	0	0	0	0	0	0	0	0
10:30 AM	0	0	0	0	0	0	0	0	0	0	0
10:45 AM	0	0	0	0	0	0	0	0	0	0	0
11:00 AM	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0
01:00 PM	0	0	0	0	0	0	0	0	0	0	0
01:15 PM	0	0	0	0	0	0	0	0	0	0	0
01:30 PM	0	0	0	0	0	0	0	0	0	0	0
01:45 PM	0	0	0	0	0	0	0	0	0	0	0
02:00 PM	0	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	0	0	0	0	0	0	0	0	0
02:30 PM	0	0	0	0	0	0	0	0	0	0	0
02:45 PM	105	10	0	9	135	0	53	17	0	329	0
03:00 PM	112	7	0	9	152	0	73	19	0	372	0
03:15 PM	116	8	0	2	149	0	69	21	0	365	0
03:30 PM	122	10	0	9	140	0	71	13	0	365	0
03:45 PM	96	16	0	14	143	0	60	18	0	347	0
04:00 PM	152	10	0	8	161	0	89	21	0	441	0
04:15 PM	123	15	0	4	170	0	59	16	0	387	0
04:30 PM	137	16	0	3	177	0	78	15	0	426	0
04:45 PM	141	10		9	194	0	56	17	0	427	0
05:00 PM	124	6	0	9	170	0	48	8	0	365	0
05:15 PM	135	10	0	6	169	0	54	14	0	388	0
05:30 PM	112	14		1	160	0	30	6	0	323	0
05:45 PM	115	12	0	1	157	0	27	8	0	320	0

Fi	le Name:	C:\Pet		021_03_24_Alt 1_Loca 4-5_ Barta							
St	art Time:	Road with VA 286 Ramps.ppd t Date: 3/24/2021 t Time: 5:45:00 AM Code: 4-5									
			Dente M								
		NB Thru :									
				3 to VA 286 NB Ramp							
Cor	mment 3:	SB LT = I	Barta WB	to VA 286 SB Ramp							
Cor	mment 4:										
	Barta	Barta Ro	oad WB								
	Road WB	From	South								
Start Time	Left	Thru	Right								
05:45 AM	6	19	2								
06:00 AM	1	20	0								
06:15 AM	2	6	2								
06:30 AM	2	1	3								
06:45 AM	4	2	2								
07:00 AM	2	0	1								
07:15 AM	3	1	1								
07:30 AM	0	2	6								
07:45 AM	1	0	2								
08:00 AM	0	0	3								
08:15 AM	1	6	4								
08:30 AM	2	4	1								
08:45 AM	0	0	0								
09:00 AM	0	0	0								
09:15 AM	0	0	0								
09:30 AM	0	0	0								
09:45 AM	0	0	0								
10:00 AM	0	0	0								
10:15 AM	0	0	0								
10:30 AM	0	0	0								
10:45 AM	0	0	0								
11:00 AM	0	0	0								
11:15 AM	0	0	0								
11:30 AM	0	0	0								
11:45 AM	0	0	0								
12:00 PM	0	0	0								
12:15 PM	0	0	0								
12:30 PM	0	0	0								
12:45 PM	0	0	0								
01:00 PM	0	0	0								
01:15 PM	0	0	0								
01:30 PM	0	0	0								
01:45 PM 02:00 PM	0 0	0 0	0 0								
02:00 PM 02:15 PM	0	0	0								
02.15 PM 02:30 PM	0	0	0								
02:30 PM 02:45 PM	0	0	0								
02.45 PM 03:00 PM	34	0 25	0 64								
03:00 PM 03:15 PM	34 25	25 13	64 47								
03.15 PM 03:30 PM	25 35	21	47 64								
03:45 PM	33	13	89								
00. 4 0 F IVI	52	13	09								

Fi	le Name:	C:\Pet	•	021_03_24_Alt 1_Loca 4-5_ Barta with VA 286 Ramps.ppd
S	tart Date:	3/24/202	1	
St	art Time:	5:45:00 A	M	
S	ite Code:	4-5		
Coi	mment 1:	NB Thru	= Barta W	/B Thru
Coi	mment 2:	NB RT =	Barta WE	B to VA 286 NB Ramp
				to VA 286 SB Ramp
	mment 4:			
	Barta	Barta Ro	oad WB	
	Road WB	From	South	
Start Time	Left	Thru	Right	
04:00 PM	26	26	75	
04:15 PM	32	23	74	
04:30 PM	32	19	72	
04:45 PM	22	25	70	
05:00 PM	20	21	67	
05:15 PM	22	16	55	
05:30 PM	16	16	40	
05:45 PM	19	13	36	

Location:	Location 6 Heller Road with I-95 NB/I-95 SB Express Lane
Counter ID:	Counter D
Scheme:	FHWA
Notes:	Counter set 3/22/21 (approximately 7 PM) and picked up after 3/23/21 PM Count (approximately 7 PM)

Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9	Class #10	Class #11	Class #12	Class #13
3/23/2021 0:00	2	0	2	0	0	0	0	0	0	0				0
3/23/2021 0:15	3	0			0	0	0		0	0	-			
3/23/2021 0:30 3/23/2021 0:45	3	0	3	0	0	0	0	0	0	0	-	-	-	0
3/23/2021 0:43	2	0	2	0	0	0	0	-	0	0	-	-		0
3/23/2021 1:15	0	0	0	0	0	0	0	-	0	0	0	-	-	0
3/23/2021 1:30	2	0	1	1	0	0	0	0	0	0	-	-	-	0
3/23/2021 1:45 3/23/2021 2:00	0	0	0	0	0	0	0		0	0				
3/23/2021 2:15	0	0	0	-	0	0	0	-	0	0	-	-	-	-
3/23/2021 2:30	0	0	0	0	0	0	0	-	0	0	-			
3/23/2021 2:45	0	0	0	0	0	0	0	-	0	0	-	-	-	-
3/23/2021 3:00 3/23/2021 3:15	0	0	0	-	0	0	0		0	0	-			
3/23/2021 3:30	0	0	-		0	0	0	-	0	0	-			-
3/23/2021 3:45	0		0	-	0	0	0	-	0	0	-	-		-
3/23/2021 4:00 3/23/2021 4:15	0	0	0	0	0	0	0	0	0	0	-			0
3/23/2021 4:15	0	0	-	-	0	0	0	-	0	-	-	-	-	-
3/23/2021 4:45	0		0		0	0	0		0	0				
3/23/2021 5:00	0	0	0	0	0	0	0	-	0	0	-	-		
3/23/2021 5:15 3/23/2021 5:30	0	0	0	0	0	0	0	0	0	0	-			-
3/23/2021 5:30	0	0	0	-	0	0	0	-	0	0	-	-	-	-
3/23/2021 6:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 6:15	0	0	-	-	0	0	0	-	0	0	-	-		
3/23/2021 6:30 3/23/2021 6:45	0		0	0	0	0	0	-	0	0	-	-	-	-
3/23/2021 0.43	0	0	0	0	0	0	0	0	0	0	-	-		0
3/23/2021 7:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 7:30	0	0	0	-	0	0	0	-	0	0	-	-	-	-
3/23/2021 7:45 3/23/2021 8:00	0	0	0	0	0	0	0	-	0	0	-	-		0
3/23/2021 8:15	0		0	0	0	0	0		0	0	-			÷
3/23/2021 8:30	0	0	0	0	0	0	0	0	0	0	0			0
3/23/2021 8:45	0	0	0	0	0	0	0	0	0	0	-			
3/23/2021 9:00 3/23/2021 9:15	0	0	0	-	0	0	0	-	0	0	-	-	-	-
3/23/2021 9:30	0	0	0	-	0	0	0		0	0	-			
3/23/2021 9:45	0	0	0	0	0	0	0	0	0	0	-	-		0
3/23/2021 10:00 3/23/2021 10:15	0	0	0	-	0	0	0	-	0	0	-	-		
3/23/2021 10:15	0	0	0	0	0	0	0	0	0	0				0
3/23/2021 10:45	0	0	0	-	0	0	0	-	0	-	-	-	-	-
3/23/2021 11:00	0		0	-	0	0	0	-	0	-	-	-		-
3/23/2021 11:15 3/23/2021 11:30	0	0	0	0	0	0	0	-	0	0	-	-		0
3/23/2021 11:45	0	0	0	0	0	0	0	-	0	0	-			
3/23/2021 12:00	0	0	0	0	0	0	0	0	0	0	0			0
3/23/2021 12:15	0	0	0	0	0	0	0	-	0	0	-	-		0
3/23/2021 12:30 3/23/2021 12:45	0	0	0	0	0	0	0	-	0	0	÷	-		0
3/23/2021 12:45	0		-		0	0	-	-	0	-	-			
3/23/2021 13:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 13:30	0	0	0		0	0	0	-	0	0				
3/23/2021 13:45 3/23/2021 14:00	0	0	0		0	0	0	-	0		-			
3/23/2021 14:00	10	0			0	0	0		0	-	-	-		
3/23/2021 14:30	11	0	11	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 14:45	10	0	10	0	0	0	0		0					
3/23/2021 15:00 3/23/2021 15:15	13 42	0	10 37	3	0	0	0	-	0	-	-	-	-	-
3/23/2021 15:30	42	0	11	4	0	0	0		0		-			
3/23/2021 15:45	35	0	35	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 16:00	60	0	52	7	0	1	0	0	0	0	-	-		
3/23/2021 16:15 3/23/2021 16:30	64 45	2	58 41	4	0	0	0		0	0				
3/23/2021 16:45	43	0	41	7	0	0	0		0	0				
3/23/2021 17:00	37	0	33	4	0	0	0		0					
3/23/2021 17:15	68	1	64	3	0	0	0	-	0	-	-	-	-	-
3/23/2021 17:30 3/23/2021 17:45	59 65	0	54 59	5	0	0	0		0	-	-	-		
3/23/2021 17:43	54	1	50	3	0	0	0	-	0	0	-			
3/23/2021 18:15	87	0	79	8	0	0	0		0	0	-	0	0	0
3/23/2021 18:30	71	2	64	3	1	1	0	-	0		-	-		
3/23/2021 18:45 3/23/2021 19:00	55 62	0 0		5 5	0 Đ	0 0	0 0		0 0		-			
37 237 2021 13.00	02	U 0	3/	÷	U 0	U U	- -	U U	U 0	I A	1 4	I A	I A	L A

Location:	Location 6 Heller Road with I-95 NB/I-95 SB Express Lane
Counter ID:	Counter D
Scheme:	FHWA
Notes:	Counter set 3/22/21 (approximately 7 PM) and picked up after 3/23/21 PM Count (approximately 7 PM)

Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9	Class #10	Class #11	Class #12	Class #12
3/23/2021 19:15				Class #5			Class #0		Class #0					Class #15
	66	÷.	64	£	0	0	÷.	0	÷.	0	0	÷.	0	÷
3/23/2021 19:30	53	1	48	4	0	0	0	0	0	0	0	0	0	0
3/23/2021 19:45	45	1	40	4	0	0	0	0	0	0	0	0	0	0
3/23/2021 20:00	33	θ	32	1	θ	θ	θ	θ	θ	θ	θ	θ	θ	θ
3/23/2021 20:15	33	θ	30	3	Ð	0	θ	0	θ	0	0	θ	0	0
3/23/2021 20:30	2 4	θ	24	0	Ð	0	θ	0	θ	0	0	θ	0	0
3/23/2021 20:45	θ	θ	θ	θ	θ	θ	θ	θ	θ	θ	θ	θ	θ	θ
3/23/2021 21:00	Ð	Ð	θ	0	Ð	θ	Ð	Ð	Ð	0	Ð	θ	0	Ð
3/23/2021 21:15	0	θ	θ	0	Ð	θ	θ	0	θ	0	0	θ	0	0
3/23/2021 21:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 21:45	0	0	θ	0	Ð	0	0	0	0	0	0	θ	0	0
3/23/2021 22:00	0	θ	θ	0	Ð	0	θ	0	θ	0	0	θ	0	0
3/23/2021 22:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 22:30	Ð	Ð	θ	0	0	θ	Ð	Ð	Ð	0	Ð	θ	0	Ð
3/23/2021 22:45	0	θ	θ	0	Ð	0	θ	0	θ	0	0	θ	0	0
3/23/2021 23:00	0	θ	θ	0	Ð	0	θ	0	θ	0	0	θ	0	θ
3/23/2021 23:15	θ	θ	θ	θ	θ	θ	θ	θ	θ	θ	θ	θ	θ	θ
3/23/2021 23:30	Ð	θ	θ	0	Ð	0	0	0	θ	0	0	θ	0	θ
3/23/2021 23:45	0	0	Ð	0	0	0	0	0	0	0	0	9	0	Ð

Location 7_Heller Road with I-95 SB_Channelized RT Lane
Counter A
FHWA
Counter picked up after 3/23/21 PM Count (approximately 7 PM)

Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9	Class #10	Class #11	Class #12	Class #13
3/23/2021 0:00	0		-	-	-	0			-		-		-	-
3/23/2021 0:15 3/23/2021 0:30	1	-		0	-	-			-	-				-
3/23/2021 0:45	0	-	-						-					
3/23/2021 1:00	0	-	-	-	-	-	-	-	-	-	-		0 0	-
3/23/2021 1:15 3/23/2021 1:30				0					-				0 0	-
3/23/2021 1:45				0			-	-	-					
3/23/2021 2:00	1	-		0	-	-			-	-	-		-	-
3/23/2021 2:15					-		-		-	-			0 0	
3/23/2021 2:30 3/23/2021 2:45			-	0	-	-		-	Ţ	-				-
3/23/2021 3:00		-		-	-	-			-	-			0 0	-
3/23/2021 3:15			-			-		-	Ţ	-			0 0	
3/23/2021 3:30 3/23/2021 3:45	5		-	0	0	-	-		-	-	-			-
3/23/2021 3:43	7	-		0	-	-	-	-	-	-	-			-
3/23/2021 4:15				1	0	0	-		-	-	-		-	
3/23/2021 4:30 3/23/2021 4:45	9		-	-	-	-	-		-	-			0 0	-
3/23/2021 4:45	9		-						-				-	
3/23/2021 5:15	11		9	1			-						0 0	
3/23/2021 5:30 3/23/2021 5:45	11			0	-	-	-		Ţ	-	-		0 0	
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3/23/2021 6:15	16	0	15	1	0	0	0	0 0	0 0	0	0 0) (0 0	0 0
3/23/2021 6:30	11	-		0	-	-	-	-	-	-	-		0 0	
3/23/2021 6:45 3/23/2021 7:00	13 10			2									0 0	
3/23/2021 7:00			-		-	-	-		-	-	-			
3/23/2021 7:30				1					-				0 0	
3/23/2021 7:45 3/23/2021 8:00				0	-	-	-		-	-	-			-
3/23/2021 8:00		-		1	-	-	-	-	-	-			-	
3/23/2021 8:30	32		-	1	-	-	-	-	-	-	-		0 0	
3/23/2021 8:45	14				-	-		-	-	-			0 0	-
3/23/2021 9:00 3/23/2021 9:15	17			0										
3/23/2021 9:30		-	-	-	-	-		-	-	-	-		0 0	-
3/23/2021 9:45	10					-			-				-	
3/23/2021 10:00 3/23/2021 10:15	10		-	1	-	-	-	-	-	-	-		0 0	-
3/23/2021 10:30	11			0	-				-				-	
3/23/2021 10:45	12			1	-	-	-		-	-	-		-	-
3/23/2021 11:00 3/23/2021 11:15	9 10					-			-	-	-			
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3/23/2021 11:45	5												0 0	
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3/23/2021 12:30	8		-	1	0	-	-	-	-	-	-			
3/23/2021 12:45	5		-		-	-	-	-	-	-	-		0 0	-
3/23/2021 13:00	2				-	-		-	-	-			0 0	-
3/23/2021 13:15 3/23/2021 13:30									-					-
3/23/2021 13:45	1	0	1	0	0	0	C	0 0	0 0	0	0 0) (0 0) 0
3/23/2021 14:00		-			-				-				0 0	
3/23/2021 14:15 3/23/2021 14:30					-				-				0 0	
3/23/2021 14:45	1	0	1	0	0	0	C	0 0	0 0	0	0 0) (0 0	0 0
3/23/2021 15:00					-				-				0 0	
3/23/2021 15:15 3/23/2021 15:30	-				-	-			-	-	-		0 0	-
3/23/2021 15:45	10				-	-			-	-				-
3/23/2021 16:00						-			-	-			0 0	-
3/23/2021 16:15 3/23/2021 16:30					-								0 0	
3/23/2021 16:30					-				-					
3/23/2021 17:00	3												0 0	
3/23/2021 17:15 3/23/2021 17:30		-		0	-				-				0 0	
3/23/2021 17:30		-	-	-	-	-			-	-				-
3/23/2021 18:00	0	0	0	0	0	0	C	0 0	0	0	0 0) (0 0	0 0
3/23/2021 18:15														
3/23/2021 18:30 3/23/2021 18:45		-			0 0	6 6			-	-			-	-
3/23/2021 18:45 3/23/2021 19:00	-	-			-				-	-	-			
3/23/2021 19:00 3/23/2021 19:15	-													
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Location:	Location 7_Heller Road with I-95 SB_Channelized RT Lane
Counter ID:	Counter A
Scheme:	FHWA
Notes:	Counter picked up after 3/23/21 PM Count (approximately 7 PM)

Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9	Class #10	Class #11	Class #12	Class #13
3/23/2021 19:30	θ	θ	0	Ð	0	Ð	0	0	0	0	Ð	Ð	0	0
3/23/2021 19:45	θ	θ	θ	θ	θ	Ð	θ	θ	θ	θ	θ	θ	θ	0
3/23/2021 20:00	θ	θ	0	Ð	0	Ð	0	0	0	0	Ð	Ð	0	0
3/23/2021 20:15	0	0	0	0	0	0	0	0	0	0	Ð	Ð	0	0
3/23/2021 20:30	0	0	0	θ	Ð	0	θ	θ	0	θ	Ð	Ð	θ	0
3/23/2021 20:45	0	0	0	0	0	0	0	0	0	0	Ð	Ð	0	0
3/23/2021 21:00	0	0	0	0	0	Ð	0	0	0	0	Ð	Φ	0	0
3/23/2021 21:15	0	0	0	0	0	0	0	0	0	Ð	Ð	θ	0	0
3/23/2021 21:30	0	0	0	0	0	0	0	0	0	0	Ð	Ð	0	0
3/23/2021 21:45	θ	θ	θ	θ	θ	Ð	θ	θ	θ	θ	θ	θ	θ	0
3/23/2021 22:00	0	0	0	0	0	Ð	0	0	0	0	Ð	Φ	0	0
3/23/2021 22:15	0	0	0	0	0	Ð	0	0	0	0	Ð	Φ	0	0
3/23/2021 22:30	0	0	0	0	0	0	0	0	0	Ð	Ð	θ	0	0
3/23/2021 22:45	0	0	0	0	0	Ð	0	0	0	0	Ð	Φ	0	0
3/23/2021 23:00	9	0	0	0	0	Ð	0	9	0	0	Ð	Ð	0	0
3/23/2021 23:15	0	0	0	0	0	0	0	0	0	0	Ð	0	0	0
3/23/2021 23:30	0	0	0	0	0	0	0	0	0	0	Ð	Ð	0	0
3/23/2021 23:45	θ	θ	0	θ	θ	Ð	0	0	θ	θ	θ	θ	θ	θ

Location: Counter ID:	Location 7_ Heller Road with I-95 SB_Left Turns Counter B
Scheme:	FHWA
Notes:	Counter set 3/22/21 (approximately 7 PM) and picked up after 3/23/21 PM Count (approximately 7 PM)

Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9			Class #12	
3/23/2021 0:00	0	-	-	-	-	0	-	0	÷		-	0	0	-
3/23/2021 0:15 3/23/2021 0:30	0	-	-	-	-	-	-		-	-		0	-	-
3/23/2021 0:30	0		-		-				-			0	÷	
3/23/2021 1:00	0				0							0	0	
3/23/2021 1:15	0						-		-			0		
3/23/2021 1:30 3/23/2021 1:45	0	-			-	-			-	-		0	-	-
3/23/2021 1:45	0	-	-		-	-	-		-			0	-	
3/23/2021 2:15			-	-	-				-			0		
3/23/2021 2:30									-			0	-	
3/23/2021 2:45 3/23/2021 3:00	0		-		-				-			0	-	-
3/23/2021 3:00	0	-	-	-	-	-	-	-	-	-	-	0	-	-
3/23/2021 3:30									-			0	-	
3/23/2021 3:45	0	-	-	-	-	-	-	-	-	-	-	0	-	
3/23/2021 4:00	0								-			0	-	
3/23/2021 4:15 3/23/2021 4:30	0		-		-		-		-	-		0	-	-
3/23/2021 4:45	0	-	-	-	-	-	-	-	-	-	-	0	-	
3/23/2021 5:00	0	0	0	0	0	0	0	0	0	0	0	0	0	
3/23/2021 5:15		-	-	-	-	-	-	-	-	-	-	0	÷	-
3/23/2021 5:30 3/23/2021 5:45	0								-			0		
3/23/2021 5:45	-	-	-	-	-	-	-	-	-	-	-	0	-	
3/23/2021 6:15	0		-		-				-			0	÷	
3/23/2021 6:30	0	-	-	-	-	-	-	-	-	-	-	0	-	
3/23/2021 6:45			-		-	-		-	-	-	-	0	÷	-
3/23/2021 7:00 3/23/2021 7:15	0					-			-			0	-	
3/23/2021 7:30	0	-	-	-	-	-	-	-	-	-	-	0	-	
3/23/2021 7:45	0			0	0	0	0	0	0	0	0	0	0	0
3/23/2021 8:00			-		-		-		-	-	-	0	-	-
3/23/2021 8:15 3/23/2021 8:30	0		-		-							0	-	
3/23/2021 8:45	0		-	-	-	-	-		-	-		0	-	
3/23/2021 9:00					-				-			0	-	
3/23/2021 9:15	0	-	-	-	-	-	-	-	-	-	-	0	-	
3/23/2021 9:30 3/23/2021 9:45	0		-		-	-	-		-			0	-	-
3/23/2021 9.45	0		-					-	-			0	-	
3/23/2021 10:15	0	-	-	-	-	-	-		-	-	-	0	-	
3/23/2021 10:30	0		-		-		-		-			0	-	
3/23/2021 10:45	0		-	-	-	-	-	-	-	-	-	0	÷	-
3/23/2021 11:00 3/23/2021 11:15	0								-			0	-	-
3/23/2021 11:30		-		-			-		-	-		0	-	
3/23/2021 11:45	0	-			-	-			-	-		0	-	-
3/23/2021 12:00	0	-	-	-	-		-		-	-	-	0	-	-
3/23/2021 12:15 3/23/2021 12:30	0	-	-		-	-	-		÷			0	÷	
3/23/2021 12:45	0				-	-			-			0	-	
3/23/2021 13:00	0	-	-	-	-	-	-	-	-	-	-	0	-	-
3/23/2021 13:15		1	1		-				-			0	-	-
3/23/2021 13:30 3/23/2021 13:45														
3/23/2021 13:43	0								-			0		
3/23/2021 14:15									-				-	
3/23/2021 14:30		-			-				-			-	-	-
3/23/2021 14:45 3/23/2021 15:00	-	-	-	-	-	-	-		-	-		-	-	-
3/23/2021 15:00									-					
3/23/2021 15:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 15:45									-			-		
3/23/2021 16:00 3/23/2021 16:15	-	-	-	-	-		-		-	-	-	-	-	-
3/23/2021 16:15 3/23/2021 16:30					-		-		-	-		0	-	
3/23/2021 16:45									-					
3/23/2021 17:00	-	-	-		-	-		-	-	-		-	-	-
3/23/2021 17:15									-			0		
3/23/2021 17:30 3/23/2021 17:45		-	-	-	-		-	-	-	-	-	-	-	-
3/23/2021 17:43									-			0	-	
3/23/2021 18:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 18:30			-			1			-	-	-		-	-
3/23/2021 18:45 3/23/2021 19:00									-			0	-	-
3/23/2021 19:00														
5/25/2021 15:15	L A		- -	1 4	- -	U U	г –	- A	l A	1 4	U U	U U	- -	• •

Location:	Location 7_ Heller Road with I-95 SB_Left Turns
Counter ID:	Counter B
Scheme:	FHWA
Notes:	Counter set 3/22/21 (approximately 7 PM) and picked up after 3/23/21 PM Count (approximately 7 PM)

Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9	Class #10	Class #11	Class #12	Class #13
3/23/2021 19:30	1	θ	0	0	θ	1	0	0	0	Ð	Ð	θ	θ	θ
3/23/2021 19:45	3	θ	θ	1	1	1	θ	θ	θ	θ	θ	θ	θ	θ
3/23/2021 20:00	1	θ	0	1	θ	Ð	0	0	0	Ð	Ð	θ	θ	θ
3/23/2021 20:15	2	0	0	0	0	2	0	0	0	0	Ð	Ð	0	0
3/23/2021 20:30	59	θ	0	20	1	38	0	0	0	θ	Ð	Ð	0	0
3/23/2021 20:45	43	0	0	20	0	23	0	0	0	0	Ð	Ð	0	0
3/23/2021 21:00	48	Φ	0	22	0	26	0	Φ	0	0	Ð	Φ	0	0
3/23/2021 21:15	47	Φ	0	26	1	20	0	Φ	0	0	Ð	θ	0	0
3/23/2021 21:30	48	Φ	0	33	0	15	0	Φ	0	0	Ð	Φ	0	0
3/23/2021 21:45	44	θ	θ	33	θ	11	0	0	θ	0	θ	θ	θ	0
3/23/2021 22:00	51	Φ	0	34	1	16	0	Φ	0	0	Ð	Φ	0	0
3/23/2021 22:15	39	Φ	0	13	0	26	0	Φ	0	0	Ð	Φ	0	0
3/23/2021 22:30	31	Φ	0	9	0	22	0	Φ	0	0	Ð	θ	0	0
3/23/2021 22:45	12	Φ	0	4	0	ᅇ	0	Φ	0	0	Ð	Φ	0	0
3/23/2021 23:00	9	0	0	2	0	7	0	0	0	0	0	Φ	0	0
3/23/2021 23:15	1	Φ	0	0	0	1	0	Φ	0	0	Ð	θ	0	0
3/23/2021 23:30	0	0	0	0	0	0	0	0	0	0	Ð	Ð	0	0
3/23/2021 23:45	2	0	0	0	1	1	0	0	0	θ	Ð	θ	0	0

Location:	Location 8_Heller Road with NGA South Gate_Outbound
Counter ID:	Counter A
Scheme:	FHWA
Notes:	Counter set after 3/23/21 PM Count (approximately 7 PM) and picked up after 3/24/21 PM Count (approximately 7 PM)
Notes:	Counter set after 3/23/21 PM Count (approximately 7 PM) and picked up after 3/24/21 PM Count (approximately 7 PM)

Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9	Class #10	Class #11	Class #12	Class #13
3/24/2021 0:00	0			0	0	0	C	0 0	0 0	0	0	0	0	0
3/24/2021 0:15									-					-
3/24/2021 0:30		-	-		-	-	-		-	-		-	-	-
3/24/2021 0:45 3/24/2021 1:00		-	-	-	-	-	-	-	-	-	-	-	-	-
3/24/2021 1:15					-	-								-
3/24/2021 1:30		0	0	0	0	0	C	0 0	0	0	0	0	0	
3/24/2021 1:45									-					
3/24/2021 2:00		-	-	-	-	-	-	-	-	-	-	-	-	-
3/24/2021 2:15 3/24/2021 2:30			-		-				-	-		-	-	
3/24/2021 2:30	0					-			-			-		-
3/24/2021 3:00	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3/24/2021 3:15	0	0 0	0	0	0	0	C	0 0	0 0	0	0	0	0	0
3/24/2021 3:30			-		-	-			-	-		-	-	-
3/24/2021 3:45										-				
3/24/2021 4:00 3/24/2021 4:15					-		-		-	-		-		-
3/24/2021 4:30	-	-	-		-	-			-	-		-	-	
3/24/2021 4:45					0		C		0			0		
3/24/2021 5:00	0	0 0	0	0	0	0	C	0 0	0 0	0	0	0	0	0
3/24/2021 5:15						1			-	-			-	-
3/24/2021 5:30		-	-	-	-		-		-	-		-	-	-
3/24/2021 5:45	1				-				-	-		-	-	-
3/24/2021 6:00 3/24/2021 6:15	1			-	-		-	-	-	-		-	-	-
3/24/2021 6:30		-	-	-	-		-	-		-				-
3/24/2021 6:45	1	0	0	0	1	0	C	0 0	0 0	0	0	0	0	0
3/24/2021 7:00			-	-	-	-			÷	-		-	-	-
3/24/2021 7:15					-	-			-	-		-		-
3/24/2021 7:30			-		-				-	-		-	-	-
3/24/2021 7:45 3/24/2021 8:00		-	-			-			-	-		-	-	-
3/24/2021 8:00	1		-	-	-	-		-	-	-	-	-	-	-
3/24/2021 8:30		-	-		-				-	-		-	-	-
3/24/2021 8:45	1	0	0	0	0	1	C	C	0 0	0	0	0	0	0
3/24/2021 9:00			-		-		-		-	-		-	-	-
3/24/2021 9:15			-			-			-			-	-	
3/24/2021 9:30				0	-		-		-	-	-	-	-	-
3/24/2021 9:45 3/24/2021 10:00										-				
3/24/2021 10:15		-	-		-				÷	-		-	-	-
3/24/2021 10:30	1	-	-	-	-		-	-	-	-	-	-	-	-
3/24/2021 10:45	1		-	0	0	1	C	0 0	0 0	0	0	0	0	-
3/24/2021 11:00			-	-	-	-			-	-		-	-	-
3/24/2021 11:15 3/24/2021 11:30	1						-							
3/24/2021 11:45			-	-	-	-	-		-	-		-	-	-
3/24/2021 12:00		-	-	-	-		-	-	-			-	-	
3/24/2021 12:15														
3/24/2021 12:30			-	-	-			-	-	-		-	-	-
3/24/2021 12:45									-					-
3/24/2021 13:00	0	-	-	-	-	-	-	-	-	-	-	-	-	-
3/24/2021 13:15 3/24/2021 13:30					1									
3/24/2021 13:30	0													
3/24/2021 14:00	-	0	0	-	-	-	-	-	-	0	0	0	-	-
3/24/2021 14:15									-					-
3/24/2021 14:30	-	-			-	-	-		-	-		-	-	-
3/24/2021 14:45 3/24/2021 15:00					-									
3/24/2021 15:00	-				-				-			-		-
3/24/2021 15:30					-				-			-		-
3/24/2021 15:45									0					
3/24/2021 16:00	0	-			-				-	0	0	-		-
3/24/2021 16:15														
3/24/2021 16:30	-		-	-	-	-			-			-	-	-
3/24/2021 16:45 3/24/2021 17:00		-	-		-		-		-	-		-		-
3/24/2021 17:00					-				-					
3/24/2021 17:30		-	-	-	-				-			-	-	-
3/24/2021 17:45	0	0 0	0	0	-	0	C		-	0	0			0
3/24/2021 18:00			-	-	-							-	-	
3/24/2021 18:15		-			-				-	-		-		-
3/24/2021 18:30					-	-	-		-	-		-	-	-
3/24/2021 18:45 3/24/2021 19:00			-	-	-				÷			-	-	-
3/24/2021 19:00		-							-	-		-		-
3,2 1/2021 10.10	. 0	. 0	. 0		. 0					. 0	. 0	. 0	. 0	. J

Location:	Location 8_Heller Road with NGA South Gate_Outbound
Counter ID:	Counter A
Scheme:	FHWA
Notes:	Counter set after 3/23/21 PM Count (approximately 7 PM) and picked up after 3/24/21 PM Count (approximately 7 PM)

Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9	Class #10	Class #11	Class #12	Class #13
3/24/2021 19:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 19:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 20:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 20:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 20:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 20:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 21:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 21:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 21:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 21:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 22:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 22:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 22:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 22:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 23:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 23:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 23:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 23:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Location:	Location 8_Heller Road with NGA South Gate_Inbound
Counter ID:	Counter B
Scheme:	FHWA
Notes:	Counter set after 3/23/21 PM Count (approximately 7 PM) and picked up after 3/24/21 PM Count (approximately 7 PM)

3442901 0 0 0 0 <th>Data /Time</th> <th>Maluma</th> <th>Class #1</th> <th>Class #2</th> <th>Class #2</th> <th>Class #4</th> <th>Class #F</th> <th>Class #C</th> <th>Class #7</th> <th>Class #0</th> <th>Class #0</th> <th>Class #10</th> <th>Class #11</th> <th>Class #12</th> <th>Class #12</th>	Data /Time	Maluma	Class #1	Class #2	Class #2	Class #4	Class #F	Class #C	Class #7	Class #0	Class #0	Class #10	Class #11	Class #12	Class #12
3242001 0.5 0 0 0		Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9				Class #13
38-42001 0.50 <						-	-		-		-	-	-	-	0
3242001 10 0 0 0 0 </td <td></td>															
3242091 116 1 0 0 0	3/24/2021 0:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3242891130 2 0 0 0 0	3/24/2021 1:00	0						-	-			0			0
3242001 146 0 0 0			-	-	-	-		-	-	-	-	-	-	-	-
3242071 200 1 0 0 0															0
3824201136 0 0 0 0				-	-	-	-	-	-	-	-	_			0
392-2021 30 1 0 0 0 0<			-	-	-	-		-	-	-	-	_	-		-
3242021 345 0 0 0					-		-			-	-	_			-
3242021315 1 0		0				0	0	0	0	0	0	0		0	
3242021 3.30 1 0 <t< td=""><td>3/24/2021 3:00</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td></t<>	3/24/2021 3:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
39242021 345 2 0 0 0 0							-		-			-	-	-	-
3242021 400 0 0 0			-	-	-	-		-	-	-	-	_			-
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3242021 430 0 <th< td=""><td></td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>_</td><td>-</td><td>-</td><td>-</td></th<>		-	-	-	-	-	-	-	-	-	-	_	-	-	-
3242021 445 0 <th< td=""><td></td><td></td><td></td><td></td><td>-</td><td></td><td>-</td><td></td><td></td><td>-</td><td>-</td><td></td><td></td><td></td><td>0</td></th<>					-		-			-	-				0
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3242221 530 0 <th< td=""><td>3/24/2021 5:00</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td></th<>	3/24/2021 5:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3242221 5.46 0 <t< td=""><td></td><td>-</td><td>-</td><td>-</td><td></td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>-</td><td>0</td></t<>		-	-	-		-	-	-	-	-	-	-	-	-	0
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32442021 745 0 <t< td=""><td>3/24/2021 7:15</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td></t<>	3/24/2021 7:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3244021 6:0 0				-		-	-	-	-	-	-	-	-	-	-
32424021 1:5 0															0
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32442021 9:00 <					-	-	-	-	-	-	-	-			-
3242021 9:30 0 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td>-</td><td>-</td><td>-</td><td></td><td>-</td><td></td><td></td><td></td><td>-</td></t<>							-	-	-		-				-
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3/24/02110:00 <	3/24/2021 9:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/02110:16 0 <		-	-	-		-	-	-	-	-	-	-	-		0
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3/22/2021 11:15 0		-	-	-	-	-	-	-	-	-	-	-	-	-	0
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3/24/2021 12:30 <					-	-				-		_			-
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3/24/2021 15:00 0								-	-	-		_			-
3/24/2021 15:15 0					-	-			-	-	-	_			-
3/24/2021 15:45 0					-			-	-	-	-	_			-
3/24/2021 16:00 0															
3/24/2021 16:15 0					-	-	-	-	-	-	-	_			
3/24/2021 16:30 0				-			-			-	-	_			
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3/24/2021 17:45 0				-	-	-	-	-	-		-	_			-
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3/24/2021 18:45 6 0 0 2 0 4 0 0 0 0 0 0 0 0 0 0															
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Location:	Location 8_Heller Road with NGA South Gate_Inbound
Counter ID:	Counter B
Scheme:	FHWA
Notes:	Counter set after 3/23/21 PM Count (approximately 7 PM) and picked up after 3/24/21 PM Count (approximately 7 PM)

Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9	Class #10	Class #11	Class #12	Class #13
3/24/2021 19:15	28	0	0	21	0	7	0	0	0	0	0	0	0	0
3/24/2021 19:30	26	0	1	20	0	5	0	0	0	0	0	0	0	0
3/24/2021 19:45	29	0	1	22	1	5	0	0	0	0	0	0	0	0
3/24/2021 20:00	39	0	4	28	0	6	1	0	0	0	0	0	0	0
3/24/2021 20:15	37	0	5	30	0	2	0	0	0	0	0	0	0	0
3/24/2021 20:30	44	0	9	34	0	1	0	0	0	0	0	0	0	0
3/24/2021 20:45	49	0	11	36	0	2	0	0	0	0	0	0	0	0
3/24/2021 21:00	50	0	21	23	1	5	0	0	0	0	0	0	0	0
3/24/2021 21:15		0	32	29	0	0	0	0	0	0	0	0	0	0
3/24/2021 21:30	62	0	42	19	0	1	0	0	0	0	0	0	0	0
3/24/2021 21:45	37	1	25	11	0	0	0	0	0	0	0	0	0	0
3/24/2021 22:00	58	0	40	16	0	2	0	0	0	0	0	0	0	0
3/24/2021 22:15	42	0	35	7	0	0	0	0	0	0	0	0	0	0
3/24/2021 22:30	35	0	24	7	0	4	0	0	0	0	0	0	0	0
3/24/2021 22:45	9	0	5	3	0	1	0	0	0	0	0	0	0	0
3/24/2021 23:00	1	0	1	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 23:15	5	0	2	3	0	0	0	0	0	0	0	0	0	0
3/24/2021 23:30		0	0	1	1	0	0	0	0	0	0	0	0	0
3/24/2021 23:45	2	0	1	0	1	0	0	0	0	0	0	0	0	0

Location:	Location 9 Barta Road with NGA West Gate Entrance
Counter ID: Scheme:	Counter D FHWA
Notes:	Counter set after 3/23/21 PM Count (approximately 7 PM) and picked up after 3/24/21 PM Count (approximately 7 PM)

Data /Tima	Volumo	Class #1	Class #2	Class #2	Class #4	Class #F	Class #C	Class #7	Class #0	Class #0	Class #10	Class #11	Class #12	Class #12
Date/Time 3/24/2021 0:00	Volume 0	Class #1 0	Class #2 0	Class #3 0	Class #4 0	Class #5 0	Class #6 0	Class #7 0	Class #8 0	Class #9	Class #10	Class #11 0		Class #13
3/24/2021 0:00	0	0	0		0	0	0			-				0
3/24/2021 0:30	0	0	0	0	0	0	0		0		-			
3/24/2021 0:45	0	0	0	0	0	0	0	0	0	0	-	-	-	0
3/24/2021 1:00	0	0	0	-	0	0	0	-	0	-	-	-		0
3/24/2021 1:15	0	0	0	0	0	0	0		0					
3/24/2021 1:30	0	0	0	0	0	0	0	0	0	0	0	0		0
3/24/2021 1:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 2:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 2:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 2:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 2:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 3:00	0	0	0	-	0	0	0		0		-			0
3/24/2021 3:15	0	0	0		0	0	0	-	0		-			-
3/24/2021 3:30	0	0	-	-	0	0	0	-	_	-	-	-		
3/24/2021 3:45	0		0	-	0	0	0	-	_	-	-	-		-
3/24/2021 4:00	0		0		0	0	0				-			
3/24/2021 4:15	0	0	0	0	0	0	0	0	0	-	-	-	-	0
3/24/2021 4:30	0	0			0	0	0		0					
3/24/2021 4:45	0		0	-	0	0	0	-	0	-	-	-		-
3/24/2021 5:00	0	0	0	0	0	0	0	-	0	-	-	-		0
3/24/2021 5:15	0	0	0	0	0	0	0	0	0	-	-			-
3/24/2021 5:30 3/24/2021 5:45	0	0	0	-	0	0	0	-	_	-	-	-	-	-
3/24/2021 5:45	0	0	0	0	0	0	0		0	-				0
3/24/2021 6:00	0	0	0	-	0	0	0	-	0	-	-	-	-	0
3/24/2021 6:15	0	-	0	-	0	0	0	-	_	-	-	-		
3/24/2021 6:30	0	0	0	0	0	0	0	-	0	-	-	-	-	-
3/24/2021 0.45	15	0	13	2	0	0	0	0	0	0	-	-		0
3/24/2021 7:15	40	0	33	7	0	0	0	-	0	-	-	-		-
3/24/2021 7:30	44	0	37	7	0	0	0		0					
3/24/2021 7:45	81	0	64	17	0	0	0	0	0	-	-	-	-	0
3/24/2021 8:00	89	0	76	11	0	2	0	-	0	-	-			0
3/24/2021 8:15	93	0	78	12	0	3	0		0					0
3/24/2021 8:30	92	0	75	12	0	5	0	0	0	0	0	0	0	0
3/24/2021 8:45	88	0	77	10	0	1	0	0	0	0	0	0	0	0
3/24/2021 9:00	101	1	93	6	0	1	0	0	0	0	0	0	0	0
3/24/2021 9:15	108	0	96	10	0	2	0	0	0	0	0	0	0	0
3/24/2021 9:30	117	0	99	18	0	0	0	0	0	0	0	0	0	0
3/24/2021 9:45	124	0	114	9	0	1	0	0	0	0	0	0	0	0
3/24/2021 10:00	116	0	106	10	0	0	0	0	0	0	0	0	0	0
3/24/2021 10:15	112	0	102	9	0	1	0	-	0	-	0			0
3/24/2021 10:30	102	0	93	9	0	0	0	0	0	-	-	-	-	0
3/24/2021 10:45	81	0	70	10	0	1	0		0					
3/24/2021 11:00	44	0	39	5	0	0	0	-	-	-	-	-		
3/24/2021 11:15	0	0	0	0	0	0	0	-	0	-	-	-		0
3/24/2021 11:30	0	0	0	0	0	0	0	-	0		-			
3/24/2021 11:45	0	0	0	-	0	0	0	-	0	-	-	-		-
3/24/2021 12:00	0	0	0	0	0	0	0		0	-	-			0
3/24/2021 12:15 3/24/2021 12:30	0	0	0	0	0	0	0	-	0	-	-	-		0
3/24/2021 12:30	0	-	0	0	0	0	0	-	0	-	÷	-		÷
3/24/2021 12:45	0		-		0	0	-	-			-			-
3/24/2021 13:00	0	0	0	0	0	0	0	0	-	0				
3/24/2021 13:13	0	0	0		0	0	0		0	-	-			
3/24/2021 13:30	0				0	0	0							
3/24/2021 13:43	0	0	0		0	0	0	-	0		-			
3/24/2021 14:15	0	0			0	0	0		-	-	-			
3/24/2021 14:30	0		0		0	0	0							
3/24/2021 14:45	0		-	-	0	0	0	-	_		-	-		-
3/24/2021 15:00	0	0	0		0	0	0		0					
3/24/2021 15:15	0	0	-	-	0	0	0	-	_	-	-	-	-	-
3/24/2021 15:30	0				0	0	0				-			
3/24/2021 15:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 16:00	0	0	0	0	0	0	0	-			0	-		
3/24/2021 16:15	0	0	0		0	0	0							
3/24/2021 16:30	0		0	0	0	0	0		0	-	-			
3/24/2021 16:45	0	0	0	0	0	0	0	-	0	-	-	-		-
3/24/2021 17:00	0				0	0	0							
3/24/2021 17:15	0		0	-	0	0	0			-	-	-		-
3/24/2021 17:30	0			-	0	0	0			-	-	-		
3/24/2021 17:45	0	0	0		0	0	0	-						
3/24/2021 18:00	0	0	0		0	0	0	-	0		-	-		
3/24/2021 18:15	0			-	0	0	0		-		-			
3/24/2021 18:30	0		0		0	0	0	-	0		-			
3/24/2021 18:45 3/24/2021 19:00	0				0	0					-			
	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Location:	Location 9 Barta Road with NGA West Gate Entrance
Counter ID:	Counter D
Scheme:	FHWA
Notes:	Counter set after 3/23/21 PM Count (approximately 7 PM) and picked up after 3/24/21 PM Count (approximately 7 PM)

Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9	Class #10	Class #11	Class #12	Class #13
3/24/2021 19:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 19:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 19:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 20:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 20:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 20:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 20:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 21:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 21:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 21:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 21:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 22:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 22:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 22:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 22:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 23:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 23:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 23:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 23:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Location:	Location 10_Barta Road and NGA West Gate_Exit
Counter ID:	Counter C
Scheme:	FHWA
Notes:	Counter set after 3/23/21 PM Count (approximately 7 PM) and picked up after 3/25/21 AM Count (approximately 10 AM)

D / 7'	M.1	61 114	0	Cl	6 1	61 // F	01	<u> </u>	0	0	0	0	0	01
Date/Time 3/24/2021 0:00	Volume 0	Class #1 0	Class #2	Class #3	Class #4	Class #5 0		Class #7 0	Class #8	Class #9		Class #11 0	Class #12	
3/24/2021 0:00	0		-			0	-	0	-	0	-	0	-	0
3/24/2021 0:30	0		-	-	-	0	÷	0	-			0		0
3/24/2021 0:45	0	-		-	-	0	_	0	-	-		0	-	0
3/24/2021 1:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 1:15	0	-	-	-	-	0	_	-	-	-		0	-	0
3/24/2021 1:30	0	-	-		-	0	_	0		-		0	-	0
3/24/2021 1:45	0	-	-	-	-	0	-	-	-	-		0	-	0
3/24/2021 2:00 3/24/2021 2:15	0				-	0	-	0	-	0		0	-	0
3/24/2021 2:30	0	-	-	-		0	_	0	-	-	-	0	-	0
3/24/2021 2:45	0	-				0	_		-	-		0		0
3/24/2021 3:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 3:15	0					0	-	0				0		0
3/24/2021 3:30	0	-	-	-	-	0	-	-	-	-	-	0	-	0
3/24/2021 3:45	0					0	_	0		-		0	-	0
3/24/2021 4:00 3/24/2021 4:15	0	-	-	-	-	0	_	0	-	-		0	-	0
3/24/2021 4:13	0	-	-	-	-	0	-	0	-	-		0	-	0
3/24/2021 4:45	0		-	-		0	_	-	-	0		0	-	0
3/24/2021 5:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 5:15	0				-	0	_	0		-		0	-	0
3/24/2021 5:30	0	-	-	-	-	0	_	0	-	-		0	-	0
3/24/2021 5:45	0	-	-	-	-	0	_	0	-	0	-	0	-	0
3/24/2021 6:00 3/24/2021 6:15	0				-	0		0				0		0
3/24/2021 6:30	0	-	-	-	-	0	_	0	-	-	-	0	-	0
3/24/2021 6:45	0					0	-	0		0		0	-	0
3/24/2021 7:00	0					0	0	0	0			0	0	0
3/24/2021 7:15	0	-	-	0		0	_	0	0	0	0	0	-	0
3/24/2021 7:30	0	-	-	-	-	0	÷	0	-	0	-	0	-	0
3/24/2021 7:45	0	-	-		-	0	_	0	-	-	-	0	-	0
3/24/2021 8:00 3/24/2021 8:15	0	-	-	-	-	0	_	0	-	0	-	0	-	0
3/24/2021 8:30	0	-	-		0	0	_	0	-	0		0	-	0
3/24/2021 8:45	0	-	-	-	-	0	-	0	-	-	-	0	-	0
3/24/2021 9:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 9:15	0	-	-	-	-	0	÷	0	-	÷	-	0	-	0
3/24/2021 9:30	0	-			-	0	-	0		-		0	-	0
3/24/2021 9:45 3/24/2021 10:00	0	-	-	-	-	0	_	0	-	-	-	0	-	0
3/24/2021 10:00	0					0	_	0		0		0	-	0
3/24/2021 10:30	0	-	-	-	-	0	_	0	-	-	-	0	-	0
3/24/2021 10:45	0	-	-	-	-	0	_	-	-	-		0	-	0
3/24/2021 11:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 11:15	0	-	-	-	-	0	-	0	-	0	-	0	-	0
3/24/2021 11:30	0					0	-	0		-		0	-	0
3/24/2021 11:45 3/24/2021 12:00	0	-	-	-	-	0	_	0	-	-		0	-	0
3/24/2021 12:15	0	-	-	-	-	0	_	0	-	-	-	0	-	0
3/24/2021 12:30	0	-			-	0		0	-	0		0		0
3/24/2021 12:45	0	0	0	0	0	0	0	0		0	0	0	-	0
3/24/2021 13:00	0	÷	-	-	÷	0	÷	-	-	÷	-	÷	÷	0
3/24/2021 13:15	0					0								0
3/24/2021 13:30 3/24/2021 13:45	0					0	_			-			-	0
3/24/2021 13:45	0			-		0			-			0		0
3/24/2021 14:15	0	-				0				-		-	-	0
3/24/2021 14:30	0	0	0	0	0	0	0	0	0	0	0	0		0
3/24/2021 14:45	0	-	-	-	-	0	_	0	-	-		0	-	0
3/24/2021 15:00	0					0						0		0
3/24/2021 15:15	0		-			0	-	-	-			0		-
3/24/2021 15:30 3/24/2021 15:45	0			-	-	0	-		-	-		0	-	0
3/24/2021 15:45	63	0		2		0				-		0	-	0
3/24/2021 16:15	59			2		0	_	-	-			0	-	0
3/24/2021 16:30	79			6	0	0	_	-	-	-		-	-	0
3/24/2021 16:45	55	0				0								-
3/24/2021 17:00	81	0			-	0	_			-		-	-	-
3/24/2021 17:15 3/24/2021 17:30	83 97	0		2	0	0	-	0		-		0		0
3/24/2021 17:30	109	0		3		0	-			-	-	0	-	0
3/24/2021 18:00	95	0		4		0						0		0
3/24/2021 18:15	116	2		3	-	0	-	0	-	-	-	0	-	0
3/24/2021 18:30	98	0		2		0		-				0		0
3/24/2021 18:45	89		-	2		0	_		-	-		-	-	0
3/24/2021 19:00	20	0	19	0	0	1	0	0	0	0	0	0	0	0

Location:	Location 10_Barta Road and NGA West Gate_Exit
Counter ID:	Counter C
Scheme:	FHWA
Notes:	Counter set after 3/23/21 PM Count (approximately 7 PM) and picked up after 3/25/21 AM Count (approximately 10 AM)

Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9	Class #10	Class #11	Class #12	Class #13
3/24/2021 19:15	0	0			-	-	-		0		-	0	-	0
3/24/2021 19:30 3/24/2021 19:45	0	0		-	-				0	-		0	-	0
3/24/2021 19:43	0	-	-	-	-		_	-	0	-	-	-	-	0
3/24/2021 20:15	0	0	-	-	-	-	_	-	0	-	-	0	-	0
3/24/2021 20:30	0	-			-		_		0	-	-	0	-	0
3/24/2021 20:45	0	0		-	-	0	_	0	0	-	-	0	-	0
3/24/2021 21:00	0				-	0		-	0	-		0	-	0
3/24/2021 21:15 3/24/2021 21:30	0	0	-	-	-	0	_	-	0	-	-	0	-	0
3/24/2021 21:45	0	-	-	-		0	_	-	0	-	-	0	-	0
3/24/2021 22:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/24/2021 22:15	0	0		-					0	-		0	-	0
3/24/2021 22:30	0	0	0		-	0	-	-	0		-	0	-	0
3/24/2021 22:45 3/24/2021 23:00	0	-	-	-			_	-	0	-	-	0	-	0
3/24/2021 23:15	0	0			-	0	_	-	0	-	-	0	-	0
3/24/2021 23:30	0	0			-	0	-	0	0	-	-	0	-	0
3/24/2021 23:45	0	-	-		-	-	_	-	0	-	-	-	-	0
3/25/2021 0:00 3/25/2021 0:15	0	0		-	-	-	_	0	0	-	-	0	-	0
3/25/2021 0:15	0	-	-	-	-		_	-	0	-	-	-	-	0
3/25/2021 0:45	0							-	0			0		0
3/25/2021 1:00	0	0		-	-	0	_	0	0	-	-	0	-	0
3/25/2021 1:15	0	0			-	0		0	0	-	-	0	-	0
3/25/2021 1:30 3/25/2021 1:45	0	0	-	-	-	0	_	-	0	-	-	0	-	0
3/25/2021 1:45	0	0			-	-	-	-	0	-	-	0	-	0
3/25/2021 2:15	0	-	0	0	-		_	-	0	-	-	0	-	0
3/25/2021 2:30	0	0	-		-	-	_	-	0	-	-	0	-	0
3/25/2021 2:45	0	-		-	-			-	0	-		0	-	0
3/25/2021 3:00 3/25/2021 3:15	0	0			-	0	_	0	0	-	-	0	-	0
3/25/2021 3:13	0					0			0	-		0	-	0
3/25/2021 3:45	0	0			-	0	_	0	0	-	-	0	-	0
3/25/2021 4:00	0	0			-	0		0	0	-	-	0		0
3/25/2021 4:15	0	0	-	-	-	-	_	-	0	-	-	0	-	0
3/25/2021 4:30 3/25/2021 4:45	0	0		-		0	_		0	-		0	-	0
3/25/2021 4:43	0	-			-	-	_	-	0	-	-	-	-	0
3/25/2021 5:15	0	-	-	-	-		-	-	0	-	-	-	-	0
3/25/2021 5:30	0	0		-	-	0	_	-	0	-	-	0	-	0
3/25/2021 5:45	0	0		-	-	0	-	0	0	-	-	0	-	0
3/25/2021 6:00 3/25/2021 6:15	0	0			-	0		-	0	-		0	-	0
3/25/2021 6:30	0	0		-	-	-	_	0	0	-	-	0	-	0
3/25/2021 6:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/25/2021 7:00	0	0	-	-	-	-	_	-	0	-	-	0	-	0
3/25/2021 7:15 3/25/2021 7:30	0			-	-		_		0	-	-	0	-	0
3/25/2021 7:30	0	0	0		-	0		-	0	-		0	-	0
3/25/2021 8:00	0	0	-	-	-	0	-	-	0	-	-	0	-	0
3/25/2021 8:15	0	0				0			0			0		0
3/25/2021 8:30		0							0		0			
3/25/2021 8:45 3/25/2021 9:00	0	-	-	-	-	-	_	-	0	-	-	-	-	0
3/25/2021 9:00	0	0		-	-			-	0	-		0	-	0
3/25/2021 9:30	0	-	-	-	-		-	-	0		-	-	-	
3/25/2021 9:45	0	-			-		_		0	-		-	-	0
3/25/2021 10:00	0	0	-	-	-		-	-	0	-	-	-	-	0
3/25/2021 10:15 3/25/2021 10:30	0				-			-	0			0	-	0
3/25/2021 10:30	0	-			-	-	-		0			-	-	0
3/25/2021 11:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/25/2021 11:15	0				-			-	0			-	-	-
3/25/2021 11:30 3/25/2021 11:45	0		-		-		_	-	0	-		-	-	0
3/25/2021 11:45	0	0			-				0	-		0		0
3/25/2021 12:00	0				-		-	-	0	-	-	0	-	0
3/25/2021 12:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/25/2021 12:45	0	-	-	-	-		_	-	0	-	-	-	-	0
3/25/2021 13:00	0								0			-		0
3/25/2021 13:15 3/25/2021 13:30	0	-	-	-	-	-	_	-	0	-	-	-	-	0
3/25/2021 13:45	0	0							0	-		0	-	0
3/25/2021 14:00	0	0	0	0	0	0		-	0			-	0	0
3/25/2021 14:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Location: Counter ID:	Location 1 Counter C	-	ad and NG	A West Gat	te_Exit									
Scheme:	FHWA													
Notes:	Counter se	et after 3/2	3/21 PM C	ount (appro	oximately 7	7 PM) and p	picked up a	fter 3/25/2	1 AM Coun	t (approxin	nately 10 A	M)		
	I								-					
Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9	Class #10	Class #11	Class #12	Class
3/25/2021 14:30	0	0	0	0	0	0	0	0	0	0	0	0	0	

Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9	Class #10	Class #11	Class #12	Class #13
3/25/2021 14:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/25/2021 14:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/25/2021 15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/25/2021 15:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/25/2021 15:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/25/2021 15:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/25/2021 16:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/25/2021 16:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/25/2021 16:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/25/2021 16:45	0	0	-	0	0	0	0	0	0	0	0	0	0	0
3/25/2021 17:00	0	0	-	0	0	0	0	0	0	-	0	0	0	0
3/25/2021 17:15	0	0	-	0	÷	0	0	0	0	-	0	0	÷	0
3/25/2021 17:30	0	0	-	0	÷	0	0	0	0	-	0	0	-	0
3/25/2021 17:45	0	0	-	0	0	0	0	0	0		0	0	-	0
3/25/2021 18:00	0	0	-	0	-	0	0	0	0	-	0	0	÷	0
3/25/2021 18:15	0	0	-	0	-	0	0	0	0	-	0	0	÷	0
3/25/2021 18:30	0	0	-	0	-	0	0	0	0	÷	0	0	÷	0
3/25/2021 18:45	0	0	-	0	-	0	0	0	0	-	0	0	-	0
3/25/2021 19:00	0	0	-	0	-	0	0	0	0	÷	0	÷	÷	0
3/25/2021 19:15	0	0	-	0	-	0	0	0	0	-	0	0	÷	0
3/25/2021 19:30	0	0	-	0	0	0	0	0	0	-	0	0	÷	0
3/25/2021 19:45	0	0	-	0	÷	0	0	0	0	-	0	0	-	0
3/25/2021 20:00	0	0		0	-	0	0	0	0	-	0	0	-	0
3/25/2021 20:15	0	0	-	0	0	0	0	0	0	-	0	0	-	0
3/25/2021 20:30	0	0	-	0	0	0	0	0	0	-	0	0	-	0
3/25/2021 20:45	0	0	-	0	÷	0	0	0	0	-	0	0	÷	0
3/25/2021 21:00 3/25/2021 21:15	0	0	-	0	0	0	0	0	0	-	0	0	-	0
3/25/2021 21:15	0	0	-	0	-	0	0	0	0	-	0	-	÷	0
3/25/2021 21:45	0	0	-	0	÷	0	0	0	0	-	0	0	÷	0
3/25/2021 21:43	0	0	-	0	-	0	0	0	0	÷	0	0	÷	0
3/25/2021 22:00	0	0	-	0	-	0	0	0	0	-	0	0	-	0
3/25/2021 22:30	0	0	-	0	÷	0	0	0	0	-	0	0	÷	0
3/25/2021 22:45	0	0	-	0	-	0	0	0	0	-	0	0	÷	0
3/25/2021 22:43	0	0	-	0	0	0	0	0	0	÷	0	0	÷	0
3/25/2021 23:15	0	0	-	0	-	0	0	0	0	-	0	-	-	0
3/25/2021 23:30	0	0	-	0	-	0	0	0	0	-	0	-	÷	0
3/25/2021 23:45	0	0	-	0	0	0	0	0	0	-	0	0	-	0

Location:	Location 11_NGA Visitor Center Access Roadway
Counter ID:	Counter C
Scheme:	FHWA
Notes:	Counter set on 3/22/21 PM (approximately 7 PM) and picked up after 3/23/21 PM Count (approximately 7 PM)

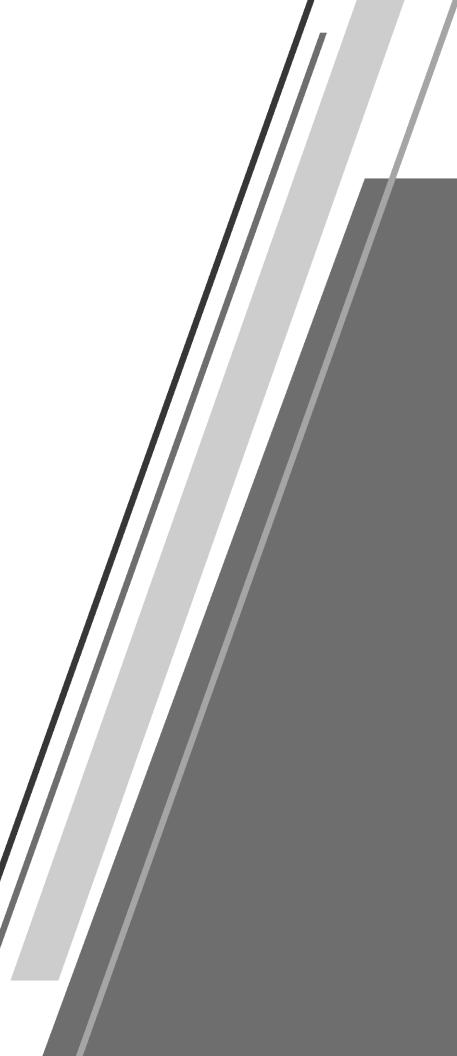
			Class #2	Class #3		Class #5		Class #7	Class #8	Class #9			Class #12	
3/23/2021 0:00	0	-	0		-	0	-	0	-	-		0		0
3/23/2021 0:15	0		0	-	-	0	÷	0	0	÷		0	-	0
3/23/2021 0:30	0	_	0	-	-	0	_	0	0	-		0	-	0
3/23/2021 0:45	0	_	0	-	-	0	-	0	0	-	-	0	-	0
3/23/2021 1:00 3/23/2021 1:15	0		0	-		0	-	0	0	-		0	-	0
3/23/2021 1:30	0	-	0	-	-	0	_	0	-	-		0	-	0
3/23/2021 1:45	0	-	0		-	0	_		0	-		0	-	0
3/23/2021 1:45	0	-	0	-	-	0	_	0	0	-		0	-	0
3/23/2021 2:15	0		0	-	-	0	-	0	0			0		0
3/23/2021 2:30	0	-	0	-		0	_	0	0	-	-	0	-	0
3/23/2021 2:45	0	_	0			0	_		0	-		0		0
3/23/2021 3:00	0	-	0	-	-	0	_	0	0			0	-	0
3/23/2021 3:15	0		0	-	-	0	÷	0	0	÷	-	0	-	0
3/23/2021 3:30	0	-	0			0	-		0	-		0		0
3/23/2021 3:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 4:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 4:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 4:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 4:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 5:00	1	0	1	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 5:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 5:30	3		3		-	0	_	0	0	-		0	-	0
3/23/2021 5:45	1	0	1	-	-	0	_	0	0	-	-	0	-	0
3/23/2021 6:00	0		0		-	0		0	0			0		0
3/23/2021 6:15	1	-	0		0	0	-	-	0	-	-	0	-	0
3/23/2021 6:30	6		6			0	-	0	0	-		0	-	0
3/23/2021 6:45	3		3		-	0	-	0	0	-		0	-	0
3/23/2021 7:00	2		2		-	0	-		0	-		0	-	0
3/23/2021 7:15	6		5			1	_	0	0	-		0	-	0
3/23/2021 7:30	5		3		0	1	÷	0	0	-	-	0	-	0
3/23/2021 7:45	8		8		-	0	_	0	0	-	-	0	-	0
3/23/2021 8:00	11	0	9		0	0	_	-	0	-	-	0	-	0
3/23/2021 8:15	8		7	1	0	0	_	0	0	-		0	-	0
3/23/2021 8:30	2	0	2		0	0	_	0	0	÷	-	0	-	0
3/23/2021 8:45	3		3			0	-					0	-	0
3/23/2021 9:00	10		10		-	0	-		0	-		0	-	0
3/23/2021 9:15	12	0	11	1	0	0	÷	0	0	÷	-	0	-	0
3/23/2021 9:30	15	0	14	1	0	0		0	0	-		0	-	0
3/23/2021 9:45	12	0	11	1	0	0	-	0	0	-	-	0	-	0
3/23/2021 10:00	13 16	0	13 16	0	0	0	_	0	0	-		0		0
3/23/2021 10:15	10	0	10	0	-	0	_	0	0	-		0	-	0
3/23/2021 10:30 3/23/2021 10:45	12	0	12	0		1	-	-	0	-		0	-	0
3/23/2021 10:45	13	0	14	2	-	0	_	-	0	-	-	0	-	0
3/23/2021 11:15	13	0	12	0	0	0	÷	0	0			0	-	0
3/23/2021 11:30	8	-	8	-		0	_	0	0	-	-	0	-	0
3/23/2021 11:45	10		9			1	-		0	-		0		0
3/23/2021 12:00	3		3		-	0	_	0	0			0	-	0
3/23/2021 12:15	6		5		0	0	_	0	0		-	0	-	0
3/23/2021 12:30	10	0	9		0	0	_	0	0			0		0
3/23/2021 12:45	4	0	4	0	-	0	_	0	0	-	-	0	-	0
3/23/2021 13:00	5				0	0	_		-			0		0
3/23/2021 13:15	5	_			-	0	÷	-	-	÷	-	÷	-	0
3/23/2021 13:30	3					0								0
3/23/2021 13:45	4		3			1							0	0
3/23/2021 14:00	8		5			1			0			0		0
3/23/2021 14:15	4	0			0	0	0	0	0	0	0	0	0	0
3/23/2021 14:30	3	0	1	2	0	0	0	0	0	0	0	0	0	0
3/23/2021 14:45	4	0	4	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 15:00	6			0		0	0							0
3/23/2021 15:15	6	-				1	-	-		-		-		0
3/23/2021 15:30	6	0	6		0	0	0	0	0	0	0	0	-	0
3/23/2021 15:45	4	-	3		-	0	-			-	-	0	-	0
3/23/2021 16:00	4	_	4			0	_		-	-	-	0	-	0
3/23/2021 16:15	1	0	0		0	0			0	-		0	-	0
3/23/2021 16:30	4		-		-	1	_	-	-	-		0	-	0
3/23/2021 16:45	3				-	0						0		0
3/23/2021 17:00	3		-		-	0	_		-	-		-	-	0
3/23/2021 17:15	2		2			0	-			-		0		0
3/23/2021 17:30	3		3		-	0	_	0		-		0	-	0
3/23/2021 17:45	0		0			0	_					0	-	0
3/23/2021 18:00	2		2		-	0	-		-	-	-	0	-	0
3/23/2021 18:15	4		4		-	0	-	-	-	÷	-	0	-	0
3/23/2021 18:30	3		2			1			0			0		0
3/23/2021 18:45	2	0	2	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 18:45	5	0	3	2	0	0	0	0	0	0	0	0	0	0

Location:	Location 11_NGA Visitor Center Access Roadway
Counter ID:	Counter C
Scheme:	FHWA
Notes:	Counter set on 3/22/21 PM (approximately 7 PM) and picked up after 3/23/21 PM Count (approximately 7 PM)

Date/Time	Volume	Class #1	Class #2	Class #3	Class #4	Class #5	Class #6	Class #7	Class #8	Class #9	Class #10	Class #11	Class #12	Class #13
3/23/2021 19:15	3	0	2	0	0	1	0	0	0	0	0	0	0	0
3/23/2021 19:30	2	0	1	0	0	1	0	0	0	0	0	0	0	0
3/23/2021 19:45	3	0	3	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 20:00	1	0	1	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 20:15	1	0	1	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 20:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 20:45	1	0	1	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 21:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 21:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 21:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 21:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 22:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 22:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 22:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 22:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 23:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 23:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 23:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3/23/2021 23:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0

APPENDIX C

Synchro Files



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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	∱ î,			<u></u>		1
Traffic Volume (vph)	86	0	6	30	0	0
Future Volume (vph)	86	0	6	30	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	0.95	0.95	0.95	1.00	1.00
Frt						
Flt Protected				0.991		
Satd. Flow (prot)	3539	0	0	3507	0	1863
Flt Permitted				0.991		
Satd. Flow (perm)	3539	0	0	3507	0	1863
Link Speed (mph)	30			30	30	
Link Distance (ft)	404			491	211	
Travel Time (s)	9.2			11.2	4.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	93	0	7	33	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	93	0	0	40	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	12			24	0	
Link Offset(ft)	0			6	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 8.4%			IC	U Level o	of Service
Analysis Period (min) 15						

Analysis Period (min) 15

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Lane Group	SBL	SBR	NEL	NET	SWT	SWR	
Lane Configurations		77	ሻሻ	•			
Traffic Volume (vph)	0	24	115	355	0	0	
Future Volume (vph)	0	24	115	355	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	0.88	0.97	1.00	1.00	1.00	
Frt		0.850					
Flt Protected			0.950				
Satd. Flow (prot)	0	2787	3433	1863	0	0	
Flt Permitted			0.950				
Satd. Flow (perm)	0	2787	3433	1863	0	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	227			1186	549		
Travel Time (s)	5.2			27.0	12.5		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	26	125	386	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	26	125	386	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	12			36	36		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15	9	15			9	
Sign Control	Free			Free	Free		
Intersection Summary							
31	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	tion 22.0%			IC	U Level o	of Service A	А
Analysis Dariad (min) 15							

Analysis Period (min) 15

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Lane Group	EBL	EBR	NEL	NET	SWT	SWR
Lane Configurations				<u></u>	^	1
Traffic Volume (vph)	0	0	0	470	14	10
Future Volume (vph)	0	0	0	470	14	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	0.91	1.00
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	0	0	3539	5085	1583
Flt Permitted						
Satd. Flow (perm)	0	0	0	3539	5085	1583
Link Speed (mph)	30			30	30	
Link Distance (ft)	1042			233	359	
Travel Time (s)	23.7			5.3	8.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	511	15	11
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	511	15	11
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	8			0	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Free			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 16.3%			IC	U Level	of Service
Analysis Dariad (min) 15						

Analysis Period (min) 15

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Lane Group	SBL	SBR	NWL	NWR	NEL	NER
Lane Configurations	<u> </u>			7	ሻሻ	
Traffic Volume (vph)	0	24	0	·	470	r
Future Volume (vph)	0	24	0	0	470	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1,00	0.88	1.00	1,00	0.97	1,00
Frt	1.00	0.850	1.00	1.00	0.77	1.00
Fit Protected		0.000			0.950	
Satd. Flow (prot)	1863	2787	1863	1863	3433	1863
Fit Permitted	1003	2101	1003	1003	3433 0.950	1003
	1040	2202	1040	1040		1040
Satd. Flow (perm)	1863	2787	1863	1863 Voc	3433	1863 Voc
Right Turn on Red		Yes		Yes		Yes
Satd. Flow (RTOR)	20	1920	0.0		20	
Link Speed (mph)	30		30		30	
Link Distance (ft)	763		723		430	
Travel Time (s)	17.3		16.4		9.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	26	0	0	511	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	26	0	0	511	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	30	5	32	5	32	5
Link Offset(ft)	30		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	9	1.00	9	1.00	9
Number of Detectors	0	0	0	0	0	0
Detector Template	Thru	Thru	Thru	Thru	Thru	Thru
Leading Detector (ft)	0	0	0	0	0	0
Trailing Detector (ft)	0 Drot	0	0 Drot	0 Dorm	0 Drot	0 Dorm
Turn Type	Prot	pt+ov	Prot	Perm	Prot	Perm
Protected Phases	5	56	4		6	,
Permitted Phases				4		6
Detector Phase	5	56	4	4	6	6
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (s)	15.5		15.5	15.5	19.0	19.0
Total Split (%)	31.0%		31.0%	31.0%	38.0%	38.0%
Maximum Green (s)	11.0		11.0	11.0	14.5	14.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag	Lead		т.J	+.J	Lag	Lag
	Yes				•	Yes
Lead-Lag Optimize?			2.0	2.0	Yes	
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	None		None	None	C-Max	C-Max

01 Alt 01 AM Existing As Counted Existing 2021 volumes 7:45 am 03/24/2021 01 AM

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Lane Group	SBL	SBR	NWL	NWR	NEL	NER		
Walk Time (s)	7.0		7.0	7.0	7.0	7.0		
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	11.0		
Pedestrian Calls (#/hr)	0		0	0	0	0		
Act Effct Green (s)		50.0			47.1			
Actuated g/C Ratio		1.00			0.94			
v/c Ratio		0.01			0.16			
Control Delay		0.0			0.9			
Queue Delay		0.0			0.0			
Total Delay		0.0			0.9			
LOS		А			А			
Approach Delay					0.9			
Approach LOS					А			
Queue Length 50th (ft)		0			0			
Queue Length 95th (ft)		0			29			
Internal Link Dist (ft)	683		643		350			
Turn Bay Length (ft)								
Base Capacity (vph)		2787			3234			
Starvation Cap Reductn		0			0			
Spillback Cap Reductn		0			0			
Storage Cap Reductn		0			0			
Reduced v/c Ratio		0.01			0.16			
Intersection Summary								
Area Type:	Other							
Cycle Length: 50								
Actuated Cycle Length: 50								
Offset: 0 (0%), Referenced	to phase 6:1	VEL, Sta	rt of Gree	n				
Natural Cycle: 70								
Control Type: Actuated-Co	ordinated							
Maximum v/c Ratio: 0.16								
Intersection Signal Delay: 0					tersection			
Intersection Capacity Utilization	ation 17.2%			IC	U Level o	f Service	A	
Analysis Period (min) 15								
Splite and Dhasses 102								
Splits and Phases: 102:								

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15.5 s	19 s	15.5 s

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>^</u>			^	ኘኘ	1
Traffic Volume (vph)	115	0	0	24	0	0
Future Volume (vph)	115	0	0	24	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	1.00
Frt	0.75	1.00	1.00	0.75	0.77	1.00
Flt Protected						
Satd. Flow (prot)	3539	0	0	3539	3614	1863
Flt Permitted	0007	0	0	5557	5014	1005
Satd. Flow (perm)	3539	0	0	3539	3614	1863
Right Turn on Red	5557	Yes	0	3337	3014	Yes
Satd. Flow (RTOR)		162				162
	30			20	30	
Link Speed (mph)				30 522		
Link Distance (ft)	923			533	500	
Travel Time (s)	21.0	0.00	0.00	12.1	11.4	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	125	0	0	26	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	125	0	0	26	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Number of Detectors	2			2	1	1
Detector Template	Thru			Thru	Left	Right
Leading Detector (ft)	100			100	20	20
Trailing Detector (ft)	0			0	0	0
Detector 1 Position(ft)	0			0	0	0
Detector 1 Size(ft)	6			6	20	20
Detector 1 Type	CI+Ex			CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel	OTLA					OFLA
Detector 1 Extend (s)	0.0			0.0	0.0	0.0
	0.0			0.0	0.0	0.0
Detector 1 Queue (s)						
Detector 1 Delay (s)	0.0			0.0	0.0	0.0
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		_
Turn Type	NA			NA	Prot	Perm
Protected Phases	6			2	4	
Permitted Phases						4
Detector Phase	6			2	4	4
Switch Phase						
Minimum Initial (s)	5.0			5.0	5.0	5.0

01 Alt 01 AM Existing As Counted Existing 2021 volumes 7:45 am 03/24/2021 01 AM

	-	\mathbf{r}	1	-	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	22.5	LBR	WDL	22.5	22.5	22.5
Total Split (s)	22.5			22.5	22.5	22.5
Total Split (%)	50.0%			50.0%	50.0%	50.0%
Maximum Green (s)	18.0			18.0	18.0	18.0
Yellow Time (s)	3.5			3.5	3.5	3.5
All-Red Time (s)	1.0			1.0	1.0	1.0
Lost Time Adjust (s)	0.0			0.0	0.0	0.0
Total Lost Time (s)	4.5			4.5	4.5	4.5
Lead/Lag	4.0			4.5	4.5	4.5
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0			3.0	3.0	3.0
. ,						
Recall Mode	C-Max			C-Max	None	None
Walk Time (s)	7.0			7.0	7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	45.0			45.0		
Actuated g/C Ratio	1.00			1.00		
v/c Ratio	0.04			0.01		
Control Delay	0.0			0.0		
Queue Delay	0.0			0.0		
Total Delay	0.0			0.0		
LOS	А			А		
Approach Delay						
Approach LOS						
Queue Length 50th (ft)	0			0		
Queue Length 95th (ft)	0			1		
Internal Link Dist (ft)	843			453	420	
Turn Bay Length (ft)						
Base Capacity (vph)	3539			3539		
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.04			0.01		
Intersection Summary	0.01			0.01		
	Othor					
Area Type:	Other					
Cycle Length: 45						
Actuated Cycle Length: 45	ta akara Ol					
Offset: 0 (0%), Referenced	to phase 2:	WB1 and	6:EBT, 3	Start of G	reen	
Natural Cycle: 45						
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.04						
Intersection Signal Delay: (ntersectio	
Intersection Capacity Utiliz	ation 7.9%			[(CU Level	of Service
Analysis Period (min) 15						

Splits and Phases: 103:

← Ø2 (R)	▲ Ø4	
22.5 s	22.5 s	
₩ 22.5 s		
22.5 s		

	-	\mathbf{F}	4	←	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	≜ †⊅		<u> </u>	^	<u>`````````````````````````````````````</u>	1
Traffic Volume (vph)	86	29	6	24	0	0
Future Volume (vph)	86	29	6	24	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	0.95	1.00	0.95	1.00	1.00
Frt	0.962	0.70	1.00	0.70	1.00	1100
Flt Protected	0.702		0.950			
Satd. Flow (prot)	3405	0	1770	3539	1863	1863
Flt Permitted	0100	U	0.673	0007	1000	1000
Satd. Flow (perm)	3405	0	1254	3539	1863	1863
Right Turn on Red	0100	Yes	1201	0007	1000	Yes
Satd. Flow (RTOR)	32	103				103
Link Speed (mph)	30			30	30	
Link Distance (ft)	533			404	428	
Travel Time (s)	12.1			9.2	420 9.7	
Peak Hour Factor	0.92	0.92	0.92	9.2 0.92	0.92	0.92
Adj. Flow (vph)	93	0.92	0.92	26	0.92	0.92
Shared Lane Traffic (%)	73	32	/	20	0	0
Lane Group Flow (vph)	125	Λ	7	26	Λ	0
Enter Blocked Intersection	No	0 No	/ No	Zo No	0 No	No
			Left	Left		
Lane Alignment	Left 24	Right	Leit	Len 24	Left 20	Right
Median Width(ft)						
Link Offset(ft)	-12			8	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	1 00	1.00	1.00	1.00	1.00	1.00
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15 De 199		15 Dret	9
Turn Type	NA		Perm	NA	Prot	Perm
Protected Phases	4		-	8	2	-
Permitted Phases	00 -		8	06 -	06 -	2
Minimum Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (%)	50.0%		50.0%	50.0%	50.0%	50.0%
Maximum Green (s)	18.0		18.0	18.0	18.0	18.0
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag						
Lead-Lag Optimize?						
Walk Time (s)	7.0		7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0		0	0	0	0
Act Effct Green (s)	18.0		18.0	18.0		
Actuated g/C Ratio	0.40		0.40	0.40		
v/c Ratio	0.09		0.01	0.02		
Control Delay	6.9		8.3	8.2		
Queue Delay	0.0		0.0	0.0		
Total Delay	6.9		8.3	8.2		
	0.7		0.0	0.2		

01 Alt 01 AM Existing As Counted Existing 2021 volumes 7:45 am 03/24/2021 01 AM

	→	\mathbf{r}	∢	←	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
LOS	А		А	А		
Approach Delay	6.9			8.3		
Approach LOS	А			А		
Queue Length 50th (ft)	7		1	2		
Queue Length 95th (ft)	18		6	7		
Internal Link Dist (ft)	453			324	348	
Turn Bay Length (ft)						
Base Capacity (vph)	1381		501	1415		
Starvation Cap Reductn	0		0	0		
Spillback Cap Reductn	0		0	0		
Storage Cap Reductn	0		0	0		
Reduced v/c Ratio	0.09		0.01	0.02		
Intersection Summary						
Area Type:	Other					
Cycle Length: 45						
Actuated Cycle Length: 45						
Offset: 0 (0%), Reference	d to phase 2:1	VBL and 6	:, Start o	of Green		
Natural Cycle: 45						
Control Type: Pretimed						
Maximum v/c Ratio: 0.09						
Intersection Signal Delay:					tersectior	
Intersection Capacity Utiliz	zation 8.7%			IC	U Level o	of Service A
Analysis Period (min) 15						

Splits and Phases: 104:

• 1 √ø2 (R)	→ Ø4	
22.5 s	22.5 s	
	₹Ø8	
	22.5 s	

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	<u></u>	1	5	1	٦Y		
Traffic Volume (vph)	52	34	326	23	13	11	
Future Volume (vph)	52	34	326	23	13	11	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	0.95	
Frt	5.70	0.850	1.00	0.70	0.931	0.70	
Flt Protected		0.000	0.950		0.974		
Satd. Flow (prot)	3539	1583	1770	3539	3277	0	
Flt Permitted	0007	1000	0.619	0007	0.974		
Satd. Flow (perm)	3539	1583	1153	3539	3277	0	
Right Turn on Red	0007	Yes	1100	0007	0211	Yes	
Satd. Flow (RTOR)		37			12	103	
Link Speed (mph)	30			30	30		
Link Distance (ft)	491			971	1149		
Travel Time (s)	11.2			22.1	26.1		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	57	37	354	25	14	12	
Shared Lane Traffic (%)	57	57	334	20	14	12	
Lane Group Flow (vph)	57	37	354	25	26	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	24	Nynt	LCII	24	24	Ngn	
Link Offset(ft)	0			24	24		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane	10			10	10		
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	1.00	1.00	1.00	1.00	1.00	9	
Number of Detectors	2	9	10	2	10	7	
Detector Template	∠ Thru	Right	Left	Z	Left		
Leading Detector (ft)	100	20	20	100	20		
Trailing Detector (ft)	0	20	20	0	20		
Detector 1 Position(ft)	0	0	0	0	0		
Detector 1 Size(ft)	6	20	20	6	20		
()	o CI+Ex	CI+Ex	CI+Ex		CI+Ex		
Detector 1 Type	CI+EX	CI+EX	CI+EX	CI+Ex	UI+EX		
Detector 1 Channel Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		
Detector 1 Queue (s)	0.0	0.0	0.0	0.0 0.0	0.0		
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		
Detector 2 Position(ft)	0.0 94	0.0	0.0	0.0 94	0.0		
. ,	94						
Detector 2 Size(ft)				6 CI+Ex			
Detector 2 Type	CI+Ex			UI+EX			
Detector 2 Channel	0.0			0.0			
Detector 2 Extend (s)	0.0	Dorm	nment	0.0	Drot		
Turn Type Protected Phases	NA	Perm	pm+pt	NA	Prot		
	6		5	2	4		
Permitted Phases		6	2	2	4		
Detector Phase	6	6	5	2	4		
Switch Phase	F 0	F 0	F 0	F 0	F 0		
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0		

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	20.0	20.0	9.5	22.5	15.0	
Total Split (s)	15.0	15.0	20.0	35.0	15.0	
Total Split (%)	30.0%	30.0%	40.0%	70.0%	30.0%	
Maximum Green (s)	10.5	10.5	15.5	30.5	10.5	
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max	None	C-Max	None	
Walk Time (s)	7.0	7.0				
Flash Dont Walk (s)	11.0	11.0				
Pedestrian Calls (#/hr)	0	0				
Act Effct Green (s)	31.9	31.9	43.3	46.9	5.8	
Actuated g/C Ratio	0.64	0.64	0.87	0.94	0.12	
v/c Ratio	0.03	0.04	0.33	0.01	0.07	
Control Delay	6.1	3.7	2.0	1.0	15.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	6.1	3.7	2.0	1.0	15.0	
LOS	А	А	А	А	В	
Approach Delay	5.2			1.9	15.0	
Approach LOS	А			А	В	
Queue Length 50th (ft)	1	0	0	0	2	
Queue Length 95th (ft)	14	14	50	2	10	
Internal Link Dist (ft)	411			891	1069	
Turn Bay Length (ft)						
Base Capacity (vph)	2257	1023	1190	3321	697	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.03	0.04	0.30	0.01	0.04	
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50						
Offset: 0 (0%), Referenced	to phase 2	:WBTL ar	nd 6:EBT,	Start of (Green	
Natural Cycle: 45						
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.33						
Intersection Signal Delay: 3				lr	ntersection	LOS: A
Intersection Capacity Utilization	ation 36.4%)		10	CU Level o	of Service A
Analysis Period (min) 15						

Splits and Phases: 105:

✓ Ø2 (R)	•	↑ Ø4	
35 s		15 s	
√ Ø5	∎ → ¶Ø6 (R)		
20 s	15 s		

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		††			† †			•			•	
Traffic Volume (vph)	0	59	4	90	349	9	0	0	2	0	0	0
Future Volume (vph)	0	59	4	90	349	9	0	0	2	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.991			0.997			0.865				
Flt Protected					0.990							
Satd. Flow (prot)	0	3507	0	0	3493	0	0	1611	0	0	1863	0
Flt Permitted					0.990							
Satd. Flow (perm)	0	3507	0	0	3493	0	0	1611	0	0	1863	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		971			260			262			305	
Travel Time (s)		22.1			5.9			6.0			6.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	64	4	98	379	10	0	0	2	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	68	0	0	487	0	0	2	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		16			16			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
51	other											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 25.8%			IC	CU Level	of Service	A					

	-	\mathbf{r}	4	-	1	۲
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>	1		 ↑Ъ		
Traffic Volume (vph)	55	6	5	4 T 324	124	2
Future Volume (vph)	55	6	5	324	124	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
	1900	1900	1900	1900	1900	1900
Lane Width (ft)						
Lane Util. Factor	0.95	1.00	0.95	0.95	1.00	1.00
Frt Elt Droto etc.d		0.850		0.000	0.050	0.850
Flt Protected	0500	4/00		0.999	0.950	4500
Satd. Flow (prot)	3539	1689	0	3536	1770	1583
Flt Permitted	_			0.953	0.950	
Satd. Flow (perm)	3539	1689	0	3373	1770	1583
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		7				2
Link Speed (mph)	30			30	30	
Link Distance (ft)	777			738	307	
Travel Time (s)	17.7			16.8	7.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	60	7	5	352	135	2
Shared Lane Traffic (%)				302		-
Lane Group Flow (vph)	60	7	0	357	135	2
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	Left	Right
	16	R NA	Leit	16	26 Len	Right
Median Width(ft)						
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	4	0.00	1.00	1.00	1.00	1.00
Headway Factor	1.00	0.92	1.00	1.00	1.00	1.00
Turning Speed (mph)		15	15		15	9
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (ft)	100	20	20	100	20	20
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	6	20	20	6	20	20
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex		CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	6		5	2	4	
Permitted Phases		6	2			4
Detector Phase	6	6	5	2	4	4
Switch Phase	Ŭ	Ū	Ū	_		

01 Alt 01 AM Existing As Counted Existing 2021 volumes 7:45 am 03/24/2021 01 AM

	-	\mathbf{i}	•	-	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	9.5	22.5	22.5	22.5
Total Split (s)	20.0	20.0	8.5	28.5	21.5	21.5
Total Split (%)	40.0%	40.0%	17.0%	57.0%	43.0%	43.0%
Maximum Green (s)	15.5	15.5	4.0	24.0	17.0	17.0
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None
Walk Time (s)	7.0	7.0		7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0		11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0		0	0	0
Act Effct Green (s)	34.8	34.8		34.8	9.1	9.1
Actuated g/C Ratio	0.70	0.70		0.70	0.18	0.18
v/c Ratio	0.02	0.01		0.15	0.42	0.01
Control Delay	2.7	1.5		5.2	21.4	11.5
Queue Delay	0.0	0.0		0.0	0.0	0.0
Total Delay	2.7	1.5		5.2	21.4	11.5
LOS	А	А		А	С	В
Approach Delay	2.6			5.2	21.2	
Approach LOS	А			А	С	
Queue Length 50th (ft)	4	0		17	35	0
Queue Length 95th (ft)	1	0		48	69	4
Internal Link Dist (ft)	697			658	227	
Turn Bay Length (ft)						
Base Capacity (vph)	2465	1179		2350	601	539
Starvation Cap Reductn	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0
Reduced v/c Ratio	0.02	0.01		0.15	0.22	0.00
Intersection Summary						
Area Type:	Other					
Cycle Length: 50	e inter					
Actuated Cycle Length: 50)					
Offset: 0 (0%), Reference		:WBTL ar	nd 6:FBT	. Start of (Green	
Natural Cycle: 55					Croon	
Control Type: Actuated-Co	oordinated					

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.42

Intersection Signal Delay: 8.8 Intersection Capacity Utilization 26.8% Analysis Period (min) 15

Intersection LOS: A ICU Level of Service A

Splits and Phases: 107:

✓ Ø2 (R)		₩Ø4	
28.5 s		21.5 s	
√ Ø5	- ↓ Ø6 (R)		
8.5s	20 s		

Lane Group EBL EBR NBL NBT SBT SBR Traffic Volume (vph) 43 14 190 599 214 139 Future Volume (vph) 43 14 190 599 214 139 Ideal Flow (vph) 133 14 190 599 214 139 Ideal Flow (vph) 1900 1900 1900 1900 1900 1900 Lane Util. Factor 0.97 0.95 1.00 0.95 0.950 5339 3539 1583 Fit Protected 0.963 0.470 5359 3539 1583 783 Stat. Flow (perm) 335 0 875 3539 3539 1583 Right Turn on Red Yes Yes Yes Yes Yes Yes Stat. Flow (prh) 30 30 30 30 30 30 Link Distance (ft) 738 727 965 151 151 151 <
Lane Configurations111111Traffic Volume (vph)4314190599214139Future Volume (vph)4314190599214139Ideal Flow (vph)190019001900190019001900Lane Util. Factor0.970.951.000.950.951.00Fit Protected0.9630.9500.9550.8500.850Satd. Flow (prot)33550875353935391583Right Turn on RedYesYesYesYesSatd. Flow (perm)30303030Link Speed (mph)30303030Link Distance (ft)738727965Travel Time (s)16.816.521.9Peak Hour Factor0.920.920.920.920.92Adj. Flow (vph)4715207651233151Shared Lane Traffic (%)Lane Group Flow (vph)620207651233151Lane Group Flow (vph)620207651233151Enter Blocked IntersectionNoNoNoNoNoLane AlignmentLeftRightLeftLeftLeftRightMedian Width(ft)161616161616Travel Turn Lane915.035.020.020.020.0Protected Phases4 </td
Traffic Volume (vph) 43 14 190 599 214 139 Future Volume (vph) 43 14 190 599 214 139 Ideal Flow (vph) 1900 1900 1900 1900 1900 1900 Lane Util. Factor 0.97 0.95 1.00 0.95 0.95 0.850 Flt Protected 0.963 0.950 0.850 0.850 0.850 Satd. Flow (prot) 3355 0 875 3539 3539 1583 Right Turn on Red Yes Yes Yes Yes Yes Satd. Flow (prOR) 15 151 151 151 Link Distance (ft) 738 727 965 727 Travel Time (s) 16.8 16.5 21.9 92 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 47 15 207 651 233 151 Shared Lane Traffic (%)
Future Volume (vph) 43 14 190 599 214 139 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 Lane Util. Factor 0.97 0.95 1.00 0.95 0.95 0.850 Fit 0.964 0.950 5 0.850 1770 3539 3539 1583 Fit Perotected 0.963 0.470
Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 1900 Lane Util. Factor 0.97 0.95 1.00 0.95 0.95 1.00 Frt 0.964 0.950 0.950 0.850 Satd. Flow (prot) 3355 0 1770 3539 3539 1583 Flt Permitted 0.963 0.470 53539 3539 1583 Satd. Flow (perm) 3355 0 875 3539 3539 1583 Right Turn on Red Yes Yes Yes Yes 151 Link Speed (mph) 30 30 30 30 1151 Link Distance (ft) 738 727 965 12.9 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 47 15 207 651 233 151 Shared Lane Traffic (%) Lane Group Flow (v
Lane Util. Factor 0.97 0.95 1.00 0.95 1.00 Frt 0.964 0.950 0.850 Satd. Flow (prot) 3355 0 1770 3539 3539 1583 Flt Permitted 0.963 0.470 0.470 0.853 1583 Satd. Flow (perm) 3355 0 875 3539 3539 1583 Right Turn on Red Yes Yes Yes Yes Yes Satd. Flow (RTOR) 15 151 151 151 Link Speed (mph) 30 30 30 151 Link Distance (ft) 738 727 965 7724 Travel Time (s) 16.8 16.5 21.9 92 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 47 15 207 651 233 151 Enter Blocked Intersection No No No No No Median Widt
Frt 0.964 0.850 Fit Protected 0.963 0.950 Satd. Flow (prot) 3355 0 1770 3539 3539 1583 Fit Permitted 0.963 0.470 5359 3539 1583 Right Turn on Red Yes Yes Yes Yes Satd. Flow (perm) 3355 0 875 3539 3539 1583 Right Turn on Red Yes Yes Yes Yes Yes Satd. Flow (RTOR) 15 151 151 151 Link Distance (ft) 738 727 965 1772 795 Travel Time (s) 16.8 16.5 21.9 151 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 47 15 207 651 233 151 Enter Blocked Intersection No No No No No Link Offset(ft) 0 0 0
Filt Protected 0.963 0.950 Satd. Flow (prot) 3355 0 1770 3539 3539 1583 Filt Permitted 0.963 0.470
Satd. Flow (prot) 3355 0 1770 3539 3539 1583 Fit Permitted 0.963 0.470
Filt Permitted 0.963 0.470 Satd. Flow (perm) 3355 0 875 3539 3539 1583 Right Turn on Red Yes Yes Yes Yes Satd. Flow (RTOR) 15 151 151 Link Speed (mph) 30 30 30 30 Link Distance (ft) 738 727 965 965 Travel Time (s) 16.8 16.5 21.9 924 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 47 15 207 651 233 151 Shared Lane Traffic (%) 47 15 207 651 233 151 Enter Blocked Intersection No No No No No No No Lane Alignment Left Right Left Left Right Right 16 16 16 Travel Time Lane 100 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Satd. Flow (perm) 3355 0 875 3539 3539 1583 Right Turn on Red Yes Yes Yes Yes Satd. Flow (RTOR) 15 151 151 Link Speed (mph) 30 30 30 30 Link Distance (ft) 738 727 965 777 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 47 15 207 651 233 151 Shared Lane Traffic (%) 1 15 207 651 233 151 Enter Blocked Intersection No No No No No No Lane Alignment Left Right Left Left Right Median Width(ft) 36 12 12 12 Link Offset(ft) 0 0 0 0 0 100 Travel Time Lane Feadway Factor 1.00 1.00 1.00 1.00 1.00
Right Turn on Red Yes Yes Satd. Flow (RTOR) 15 151 Link Speed (mph) 30 30 30 Link Distance (ft) 738 727 965 Travel Time (s) 16.8 16.5 21.9 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 47 15 207 651 233 151 Shared Lane Traffic (%) Lane Group Flow (vph) 62 0 207 651 233 151 Enter Blocked Intersection No No No No No No Lane Alignment Left Right Left Left Right Median Width(ft) 36 12 12 12 Link Offset(ft) 0 0 0 0 Crosswalk Width(ft) 16 16 16 100 Turn Type Prot pm+pt NA NA Perm Protected Phases
Satd. Flow (RTOR) 15 151 Link Speed (mph) 30 30 30 Link Distance (ft) 738 727 965 Travel Time (s) 16.8 16.5 21.9 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 47 15 207 651 233 151 Shared Lane Traffic (%) Lane Group Flow (vph) 62 0 207 651 233 151 Enter Blocked Intersection No No No No No No Lane Alignment Left Right Left Left Right Median Width(ft) 16 16 Median Width(ft) 16 1.00 1.00 1.00 1.00 1.00 Turning Speed (mph) 15 9 15 9 9 Turn Type Prot pm+pt NA NA Perm Protected Phases 4 5 2 6 <td< td=""></td<>
Link Speed (mph) 30 30 30 Link Distance (ft) 738 727 965 Travel Time (s) 16.8 16.5 21.9 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 47 15 207 651 233 151 Shared Lane Traffic (%) 0 207 651 233 151 Shared Lane Traffic (%) No
Link Distance (ft) 738 727 965 Travel Time (s) 16.8 16.5 21.9 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 47 15 207 651 233 151 Shared Lane Traffic (%) Lane Group Flow (vph) 62 0 207 651 233 151 Enter Blocked Intersection No No No No No No Link Offset(ft) 36 12 12 12 12 12 14 16 16 16 100 1.00 <
Travel Time (s) 16.8 16.5 21.9 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 47 15 207 651 233 151 Shared Lane Traffic (%) 151 Lane Group Flow (vph) 62 0 207 651 233 151 Enter Blocked Intersection No No No No No No Lane Alignment Left Right Left Left Right Left Right Median Width(ft) 36 12 12 12 12 12 12 Link Offset(ft) 0 0 0 0 0 0 1.00 1
Peak Hour Factor 0.92
Adj. Flow (vph)4715207651233151Shared Lane Traffic (%)Lane Group Flow (vph)620207651233151Enter Blocked IntersectionNoNoNoNoNoNoLane AlignmentLeftRightLeftLeftLeftRightMedian Width(ft)36121212Link Offset(ft)0000Crosswalk Width(ft)16161616Two way Left Turn Lane1.001.001.001.001.00Headway Factor1.001.001.001.001.001.00Turning Speed (mph)1591599Turn TypeProtpm+ptNANAPermProtected Phases45266Minimum Split (s)22.59.522.522.522.5Total Split (%)30.0%30.0%30.0%70.0%40.0%Maximum Green (s)10.510.530.515.515.5Yellow Time (s)3.53.53.53.53.5All-Red Time (s)1.01.01.01.01.0Lost Time Adjust (s)0.00.00.00.00.0
Shared Lane Traffic (%) Lane Group Flow (vph) 62 0 207 651 233 151 Enter Blocked Intersection No No No No No No Lane Alignment Left Right Left Left Left Left Right Median Width(ft) 36 12 12 12 12 Link Offset(ft) 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 Two way Left Turn Lane 100 1.00 1.00 1.00 1.00 Headway Factor 1.00 1.00 1.00 1.00 1.00 Turning Speed (mph) 15 9 15 9 Turn Type Prot pm+pt NA NA Perm Protected Phases 4 5 2 6 6 Minimum Split (s) 22.5 9.5 22.5 22.5 22.5 22.5 22.5 22.5 22.5 22.5 10.1 10.0 10.0 10.0 10.0
Lane Group Flow (vph) 62 0 207 651 233 151 Enter Blocked Intersection No No No No No No Lane Alignment Left Right Left Left Left Right Median Width(ft) 36 12 12 12 Link Offset(ft) 0 0 0 0 Crosswalk Width(ft) 16 16 16 Two way Left Turn Lane 1.00 1.00 1.00 1.00 1.00 Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 Turn Type Prot pm+pt NA NA Perm Protected Phases 4 5 2 6 Minimum Split (s) 22.5 9.5 22.5 22.5 Total Split (s) 15.0 30.0% 30.0% 40.0% Maximum Green (s) 10.5 30.5 3.5 3.5 3.5 Yellow Time (s)
Enter Blocked Intersection No No No No No No Lane Alignment Left Right Left Left Left Right Median Width(ft) 36 12 12 12 Link Offset(ft) 0 0 0 0 Crosswalk Width(ft) 16 16 16 Two way Left Turn Lane 1.00 1.00 1.00 1.00 1.00 Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 Turn Type Prot pm+pt NA NA Perm Protected Phases 4 5 2 6 Minimum Split (s) 22.5 9.5 22.5 22.5 Total Split (s) 15.0 15.0 35.0 20.0 Total Split (%) 30.0% 30.0% 70.0% 40.0% Maximum Green (s) 10.5 30.5 3.5 3.5 3.5 Yellow Time (s) 3.0 1.0<
Lane Alignment Left Right Left Left Left Right Median Width(ft) 36 12 12 12 Link Offset(ft) 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 Two way Left Turn Lane 1.00 1.00 1.00 1.00 1.00 Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 Turning Speed (mph) 15 9 15 9 9 Turn Type Prot pm+pt NA NA Perm Protected Phases 4 5 2 6 6 Minimum Split (s) 22.5 9.5 22.5 22.5 22.5 Total Split (s) 15.0 15.0 35.0 20.0 20.0 Total Split (%) 30.0% 30.0% 70.0% 40.0% 40.0% Maximum Green (s) 10.5 10.5 30.5 3.5 3.5
Median Width(ft) 36 12 12 Link Offset(ft) 0 0 0 Crosswalk Width(ft) 16 16 16 Two way Left Turn Lane 1.00 1.00 1.00 1.00 1.00 Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 Turning Speed (mph) 15 9 15 9 Turn Type Prot pm+pt NA NA Perm Protected Phases 4 5 2 6 6 Minimum Split (s) 22.5 9.5 22.5 22.5 22.5 Total Split (s) 15.0 15.0 30.0% 40.0% 40.0% Maximum Green (s) 10.5 10.5 30.5 15.5 15.5 Yellow Time (s) 3.0 1.0 1.0 1.0 1.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0
Link Offset(ft) 0 0 0 Crosswalk Width(ft) 16 16 16 16 Two way Left Turn Lane 1.00 1.00 1.00 1.00 1.00 1.00 Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 Turning Speed (mph) 15 9 15 9 9 Turn Type Prot pm+pt NA NA Perm Protected Phases 2 6 6 6 Minimum Split (s) 22.5 9.5 22.5 22.5 22.5 Total Split (s) 15.0 15.0 30.0% 30.0% 40.0% 40.0% Maximum Green (s) 10.5 10.5 30.5 15.5 15.5 Yellow Time (s) 3.0 1.0 1.0 1.0 1.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0
Crosswalk Width(ft)16161616Two way Left Turn Lane1.001.001.001.001.00Headway Factor1.001.001.001.001.00Turning Speed (mph)159159Turn TypeProtpm+ptNANAPermProtected Phases4526Permitted Phases266Minimum Split (s)22.59.522.522.5Total Split (s)15.015.035.020.020.0Total Split (%)30.0%30.0%70.0%40.0%40.0%Maximum Green (s)10.510.530.515.515.5Yellow Time (s)3.53.53.53.53.53.5All-Red Time (s)1.01.01.01.01.01.0Lost Time Adjust (s)0.00.00.00.00.00.0
Two way Left Turn LaneHeadway Factor1.001.001.001.001.00Turning Speed (mph)159159Turn TypeProtpm+ptNANAPermProtected Phases4526Permitted Phases266Minimum Split (s)22.59.522.522.5Total Split (s)15.015.035.020.0Total Split (%)30.0%30.0%70.0%40.0%Maximum Green (s)10.510.530.515.5Yellow Time (s)3.53.53.53.5All-Red Time (s)1.01.01.01.0Lost Time Adjust (s)0.00.00.00.0
Headway Factor1.001.001.001.001.001.00Turning Speed (mph)159159Turn TypeProtpm+ptNANAPermProtected Phases4526Permitted Phases26Minimum Split (s)22.59.522.522.5Total Split (s)15.015.035.020.020.0Total Split (%)30.0%30.0%70.0%40.0%40.0%Maximum Green (s)10.510.530.515.515.5Yellow Time (s)3.53.53.53.53.5All-Red Time (s)1.01.01.01.01.0Lost Time Adjust (s)0.00.00.00.00.0
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Permitted Phases 2 6 Minimum Split (s) 22.5 9.5 22.5 22.5 22.5 Total Split (s) 15.0 15.0 35.0 20.0 20.0 Total Split (%) 30.0% 30.0% 70.0% 40.0% 40.0% Maximum Green (s) 10.5 10.5 30.5 15.5 15.5 Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 All-Red Time (s) 1.0 1.0 1.0 1.0 1.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0
Minimum Split (s)22.59.522.522.522.5Total Split (s)15.015.035.020.020.0Total Split (%)30.0%30.0%70.0%40.0%40.0%Maximum Green (s)10.510.530.515.515.5Yellow Time (s)3.53.53.53.53.5All-Red Time (s)1.01.01.01.01.0Lost Time Adjust (s)0.00.00.00.00.0
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All-Red Time (s)1.01.01.01.01.0Lost Time Adjust (s)0.00.00.00.00.0
Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0
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1 of all ost lime (s) 15 15 15 15 15
Lead/Lag Lag Lag
Lead-Lag Optimize? Yes Yes Yes
Walk Time (s) 7.0 7.0 7.0 7.0
Flash Dont Walk (s) 11.0 11.0 11.0 11.0
Pedestrian Calls (#/hr) 0 0 0
Act Effct Green (s) 10.5 30.5 30.5 15.5 15.5
Actuated g/C Ratio 0.21 0.61 0.61 0.31 0.31
v/c Ratio 0.09 0.29 0.30 0.21 0.25
Control Delay 10.6 5.5 5.1 13.4 4.3
Queue Delay 0.0 0.0 0.0 0.0 0.0
Total Delay 10.6 5.5 5.1 13.4 4.3

01 Alt 01 AM Existing As Counted Existing 2021 volumes 7:45 am 03/24/2021 01 AM

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
LOS	В		А	А	В	А
Approach Delay	10.6			5.2	9.8	
Approach LOS	В			А	А	
Queue Length 50th (ft)	7		22	39	25	0
Queue Length 95th (ft)	18		43	60	47	30
Internal Link Dist (ft)	658			647	885	
Turn Bay Length (ft)						
Base Capacity (vph)	716		721	2158	1097	594
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.09		0.29	0.30	0.21	0.25
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50						
Offset: 0 (0%), Referenced	to phase 2:1	VBTL and	6:SBT, 1	Start of G	reen, Mas	ster Inters
Natural Cycle: 55						
Control Type: Pretimed						
Maximum v/c Ratio: 0.30						
Intersection Signal Delay: 6					tersection	
Intersection Capacity Utiliz	ation 31.9%			IC	U Level o	of Service
Analysis Period (min) 15						
Splits and Phases: 108:						

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35 s		15 s	
▲ Ø5	Ø6 (R)		
15 s	20 s		

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Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations	†			1		
Traffic Volume (vph)	134	0	0	6	0	0
Future Volume (vph)	134	0	0	6	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						
Flt Protected						
Satd. Flow (prot)	1863	0	0	1863	0	0
Flt Permitted						
Satd. Flow (perm)	1863	0	0	1863	0	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	1082			1015	590	
Travel Time (s)	24.6			23.1	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	146	0	0	7	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	146	0	0	7	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 10.4%			IC	U Level o	of Service
Analysis Doriod (min) 15						

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	•			•	Y		
Traffic Volume (vph)	17	0	0	6	0	117	
Future Volume (vph)	17	0	0	6	0	117	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt					0.865		
Flt Protected							
Satd. Flow (prot)	1863	0	0	1863	1611	0	
Flt Permitted							
Satd. Flow (perm)	1863	0	0	1863	1611	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	839			634	538		
Travel Time (s)	19.1			14.4	12.2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	18	0	0	7	0	127	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	18	0	0	7	127	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	12		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion 17.2%			IC	CU Level o	of Service	e A
Analysis Poriod (min) 15							

Lane Group EBL EBT WBT WBR SBL SBR Lane Configurations Image: Configuration in the second
Traffic Volume (vph) 0 0 0 6 17 0 Future Volume (vph) 0 0 0 6 17 0 Ideal Flow (vphpl) 1900 1900 1900 1900 1900
Traffic Volume (vph) 0 0 0 6 17 0 Future Volume (vph) 0 0 0 6 17 0 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900
Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900
Lang [14] Easter 100 100 100 100 100 100
Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00
Frt 0.865
Flt Protected 0.950
Satd. Flow (prot) 0 1863 1611 0 1770 1863
Flt Permitted 0.950
Satd. Flow (perm) 0 1863 1611 0 1770 1863
Link Speed (mph) 30 30 30
Link Distance (ft) 98 839 286
Travel Time (s) 2.2 19.1 6.5
Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92
Adj. Flow (vph) 0 0 0 7 18 0
Shared Lane Traffic (%)
Lane Group Flow (vph) 0 0 7 0 18 0
Enter Blocked Intersection No No No No No No
Lane Alignment Left Left Left Right Left Right
Median Width(ft) 0 0 36
Link Offset(ft) 0 0 0
Crosswalk Width(ft) 16 16 16
Two way Left Turn Lane
Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00
Turning Speed (mph) 15 9 15 9
Sign Control Free Stop Stop
Intersection Summary
Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 13.3% ICU Level of Service A

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	A∿			<u></u>		1	
Traffic Volume (vph)	128	12	0	215	42	21	
Future Volume (vph)	128	12	0	215	42	21	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	0.95	0.95	1.00	0.95	1.00	1.00	
Frt	0.987					0.865	
Flt Protected					0.950		
Satd. Flow (prot)	3493	0	0	3539	0	1611	
Flt Permitted					0.950		
Satd. Flow (perm)	3493	0	0	3539	0	1611	
Link Speed (mph)	30			30	30		
Link Distance (ft)	404			491	211		
Travel Time (s)	9.2			11.2	4.8		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	139	13	0	234	46	23	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	152	0	0	234	46	23	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Right	
Median Width(ft)	12			24	0		
Link Offset(ft)	0			6	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion Err%			IC	CU Level	of Service	ЭН
Analysis Poriod (min) 15							

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Lane Group	SBL	SBR	NEL	NET	SWT	SWR	
Lane Configurations		77	ሻሻ	•			
Traffic Volume (vph)	0	495	123	0	0	0	
Future Volume (vph)	0	495	123	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	0.88	0.97	1.00	1.00	1.00	
Frt		0.850					
Flt Protected			0.950				
Satd. Flow (prot)	0	2787	3433	1863	0	0	
Flt Permitted			0.950				
Satd. Flow (perm)	0	2787	3433	1863	0	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	227			1186	549		
Travel Time (s)	5.2			27.0	12.5		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	538	134	0	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	538	134	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	12			36	36		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15	9	15			9	
Sign Control	Free			Free	Free		
Intersection Summary							
31	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	tion 20.6%			IC	CU Level o	of Service A	А
Analysis Dariad (min) 15							

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Lane Group	EBL	EBR	NEL	NET	SWT	SWR	
Lane Configurations				<u></u>	^	1	
Traffic Volume (vph)	0	0	0	0	205	291	
Future Volume (vph)	0	0	0	0	205	291	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	0.95	0.91	1.00	
Frt						0.850	
Flt Protected							
Satd. Flow (prot)	0	0	0	3539	5085	1583	
Flt Permitted							
Satd. Flow (perm)	0	0	0	3539	5085	1583	
Link Speed (mph)	30			30	30		
Link Distance (ft)	1042			233	359		
Travel Time (s)	23.7			5.3	8.2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	0	0	0	223	316	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	0	0	0	223	316	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	8			0	24		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15	9	15			9	
Sign Control	Free			Free	Free		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion 21.4%			IC	CU Level	of Service <i>I</i>	А
Analysis Doriod (min) 15							

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Lane Group	SBL	SBR	NWL	NWR	NEL	NER
Lane Configurations		<u>, 11</u>		10001	ሻሻ	
Traffic Volume (vph)	1 0	6 1 1 496	1 0	r 0	11 123	r 0
Future Volume (vph)	0	490	0	0	123	0
Ideal Flow (vphpl)	1900	490	1900	1900	123	1900
Lane Util. Factor	1.00	0.88	1.00	1.00	0.97	1.00
	1.00		1.00	1.00	0.97	1.00
Frt Flt Protected		0.850			0.950	
	10/0	2202	10/0	10/0		10/0
Satd. Flow (prot)	1863	2787	1863	1863	3433	1863
Flt Permitted	10/0	2207	10/0	10/0	0.950	10/0
Satd. Flow (perm)	1863	2787	1863	1863	3433	1863
Right Turn on Red		Yes		Yes		Yes
Satd. Flow (RTOR)		1920				
Link Speed (mph)	30		30		30	
Link Distance (ft)	763		723		430	
Travel Time (s)	17.3		16.4		9.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	539	0	0	134	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	539	0	0	134	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	30	3	32	- <u></u> - <u></u>	32	.3
Link Offset(ft)	30		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane	10		10		10	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	9	1.00	9	1.00	9
Number of Detectors	0	9	0	9	0	9
Detector Template	Thru	Thru	Thru	Thru	Thru	Thru
Leading Detector (ft)	0	0	0	0	0	0
Trailing Detector (ft)	0	0	0	0	0	0
Turn Type	Prot	pt+ov	Prot	Perm	Prot	Perm
Protected Phases	5	56	4		6	
Permitted Phases				4		6
Detector Phase	5	56	4	4	6	6
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (s)	15.5		15.5	15.5	19.0	19.0
Total Split (%)	31.0%		31.0%	31.0%	38.0%	38.0%
Maximum Green (s)	11.0		11.0	11.0	14.5	14.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
			4.5		4.5	4.5
Total Lost Time (s)	4.5		4.5	4.5		
Lead/Lag	Lead				Lag	Lag
Lead-Lag Optimize?	Yes				Yes	Yes
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	None		None	None	C-Max	C-Max

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Lane Group	SBL	SBR	NWL	NWR	NEL	NER		
Walk Time (s)	7.0		7.0	7.0	7.0	7.0		
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	11.0		
Pedestrian Calls (#/hr)	0		0	0	0	0		
Act Effct Green (s)		50.0			35.5			
Actuated g/C Ratio		1.00			0.71			
v/c Ratio		0.19			0.05			
Control Delay		0.4			2.3			
Queue Delay		0.0			0.0			
Total Delay		0.4			2.3			
LOS		А			А			
Approach Delay	0.4				2.3			
Approach LOS	А				А			
Queue Length 50th (ft)		0			4			
Queue Length 95th (ft)		0			8			
Internal Link Dist (ft)	683		643		350			
Turn Bay Length (ft)								
Base Capacity (vph)		2787			2437			
Starvation Cap Reductn		0			0			
Spillback Cap Reductn		0			0			
Storage Cap Reductn		0			0			
Reduced v/c Ratio		0.19			0.05			
Intersection Summary								
	Other							
Cycle Length: 50								
Actuated Cycle Length: 50								
Offset: 0 (0%), Referenced t	o phase 6:1	VEL, Star	t of Gree	n				
Natural Cycle: 70								
Control Type: Actuated-Coo	rdinated							
Maximum v/c Ratio: 0.19								
Intersection Signal Delay: 0.					tersection			
Intersection Capacity Utilizat	tion 21.1%			IC	U Level o	f Service A		
Analysis Period (min) 15								
Splits and Phases: 102:								
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▲ ø5	₩ Ø6 (R)	₩ ⁴ ø4

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>	-LBR		1	ኘ	1001
Traffic Volume (vph)	123	0	0	257	239	17
Future Volume (vph)	123	0	0	257	239	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	1.00
Frt	0.75	1.00	1.00	0.75	0.77	0.850
Flt Protected					0.950	0.030
Satd. Flow (prot)	3539	0	0	3539	3433	1583
Flt Permitted	0007	0	0	2004	0.950	1303
	3539	0	0	3539	3433	1583
Satd. Flow (perm)	2028	Yes	0	2028	3433	Yes
Right Turn on Red		res				
Satd. Flow (RTOR)	20			20	20	18
Link Speed (mph)	30			30	30	
Link Distance (ft)	923			533	500	
Travel Time (s)	21.0		0.00	12.1	11.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	134	0	0	279	260	18
Shared Lane Traffic (%)						
Lane Group Flow (vph)	134	0	0	279	260	18
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Number of Detectors	2			2	1	1
Detector Template	Thru			Thru	Left	Right
Leading Detector (ft)	100			100	20	20
Trailing Detector (ft)	0			0	0	0
Detector 1 Position(ft)	0			0	0	0
Detector 1 Size(ft)	6			6	20	20
Detector 1 Type	CI+Ex			CI+Ex	CI+Ex	CI+Fx
Detector 1 Channel	CITLA			CITLA	CITLA	CITEX
	0.0			0.0	0.0	0.0
Detector 1 Extend (s)						
Detector 1 Queue (s)	0.0			0.0	0.0	0.0
Detector 1 Delay (s)	0.0			0.0	0.0	0.0
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA			NA	Prot	Perm
Protected Phases	6			2	4	
Permitted Phases						4
Detector Phase	6			2	4	4
Switch Phase						
Minimum Initial (s)	5.0			5.0	5.0	5.0
	0.0			0.0	0.0	0.0

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Splits and Phases: 103:

← Ø2 (R)	▲ \Ø4	
27.5 s	22.5 s	
₩Ø6 (R) 27.5 s		
27.5 s		

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	101 101				<u>NDL</u>	
Traffic Volume (vph)	123	0	0	257	0	0
Future Volume (vph)	123	0	0	257	0	0
Ideal Flow (vphpl)	123	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	0.95	1.00	0.95	1.00	1.00
Frt	0.70	0.75	1.00	0.75	1.00	1.00
Flt Protected						
Satd. Flow (prot)	3539	0	1863	3539	1863	1863
Flt Permitted	3337	0	1005	5557	1005	1005
Satd. Flow (perm)	3539	0	1863	3539	1863	1863
Right Turn on Red	3337	Yes	1005	3337	1003	Yes
Satd. Flow (RTOR)		103				162
Link Speed (mph)	30			30	30	
Link Speed (mph)	533			404	428	
• •				404 9.2		
Travel Time (s)	12.1	0.00	0.00		9.7	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	134	0	0	279	0	0
Shared Lane Traffic (%)	104	0	0	070	0	0
Lane Group Flow (vph)	134	0	0	279	0	0
Enter Blocked Intersection	No	No	No	No	No	No Diabt
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	24			24	20	
Link Offset(ft)	-12			8	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Turn Type	NA		Perm	NA	Prot	Perm
Protected Phases	4			8	2	
Permitted Phases			8			2
Minimum Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (%)	50.0%		50.0%	50.0%	50.0%	50.0%
Maximum Green (s)	18.0		18.0	18.0	18.0	18.0
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag						
Lead-Lag Optimize?						
Walk Time (s)	7.0		7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0		0	0	0	0
Act Effct Green (s)	18.0			18.0		
Actuated g/C Ratio	0.40			0.40		
v/c Ratio	0.09			0.20		
Control Delay	8.7			9.3		
Queue Delay	0.0			0.0		
Total Delay	8.7			9.3		
	0.7			7.5		

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
LOS	А			А		
Approach Delay	8.7			9.3		
Approach LOS	А			А		
Queue Length 50th (ft)	10			23		
Queue Length 95th (ft)	22			42		
Internal Link Dist (ft)	453			324	348	
Turn Bay Length (ft)						
Base Capacity (vph)	1415			1415		
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.09			0.20		
Intersection Summary						
Area Type:	Other					
Cycle Length: 45						
Actuated Cycle Length: 4						
Offset: 0 (0%), Reference	ed to phase 2:	IBL and (6:, Start o	of Green		
Natural Cycle: 45						
Control Type: Pretimed						
Maximum v/c Ratio: 0.20						
Intersection Signal Delay				In	tersection	LOS: A
Intersection Capacity Util	ization 10.9%			IC	U Level o	f Service A
Analysis Period (min) 15						

Splits and Phases: 104:

1 √iø2 (R)	→ Ø4	
22.5 s	22.5 s	
	₩	
	♥ Ø8	
	22.5 s	

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	† †	1	5	^	٦¥	
Traffic Volume (vph)	142	7	34	55	160	402
Future Volume (vph)	142	7	34	55	160	402
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	0.95
Frt	5170	0.850		5170	0.893	0.70
Flt Protected		2.000	0.950		0.986	
Satd. Flow (prot)	3539	1583	1770	3539	3182	0
Flt Permitted			0.557	0007	0.986	Ŭ
Satd. Flow (perm)	3539	1583	1038	3539	3182	0
Right Turn on Red	0007	Yes	1000	0007	0102	Yes
Satd. Flow (RTOR)		8			437	103
Link Speed (mph)	30	U		30	437	
Link Distance (ft)	491			971	1149	
Travel Time (s)	491 11.2			22.1	26.1	
.,		0.00	0.00			0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	154	8	37	60	174	437
Shared Lane Traffic (%)	1 - 4	0	07	10	/ 1 1	0
Lane Group Flow (vph)	154	8	37	60	611	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	24			24	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Number of Detectors	2	1	1	2	1	
Detector Template	Thru	Right	Left	Thru	Left	
Leading Detector (ft)	100	20	20	100	20	
Trailing Detector (ft)	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	
Detector 1 Size(ft)	6	20	20	6	20	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)	94	0.0	0.0	94	0.0	
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel				OT LA		
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	pm+pt	NA	Prot	
Protected Phases	6	1 CIIII	рш+рі 5	2	4	
Permitted Phases	U	6	2	2	4	
Detector Phase	6	6	5	2	4	
Switch Phase	U	U	5	2	4	
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	
	0.C	0.C	0.C	0.C	0.C	

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	20.0	20.0	9.5	22.5	15.0	
Total Split (s)	15.0	15.0	20.0	35.0	15.0	
Total Split (%)	30.0%	30.0%	40.0%	70.0%	30.0%	
Maximum Green (s)	10.5	10.5	15.5	30.5	10.5	
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max	None	C-Max	None	
Walk Time (s)	7.0	7.0				
Flash Dont Walk (s)	11.0	11.0				
Pedestrian Calls (#/hr)	0	0				
Act Effct Green (s)	28.3	28.3	32.7	32.7	8.3	
Actuated g/C Ratio	0.57	0.57	0.65	0.65	0.17	
v/c Ratio	0.08	0.01	0.05	0.03	0.69	
Control Delay	7.5	6.1	4.2	3.9	10.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	7.5	6.1	4.2	3.9	10.0	
LOS	А	А	А	А	В	
Approach Delay	7.5			4.0	10.0	
Approach LOS	А			А	В	
Queue Length 50th (ft)	6	0	3	2	23	
Queue Length 95th (ft)	28	6	3	1	58	
Internal Link Dist (ft)	411			891	1069	
Turn Bay Length (ft)						
Base Capacity (vph)	2005	900	906	2315	1013	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.08	0.01	0.04	0.03	0.60	
Intersection Summary						
21	Other					
Cycle Length: 50						
Actuated Cycle Length: 50						
Offset: 0 (0%), Referenced	to phase 2	:WBTL ar	nd 6:EBT,	Start of (Green	
Natural Cycle: 45						
Control Type: Actuated-Coo	ordinated					
Maximum v/c Ratio: 0.69						
Intersection Signal Delay: 8					ntersection	
Intersection Capacity Utiliza	ation 36.9%)		10	CU Level o	of Service A
Analysis Period (min) 15						

Splits and Phases: 105:

✓ Ø2 (R)		▲ Ø4
35 s		15 s
√ Ø5	▼	
20 s	15 s	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<u></u>			<u>††</u>			•			•	
Traffic Volume (vph)	0	544	0	1	71	1	16	0	65	0	0	0
Future Volume (vph)	0	544	0	1	71	1	16	0	65	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.998			0.891				
Flt Protected					0.999			0.990				
Satd. Flow (prot)	0	3539	0	0	3529	0	0	1643	0	0	1863	0
Flt Permitted					0.999			0.990				
Satd. Flow (perm)	0	3539	0	0	3529	0	0	1643	0	0	1863	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		971			260			262			305	
Travel Time (s)		22.1			5.9			6.0			6.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	591	0	1	77	1	17	0	71	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	591	0	0	79	0	0	88	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		16			16			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
51)ther											
Control Type: Unsignalized												
Intersection Capacity Utilization 26.6% ICU Level of Service A												

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>			 ↑Ъ		
Traffic Volume (vph)	TT 340	214	2	4 T 73	0	11
Future Volume (vph)	340	214	2	73	0	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	1700	14	1900	1300	1300	1700
Lane Util. Factor	0.95	1.00	0.95	0.95	1.00	1.00
Frt	0.75	0.850	0.75	0.75	1.00	0.850
Flt Protected		0.000		0.999		0.000
Satd. Flow (prot)	3539	1689	0	3536	1863	1583
Flt Permitted	5557	1007	U	0.951	1005	1303
Satd. Flow (perm)	3539	1689	0	3366	1863	1583
Right Turn on Red	3034	Yes	U	3300	1003	Yes
Satd. Flow (RTOR)		233				406
· /	30	233		30	30	400
Link Speed (mph)						
Link Distance (ft)	777			738	307	
Travel Time (s)	17.7	0.00	0.00	16.8	7.0	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	370	233	2	79	0	12
Shared Lane Traffic (%)	270	222	0	01	0	10
Lane Group Flow (vph)	370	233	0	81 No	0	12 No
Enter Blocked Intersection	No	NO	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	Left	Right
Median Width(ft)	16			16	36	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	1.00	0.00	1.00	1.00	1.00	1.00
Headway Factor	1.00	0.92	1.00	1.00	1.00	1.00
Turning Speed (mph)	-	15	15	-	15	9
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (ft)	100	20	20	100	20	20
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	6	20	20	6	20	20
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	6		5	2	4	
Permitted Phases		6	2			4
Detector Phase	6	6	5	2	4	4
Switch Phase						

02 Alt 01 PM Existing As Counted 10:06 am 04/12/2021

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	9.5	22.5	22.5	22.5
Total Split (s)	20.0	20.0	8.5	28.5	21.5	21.5
Total Split (%)	40.0%	40.0%	17.0%	57.0%	43.0%	43.0%
Maximum Green (s)	15.5	15.5	4.0	24.0	17.0	17.0
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	1.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5
Lead/Lag			Lead	4.5	4.0	4.0
0	Lag	Lag				
Lead-Lag Optimize?	Yes	Yes	Yes	2.0	2.0	2.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None
Walk Time (s)	7.0	7.0		7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0		11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0		0	0	0
Act Effct Green (s)	47.1	47.1		47.1		5.5
Actuated g/C Ratio	0.94	0.94		0.94		0.11
v/c Ratio	0.11	0.15		0.03		0.02
Control Delay	1.0	0.7		2.4		0.1
Queue Delay	0.0	0.0		0.0		0.0
Total Delay	1.0	0.7		2.4		0.1
LOS	А	А		А		А
Approach Delay	0.9			2.4	0.1	
Approach LOS	А			А	А	
Queue Length 50th (ft)	0	0		0		0
Queue Length 95th (ft)	23	m0		15		0
Internal Link Dist (ft)	697			658	227	J
Turn Bay Length (ft)	0,1			500		
Base Capacity (vph)	3334	1604		3171		806
Starvation Cap Reductn	0	0		0		000
Spillback Cap Reductin	0	0		0		0
Storage Cap Reductin	0	0		0		0
•	0.11	0.15		0.03		0.01
Reduced v/c Ratio	0.11	0.15		0.03		0.01
Intersection Summary	Othor					
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50				_		
Offset: 0 (0%), Referenced	to phase 2	:WBTL ar	nd 6:EBT,	Start of (Green	
Natural Cycle: 55						
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.15						
Intersection Signal Delay: 1	1.0			lr	ntersectio	n LOS: A
Intersection Capacity Utilization)		IC	CU Level	of Service
Analysis Period (min) 15						
m Volume for 95th percer	ntile queue	is metere	d by upst	ream sigr	nal.	

Lanes, Volumes, Timings 107:

 Splits and Phases:
 107:

 ✓ Ø2 (R)
 ✓ Ø4

 28.5 s
 21.5 s

 ✓ Ø5
 ✓ Ø6 (R)

 8.5 s
 20 s

Lane Group Lane Configurations Traffic Volume (vph) Future Volume (vph)	EBL	EBR	NBL	NBT		
Lane Configurations Traffic Volume (vph)	ኘ ¥ 282	LDR		INP I	SBT	SBR
Traffic Volume (vph)	282		۲		^	
· · · ·		69	24	702	553	51
	282	69	24	702	553	51
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.97	0.95	1.00	0.95	0.95	1.00
Frt	0.971	0.75	1.00	0.75	0.75	0.850
Flt Protected	0.961		0.950			0.000
Satd. Flow (prot)	3372	0	1770	3539	3539	1583
Flt Permitted	0.961	0	0.269	0007	0007	1303
Satd. Flow (perm)	3372	0	501	3539	3539	1583
Right Turn on Red	5572	Yes	301	5557	5557	Yes
Satd. Flow (RTOR)	56	163				55
Link Speed (mph)	30			30	30	
Link Distance (ft)	738			727	965	
Travel Time (s)	16.8			16.5	21.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0.92 307	0.92	0.92	763	601	0.92
Shared Lane Traffic (%)	307	75	20	103	001	00
Lane Group Flow (vph)	382	0	26	763	601	55
Enter Blocked Intersection	382 No	No	20 No	No	No	SS No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	26 Len	RIGHT	Leit	12	12	Right
Link Offset(ft)	30 0			12	12	
Crosswalk Width(ft)	16			16	16	
	10			10	10	
Two way Left Turn Lane Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	Prot	У		NIA	NA	9 Perm
Turn Type Protected Phases			pm+pt	NA 2	NA 6	Pelill
	4		5 2	Z	0	4
Permitted Phases	77 E			<u> Э</u> Э Е	<u> Э</u> Э Е	6 22 5
Minimum Split (s)	22.5		9.5	22.5	22.5	22.5
Total Split (s)	15.0		15.0	35.0	20.0	20.0
Total Split (%)	30.0%		30.0%	70.0%	40.0%	40.0%
Maximum Green (s)	10.5		10.5	30.5	15.5	15.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag			Lead		Lag	Lag
Lead-Lag Optimize?			Yes		Yes	Yes
Walk Time (s)	7.0			7.0	7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	10.5		30.5	30.5	15.5	15.5
Actuated g/C Ratio	0.21		0.61	0.61	0.31	0.31
v/c Ratio	0.51		0.05	0.35	0.55	0.10
Control Delay	16.4		4.1	5.4	16.6	5.1
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	16.4		4.1	5.4	16.6	5.1

02 Alt 01 PM Existing As Counted 10:06 am 04/12/2021

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
LOS	В		А	А	В	А	
Approach Delay	16.4			5.4	15.7		
Approach LOS	В			А	В		
Queue Length 50th (ft)	37		3	47	75	0	
Queue Length 95th (ft)	73		9	72	116	19	
Internal Link Dist (ft)	658			647	885		
Turn Bay Length (ft)							
Base Capacity (vph)	752		572	2158	1097	528	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.51		0.05	0.35	0.55	0.10	
Intersection Summary							
Area Type:	Other						
Cycle Length: 50							
Actuated Cycle Length: 50							
Offset: 0 (0%), Referenced	to phase 2:	VBTL and	6:SBT,	Start of G	reen, Mas	ster Inters	sectior
Natural Cycle: 55							
Control Type: Pretimed							
Maximum v/c Ratio: 0.55							
Intersection Signal Delay:				In	tersection	LOS: B	
Intersection Capacity Utiliz	ation 37.7%			IC	U Level c	of Service	A
Analysis Period (min) 15							
Splits and Phases: 108:							

		≯ _{Ø4}	
35 s		15 s	
▲ Ø5	● ♥ Ø6 (R)		
15 s	20 s		

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Lane Group	NBT	NBR	SBL	SBT	NWL	NWR	
Lane Configurations	1			†			
Traffic Volume (vph)	11	0	216	0	0	0	
Future Volume (vph)	11	0	216	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected				0.950			
Satd. Flow (prot)	1863	0	0	1770	0	0	
Flt Permitted				0.950			
Satd. Flow (perm)	1863	0	0	1770	0	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	1082			1015	590		
Travel Time (s)	24.6			23.1	13.4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	12	0	235	0	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	12	0	0	235	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	0		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type: (Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	tion 22.0%			IC	U Level o	of Service	e A
Analysis Poriod (min) 15							

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	•			•	Y		
Traffic Volume (vph)	0	0	0	0	0	11	
Future Volume (vph)	0	0	0	0	0	11	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt					0.865		
Flt Protected							
Satd. Flow (prot)	1863	0	0	1863	1611	0	
Flt Permitted							
Satd. Flow (perm)	1863	0	0	1863	1611	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	839			634	538		
Travel Time (s)	19.1			14.4	12.2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	0	0	0	0	12	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	0	0	0	12	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	12		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion 13.3%			IC	U Level o	of Service	eΑ
Analysis Poriod (min) 15							

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EBL	EBT	WBT	WBR	SBL	SBR
	ર્સ	f,		ኘ	1
0	0	0	0	0	0
0	0	0	0	0	0
1900	1900	1900	1900	1900	1900
1.00	1.00	1.00	1.00	1.00	1.00
0	1863	1863	0	1863	1863
0	1863	1863	0	1863	1863
	30	30		30	
	98	839		286	
	2.2	19.1		6.5	
0.92	0.92	0.92	0.92	0.92	0.92
0	0	0	0	0	0
0	0	0	0	0	0
No	No	No	No	No	No
Left	Left	Left	Right	Left	Right
	0	0	Ū	36	Ū.
	0	0		0	
	16	16		16	
1.00	1.00	1.00	1.00	1.00	1.00
15			9	15	9
	Free	Stop		Stop	
ther					
on 13.3%			IC	U Level o	of Service A
	0 0 1900 1.00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 1900 1900 1900 1.00 1.00 0 1863 0 1863 0 1863 0 1863 0 1863 0 1863 0 1863 0 0 0 0.92 0 0 0 0 0 0 0 0 0 0 0 0 1.00 1.00 15 Free	Image: style	Image: constraint of the state intermediate inttermediate intermediate intermediate intermediate intermediate in	0 1900 1900

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	A			<u></u>		1	
Traffic Volume (vph)	144	0	0	60	0	0	
Future Volume (vph)	144	0	0	60	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	0.95	0.95	1.00	0.95	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	3539	0	0	3539	0	1863	
Flt Permitted							
Satd. Flow (perm)	3539	0	0	3539	0	1863	
Link Speed (mph)	30			30	30		
Link Distance (ft)	404			491	211		
Travel Time (s)	9.2			11.2	4.8		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	157	0	0	65	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	157	0	0	65	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Right	
Median Width(ft)	12			24	0		
Link Offset(ft)	0			6	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
JI -	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	ion 7.3%			IC	U Level o	of Service A	А
Analysis Period (min) 15							

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Lane Group	SBL	SBR	NEL	NET	SWT	SWR
Lane Configurations		77	ሻሻ	•		
Traffic Volume (vph)	0	40	192	592	0	0
Future Volume (vph)	0	40	192	592	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.88	0.97	1.00	1.00	1.00
Frt		0.850				
Flt Protected			0.950			
Satd. Flow (prot)	0	2787	3433	1863	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	2787	3433	1863	0	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	227			1186	549	
Travel Time (s)	5.2			27.0	12.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	43	209	643	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	43	209	643	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			36	36	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Free			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 34.5%			IC	CU Level o	of Service A
Analysis Poriod (min) 15						

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Lane Group	EBL	EBR	NEL	NET	SWT	SWR
Lane Configurations				††	^	1
Traffic Volume (vph)	0	0	0	470	14	10
Future Volume (vph)	0	0	0	470	14	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	0.91	1.00
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	0	0	3539	5085	1583
Flt Permitted						
Satd. Flow (perm)	0	0	0	3539	5085	1583
Link Speed (mph)	30			30	30	
Link Distance (ft)	1042			233	359	
Travel Time (s)	23.7			5.3	8.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	511	15	11
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	511	15	11
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	8			0	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Free			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization	tion 16.3%			IC	U Level	of Service
Analysis Dariad (min) 15						

	L.	¥	Ť	•	•	ĩ
Lane Group	SBL	SBR	NWL	NWR	NEL	NER
Lane Configurations				7	ሻሻ	
Traffic Volume (vph)	• 1 0	6 1 1	0	r	784	.
Future Volume (vph)	0	40	0	0	784	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1,00	0.88	1.00	1,00	0.97	1,00
Frt	1.00	0.850	1.00	1.00	0.97	1.00
Fit Protected		0.000			0.950	
Satd. Flow (prot)	1863	2787	1863	1863	3433	1863
Fit Permitted	1003	2101	1003	1003	3433 0.950	1003
	1040	2202	10/0	1040		1040
Satd. Flow (perm)	1863	2787	1863	1863 Voc	3433	1863 Voc
Right Turn on Red		Yes		Yes		Yes
Satd. Flow (RTOR)		1920	0.0		00	
Link Speed (mph)	30		30		30	
Link Distance (ft)	763		723		430	
Travel Time (s)	17.3	_	16.4	_	9.8	-
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	43	0	0	852	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	43	0	0	852	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	30		32		32	
Link Offset(ft)	30		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15	9	15	9
Number of Detectors	0	0	0	0	0	0
Detector Template	Thru	Thru	Thru	Thru	Thru	Thru
Leading Detector (ft)	0	0	0	0	0	0
Trailing Detector (ft)	0	0	0	0	0	0
Turn Type	Prot	pt+ov	Prot	Perm	Prot	Perm
Protected Phases	5	pt+0v 5 6	4	1 CIIII	6	1 CIIII
Permitted Phases	ິ	5.0	4	1	U	6
	E	Ε 4	A	4	L	6
Detector Phase	5	56	4	4	6	6
Switch Phase	F 0		F 0	F 0	5.0	F 0
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (s)	15.5		15.5	15.5	19.0	19.0
Total Split (%)	31.0%		31.0%	31.0%	38.0%	38.0%
Maximum Green (s)	11.0		11.0	11.0	14.5	14.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag	Lead				Lag	Lag
Lead-Lag Optimize?	Yes				Yes	Yes
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	None		None	None	C-Max	C-Max
	NULLE		NULLE	NULLE		

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Lane Group	SBL	SBR	NWL	NWR	NEL	NER	
Walk Time (s)	7.0		7.0	7.0	7.0	7.0	
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	11.0	
Pedestrian Calls (#/hr)	0		0	0	0	0	
Act Effct Green (s)		50.0			44.2		
Actuated g/C Ratio		1.00			0.88		
v/c Ratio		0.02			0.28		
Control Delay		0.0			1.8		
Queue Delay		0.0			0.0		
Total Delay		0.0			1.8		
LOS		А			А		
Approach Delay					1.8		
Approach LOS					А		
Queue Length 50th (ft)		0			0		
Queue Length 95th (ft)		0			51		
Internal Link Dist (ft)	683		643		350		
Turn Bay Length (ft)							
Base Capacity (vph)		2787			3035		
Starvation Cap Reductn		0			0		
Spillback Cap Reductn		0			0		
Storage Cap Reductn		0			0		
Reduced v/c Ratio		0.02			0.28		
Intersection Summary							
Area Type:	Other						
Cycle Length: 50							
Actuated Cycle Length: 50							
Offset: 0 (0%), Referenced	to phase 6:1	VEL, Star	t of Gree	n			
Natural Cycle: 70	1						
Control Type: Actuated-Coc	ordinated						
Maximum v/c Ratio: 0.28							
Intersection Signal Delay: 1	.7			In	tersection	LOS: A	
Intersection Capacity Utiliza					U Level o		А
Analysis Period (min) 15							
Splits and Phases: 102:							

A 05	• 😾 Ø6 (R)	▶ [≜] Ø4	
15.5 s	19 s	15.5 s	

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations		LDIN	VVDL	****	NDL آآ	
Traffic Volume (vph)	TT 115	0	0	TT 40	0	·
Future Volume (vph)	115	0	0	40	0	0
Ideal Flow (vphpl)	1900	1900	1900	40	1900	1900
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	1.00
Frt	0.93	1.00	1.00	0.95	0.97	1.00
Fit Protected						
	3539	0	0	3539	3614	1863
Satd. Flow (prot) Flt Permitted	2028	U	0	2028	3014	1003
	2520	0	0	2520	2/1/	10/0
Satd. Flow (perm)	3539	0	0	3539	3614	1863
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						
Link Speed (mph)	30			30	30	
Link Distance (ft)	923			533	500	
Travel Time (s)	21.0			12.1	11.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	125	0	0	43	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	125	0	0	43	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12		2011	12	24	····gin
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	10			10	10	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
	1.00			1.00		
Turning Speed (mph)	- 1	9	15	0	15	9
Number of Detectors	2 That			2	1	1 Diadat
Detector Template	Thru			Thru	Left	Right
Leading Detector (ft)	100			100	20	20
Trailing Detector (ft)	0			0	0	0
Detector 1 Position(ft)	0			0	0	0
Detector 1 Size(ft)	6			6	20	20
Detector 1 Type	CI+Ex			CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0			0.0	0.0	0.0
Detector 1 Queue (s)	0.0			0.0	0.0	0.0
Detector 1 Delay (s)	0.0			0.0	0.0	0.0
Detector 2 Position(ft)	94			94	0.0	0.0
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel	CI+EX					
	0.0			0.0		
Detector 2 Extend (s)	0.0			0.0	Dart	Deer
Turn Type	NA			NA	Prot	Perm
Protected Phases	6			2	4	
Permitted Phases						4
Detector Phase	6			2	4	4
Switch Phase						
Minimum Initial (s)	5.0			5.0	5.0	5.0

	-	\mathbf{r}	1	-	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	22.5	LBR	WDL	22.5	22.5	22.5
Total Split (s)	22.5			22.5	22.5	22.5
Total Split (%)	50.0%			50.0%	50.0%	50.0%
Maximum Green (s)	18.0			18.0	18.0	18.0
Yellow Time (s)	3.5			3.5	3.5	3.5
All-Red Time (s)	1.0			1.0	1.0	1.0
Lost Time Adjust (s)	0.0			0.0	0.0	0.0
Total Lost Time (s)	4.5			4.5	4.5	4.5
Lead/Lag	1.0			1.0	1.0	1.0
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0			3.0	3.0	3.0
Recall Mode	C-Max			C-Max	None	None
Walk Time (s)	7.0			7.0	7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	45.0			45.0	0	0
Actuated g/C Ratio	1.00			1.00		
v/c Ratio	0.04			0.01		
Control Delay	0.0			0.0		
Queue Delay	0.0			0.0		
Total Delay	0.0			0.0		
LOS	A			A		
Approach Delay	<i>,</i> ,			,,		
Approach LOS						
Queue Length 50th (ft)	0			0		
Queue Length 95th (ft)	0			1		
Internal Link Dist (ft)	843			453	420	
Turn Bay Length (ft)	0.0					
Base Capacity (vph)	3539			3539		
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.04			0.01		
Intersection Summary						
Area Type:	Other					
Cycle Length: 45	Outor					
Actuated Cycle Length: 45						
Offset: 0 (0%), Referenced		WBT and	6:FBT.	Start of G	reen	
Natural Cycle: 45			0.2017		10011	
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.04						
Intersection Signal Delay:	0.0			lr	ntersectio	n LOS: A
Intersection Capacity Utiliz						of Service
Analysis Period (min) 15						01 001 1100

Splits and Phases: 103:

← Ø2 (R)	▲ Ø4	
22.5 s	22.5 s	
₩ 22.5 s		
22.5 s		

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	101 101	LDIV				
Traffic Volume (vph)	T ₽ 144	49	20	TT 40	- 1	r
Future Volume (vph)	144	49	20	40	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	0.95	1.00	0.95	1.00	1.00
Frt	0.962	0.75	1.00	0.75	1.00	1.00
Flt Protected	0.702		0.950			
Satd. Flow (prot)	3405	0	1770	3539	1863	1863
Flt Permitted	5-05	0	0.620	3337	1005	1005
Satd. Flow (perm)	3405	0	1155	3539	1863	1863
Right Turn on Red	3403	Yes	1155	3334	1003	Yes
Satd. Flow (RTOR)	53	103				162
Link Speed (mph)	30			30	30	
Link Speed (mph)	533			404	428	
• •						
Travel Time (s)	12.1	0.00	0.00	9.2	9.7	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	157	53	22	43	0	0
Shared Lane Traffic (%)	210	0	22	40	^	0
Lane Group Flow (vph)	210	0	22	43	0	0
Enter Blocked Intersection	No	No	No	No	No	No Diabt
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	24			24	20	
Link Offset(ft)	-12			8	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Turn Type	NA		Perm	NA	Prot	Perm
Protected Phases	4			8	2	
Permitted Phases			8			2
Minimum Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (%)	50.0%		50.0%	50.0%	50.0%	50.0%
Maximum Green (s)	18.0		18.0	18.0	18.0	18.0
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag						
Lead-Lag Optimize?						
Walk Time (s)	7.0		7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0		0	0	0	0
Act Effct Green (s)	18.0		18.0	18.0		
Actuated g/C Ratio	0.40		0.40	0.40		
v/c Ratio	0.15		0.05	0.03		
Control Delay	6.8		8.7	8.3		
Queue Delay	0.0		0.0	0.0		
Total Delay	6.8		8.7	8.3		
	0.0		0.7	0.5		

	-	\mathbf{r}	4	-	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
LOS	А		А	А		
Approach Delay	6.8			8.5		
Approach LOS	А			А		
Queue Length 50th (ft)	12		3	3		
Queue Length 95th (ft)	27		13	10		
Internal Link Dist (ft)	453			324	348	
Turn Bay Length (ft)						
Base Capacity (vph)	1393		462	1415		
Starvation Cap Reductn	0		0	0		
Spillback Cap Reductn	0		0	0		
Storage Cap Reductn	0		0	0		
Reduced v/c Ratio	0.15		0.05	0.03		
Intersection Summary						
Area Type:	Other					
Cycle Length: 45						
Actuated Cycle Length: 45						
Offset: 0 (0%), Referenced	to phase 2:1	VBL and 6	:, Start (of Green		
Natural Cycle: 45						
Control Type: Pretimed						
Maximum v/c Ratio: 0.15						
Intersection Signal Delay:					tersectior	
Intersection Capacity Utiliz	ation 17.2%			IC	CU Level o	of Service A
Analysis Period (min) 15						

Splits and Phases: 104:

• 1 √ø2 (R)	→ Ø4	
22.5 s	22.5 s	
	₹Ø8	
	22.5 s	

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	† †	1	<u> </u>	^	ኘዣ	
Traffic Volume (vph)	87	57	544	39	22	19
Future Volume (vph)	87	57	544	39	22	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	0.95
Frt	0.75	0.850	1.00	0.75	0.930	0.75
Flt Protected		0.000	0.950		0.974	
Satd. Flow (prot)	3539	1583	1770	3539	3273	0
Flt Permitted	0007	1303	0.581	0007	0.974	0
Satd. Flow (perm)	3539	1583	1082	3539	3273	0
Right Turn on Red	3337	Yes	1002	5557	5275	Yes
Satd. Flow (RTOR)		62			21	163
Link Speed (mph)	30	02		30	30	
Link Distance (ft)	491			971	1149	
Travel Time (s)	11.2	0.00	0.00	22.1	26.1	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	95	62	591	42	24	21
Shared Lane Traffic (%)		()				
Lane Group Flow (vph)	95	62	591	42	45	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	24			24	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Number of Detectors	2	1	1	2	1	
Detector Template	Thru	Right	Left	Thru	Left	
Leading Detector (ft)	100	20	20	100	20	
Trailing Detector (ft)	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	
Detector 1 Size(ft)	6	20	20	6	20	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)	94	0.0	0.0	94	0.0	
	94					
Detector 2 Size(ft)	o Cl+Ex			6 CI+Ex		
Detector 2 Type	UI+EX			UI+EX		
Detector 2 Channel	0.0			0.0		
Detector 2 Extend (s)	0.0	D		0.0	Durt	
Turn Type	NA	Perm	pm+pt	NA	Prot	
Protected Phases	6	,	5	2	4	
Permitted Phases		6	2			
Detector Phase	6	6	5	2	4	
Switch Phase	_	_		_	_	
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	20.0	20.0	9.5	22.5	15.0	
Total Split (s)	15.0	15.0	20.0	35.0	15.0	
Total Split (%)	30.0%	30.0%	40.0%	70.0%	30.0%	
Maximum Green (s)	10.5	10.5	15.5	30.5	10.5	
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max	None	C-Max	None	
Walk Time (s)	7.0	7.0		2		
Flash Dont Walk (s)	11.0	11.0				
Pedestrian Calls (#/hr)	0	0				
Act Effct Green (s)	26.1	26.1	41.1	43.8	6.0	
Actuated g/C Ratio	0.52	0.52	0.82	0.88	0.12	
v/c Ratio	0.05	0.02	0.57	0.00	0.12	
Control Delay	11.3	5.8	4.2	1.3	13.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	11.3	5.8	4.2	1.3	13.9	
LOS	B	A	A	A	B	
Approach Delay	9.1	, ,	,,	4.0	13.9	
Approach LOS	A			A	B	
Queue Length 50th (ft)	3	0	1	0	3	
Queue Length 95th (ft)	26	23	75	m3	14	
Internal Link Dist (ft)	411	20	, 0	891	1069	
Turn Bay Length (ft)				0/1	1007	
Base Capacity (vph)	1848	856	1112	3099	703	
Starvation Cap Reductn	0	0.00	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.05	0.07	0.53	0.01	0.06	
	0.03	0.07	0.00	0.01	0.00	
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50						
Offset: 0 (0%), Referenced	to phase 2	:WBTL ar	nd 6:EBT,	Start of (Green	
Natural Cycle: 60						
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.57						
Intersection Signal Delay: 5	5.5			Ir	ntersection	n LOS: A
Intersection Capacity Utilization	ation 48.5%)		[(CU Level	of Service A
Analysis Period (min) 15						
m Volume for 95th percei	ntile queue	is metere	d by upst	ream sigr	nal.	

Splits and Phases: 105:

✓ Ø2 (R)		1 Ø4	
35 s		15 s	
√ Ø5	🛡 🐨 🗷 Ø6 (R)		
20 s	15 s		

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		††			† †			•			•	
Traffic Volume (vph)	0	99	7	150	582	15	0	0	4	0	0	0
Future Volume (vph)	0	99	7	150	582	15	0	0	4	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.990			0.997			0.865				
Flt Protected					0.990							
Satd. Flow (prot)	0	3504	0	0	3493	0	0	1611	0	0	1863	0
Flt Permitted					0.990							
Satd. Flow (perm)	0	3504	0	0	3493	0	0	1611	0	0	1863	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		971			260			262			305	
Travel Time (s)		22.1			5.9			6.0			6.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	108	8	163	633	16	0	0	4	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	116	0	0	812	0	0	4	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		16			16			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
51	ther											
Control Type: Unsignalized												
Intersection Capacity Utilizatio	on 34.2%			IC	CU Level	of Service	A					

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>		VUL	 ↑₽		
Traffic Volume (vph)	TT 92	r 10	9	କ T 540	ר 207	-
Future Volume (vph)	92 92	10	9	540 540	207	4
	92 1900	1900	9 1900	1900		4
Ideal Flow (vphpl)					1900	
Lane Width (ft)	12	14	12	12	12	12
Lane Util. Factor	0.95	1.00	0.95	0.95	1.00	1.00
Frt Fit Drotostad		0.850		0.000	0.050	0.850
Flt Protected	0500	1/00	^	0.999	0.950	1500
Satd. Flow (prot)	3539	1689	0	3536	1770	1583
Flt Permitted				0.952	0.950	
Satd. Flow (perm)	3539	1689	0	3369	1770	1583
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		11				4
Link Speed (mph)	30			30	30	
Link Distance (ft)	777			738	307	
Travel Time (s)	17.7			16.8	7.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	100	11	10	587	225	4
Shared Lane Traffic (%)			10			
Lane Group Flow (vph)	100	11	0	597	225	4
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	Left	Right
Median Width(ft)	16	IN INA	LCII	16	36	Right
Link Offset(ft)	0			0	30 0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	4.00	0.00	1.00	1.00	1.00	1.00
Headway Factor	1.00	0.92	1.00	1.00	1.00	1.00
Turning Speed (mph)		15	15		15	9
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (ft)	100	20	20	100	20	20
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	6	20	20	6	20	20
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel	5 EA	<i>L</i> A	n	21.24	21.2/	n
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)	94	0.0	0.0	94	0.0	0.0
· · ·						
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	6		5	2	4	
Permitted Phases		6	2			4
Detector Phase	6	6	5	2	4	4
Switch Phase						

-	\mathbf{i}	4	-	1	1
EBT	EBR	WBL	WBT	NBL	NBR
5.0	5.0	5.0	5.0	5.0	5.0
22.5	22.5	9.5	22.5	22.5	22.5
20.0	20.0	8.5	28.5	21.5	21.5
40.0%	40.0%	17.0%	57.0%	43.0%	43.0%
15.5	15.5	4.0	24.0	17.0	17.0
3.5	3.5	3.5	3.5	3.5	3.5
1.0	1.0	1.0	1.0	1.0	1.0
0.0	0.0		0.0	0.0	0.0
4.5	4.5		4.5	4.5	4.5
Lag	Lag	Lead			
Yes	Yes	Yes			
3.0	3.0	3.0	3.0	3.0	3.0
C-Max	C-Max	None	C-Max	None	None
7.0	7.0		7.0	7.0	7.0
11.0	11.0		11.0	11.0	11.0
0	0		0	0	0
29.5	29.5		29.5	11.5	11.5
0.59	0.59		0.59	0.23	0.23
0.05	0.01		0.30	0.55	0.01
2.2	0.5		6.8	21.5	9.0
0.0	0.0		0.0	0.0	0.0
2.2	0.5		6.8	21.5	9.0
А	А		А	С	А
2.0			6.8	21.3	
А			А	С	
7	1		37	58	0
1	0		75	98	5
697			658	227	
2085	999		1984	601	540
0	0		0	0	0
0	0		0	0	0
0	0		0	0	0
0.05	0.01		0.30	0.37	0.01
Other					
)					
	:WBTL ar	nd 6:EBT.	Start of	Green	
	5.0 22.5 20.0 40.0% 15.5 3.5 1.0 0.0 4.5 Lag Yes 3.0 C-Max 7.0 11.0 0 29.5 0.59 0.05 2.2 0.0 2.2 0.0 2.2 0.0 2.2 0.0 2.2 0.0 2.2 0.0 2.2 0.0 2.2 0.0 2.2 0.0 2.2 0.0 5 0.59 0.05 2.2 0.0 5 2.2 0.0 5 0.05 2.2 0.0 5 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 2.2 0.0 0 0.05 0.05	5.0 5.0 22.5 22.5 20.0 20.0 40.0% 40.0% 15.5 15.5 3.5 3.5 1.0 1.0 0.0 0.0 4.5 4.5 Lag Lag Yes Yes 3.0 3.0 C-Max C-Max 7.0 7.0 11.0 11.0 0 0 29.5 29.5 0.59 0.59 0.59 0.59 0.00 0.0 2.2 0.5 A A 2.0 0.0 0.0 0.0 2.2 0.5 A A 2.0 1 1 0 697 0 2085 9999 0 0 0 0 0 0 0 0 0 0 <tr td=""> 0.01</tr>	5.0 5.0 5.0 22.5 22.5 9.5 20.0 20.0 8.5 40.0% 40.0% 17.0% 15.5 15.5 4.0 3.5 3.5 3.5 1.0 1.0 1.0 0.0 0.0 0.0 44.5 4.5 4.5 Lag Lag Lead Yes Yes Yes 3.0 3.0 3.0 C-Max C-Max None 7.0 7.0 11.0 11.0 11.0 0 0 0 0 29.5 29.5 0.59 0.59 0.59 0.01 2.2 0.5 A 0 0.0 0 2.2 0.5 A A 2.0 5 A A 2.0 2.0 5 A 7 1 1 1 0 0 697 0.01 0 <	5.0 5.0 5.0 5.0 22.5 22.5 9.5 22.5 20.0 20.0 8.5 28.5 40.0% 40.0% 17.0% 57.0% 15.5 15.5 4.0 24.0 3.5 3.5 3.5 3.5 1.0 1.0 1.0 1.0 0.0 0.0 0.0 0.0 44.5 4.5 4.5 4.5 Lag Lag Lead	5.0 5.0 5.0 5.0 5.0 22.5 22.5 22.5 22.5 22.5 20.0 20.0 8.5 28.5 21.5 40.0% 40.0% 17.0% 57.0% 43.0% 15.5 15.5 4.0 24.0 17.0 3.5 3.5 3.5 3.5 3.5 1.0 1.0 1.0 1.0 1.0 0.0 0.0 0.0 0.0 0.0 4.5 4.5 4.5 4.5 4.5 Lag Lag Lead

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.55

Intersection Signal Delay: 9.8 Intersection Capacity Utilization 37.5% Analysis Period (min) 15

Intersection LOS: A ICU Level of Service A

Splits and Phases: 107:

♥Ø2 (R)		▲ Ø4	
28.5 s		21.5 s	
√ Ø5	- → •Ø6 (R)		
8.5 s	20 s		

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	٦Y	LDIK		†	<u></u>	<u> </u>
Traffic Volume (vph)	72	24	317	999	357	232
Future Volume (vph)	72	24	317	999	357	232
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.97	0.95	1.00	0.95	0.95	1.00
Frt	0.962	0.70	1.00	5.70	5.75	0.850
Flt Protected	0.964		0.950			0.000
Satd. Flow (prot)	3351	0	1770	3539	3539	1583
Flt Permitted	0.964	0	0.405	0007	0007	1000
Satd. Flow (perm)	3351	0	754	3539	3539	1583
Right Turn on Red	0001	Yes	707	0007	0007	Yes
Satd. Flow (RTOR)	26	103				252
Link Speed (mph)	30			30	30	232
Link Distance (ft)	738			727	965	
Travel Time (s)	16.8			16.5	21.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	78	26	345	1086	388	252
Shared Lane Traffic (%)	70	20	545	1000	500	232
Lane Group Flow (vph)	104	0	345	1086	388	252
Enter Blocked Intersection	No	No	No	No	500 No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	26 26	Nynt	Leit	12	12	Nynt
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	10			10	10	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	1.00	1.00	1.00	1.00	1.00
Turn Type	Prot	9	pm+pt	NA	NA	Perm
Protected Phases	4		pin+pi 5	2	NA 6	Femi
Permitted Phases	4		2	Z	U	6
Minimum Split (s)	22.5		2 9.5	22.5	22.5	22.5
Total Split (s)	22.5 15.0		9.5 15.0	22.5 35.0	22.5	22.5
Total Split (%)	30.0%		30.0%	35.0 70.0%	40.0%	40.0%
				70.0%	40.0% 15.5	
Maximum Green (s)	10.5		10.5			15.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag			Lead		Lag	Lag
Lead-Lag Optimize?			Yes		Yes	Yes
Walk Time (s)	7.0			7.0	7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	10.5		30.5	30.5	15.5	15.5
Actuated g/C Ratio	0.21		0.61	0.61	0.31	0.31
v/c Ratio	0.14		0.51	0.50	0.35	0.38
Control Delay	8.3		7.7	6.5	14.5	4.2
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	8.3		7.7	6.5	14.5	4.2

Synchro 11 Report Page 15

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
LOS	А		А	А	В	А	
Approach Delay	8.3			6.8	10.5		
Approach LOS	А			А	В		
Queue Length 50th (ft)	12		40	77	45	0	
Queue Length 95th (ft)	26		74	113	74	39	
Internal Link Dist (ft)	658			647	885		
Turn Bay Length (ft)							
Base Capacity (vph)	724		673	2158	1097	664	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.14		0.51	0.50	0.35	0.38	
Intersection Summary							
Area Type:	Other						
Cycle Length: 50							
Actuated Cycle Length: 50							
Offset: 0 (0%), Referenced	to phase 2:1	VBTL and	6:SBT, 1	Start of G	reen, Mas	ster Inters	section
Natural Cycle: 60							
Control Type: Pretimed							
Maximum v/c Ratio: 0.51							
Intersection Signal Delay:				In	tersection	LOS: A	
Intersection Capacity Utiliz	ation 42.8%			IC	U Level c	of Service	А
Analysis Period (min) 15							
Splits and Phases: 108:							

√ Ø2 (R)		▶ _{Ø4}	
35 s		15 s	
▲ ø5	🛛 🗘 Ø6 (R)		
15 s	20 s		

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Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations	•			†		
Traffic Volume (vph)	211	0	0	19	0	0
Future Volume (vph)	211	0	0	19	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						
Flt Protected						
Satd. Flow (prot)	1863	0	0	1863	0	0
Flt Permitted						
Satd. Flow (perm)	1863	0	0	1863	0	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	1082			1015	590	
Travel Time (s)	24.6			23.1	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	229	0	0	21	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	229	0	0	21	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 14.4%			IC	U Level o	of Service A
Analysis Poriod (min) 15						

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	†			•	Y		
Traffic Volume (vph)	28	0	0	19	0	183	
Future Volume (vph)	28	0	0	19	0	183	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt					0.865		
Flt Protected							
Satd. Flow (prot)	1863	0	0	1863	1611	0	
Flt Permitted							
Satd. Flow (perm)	1863	0	0	1863	1611	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	839			634	538		
Travel Time (s)	19.1			14.4	12.2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	30	0	0	21	0	199	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	30	0	0	21	199	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	12		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utilization	tion 21.3%			IC	CU Level o	of Service	Α
Analysis Period (min) 15						2 2	

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ę	et		ľ	1
Traffic Volume (vph)	0	0	0	19	28	0
Future Volume (vph)	0	0	0	19	28	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.865			
Flt Protected					0.950	
Satd. Flow (prot)	0	1863	1611	0	1770	1863
Flt Permitted					0.950	
Satd. Flow (perm)	0	1863	1611	0	1770	1863
Link Speed (mph)		30	30		30	
Link Distance (ft)		98	839		286	
Travel Time (s)		2.2	19.1		6.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	21	30	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	21	0	30	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		36	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Stop		Stop	
Intersection Summary						
51	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 13.3%			IC	CU Level of	of Service

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	∱1 ≱			<u></u>		1	
Traffic Volume (vph)	214	20	0	359	70	35	
Future Volume (vph)	214	20	0	359	70	35	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	0.95	0.95	1.00	0.95	1.00	1.00	
Frt	0.987					0.865	
Flt Protected					0.950		
Satd. Flow (prot)	3493	0	0	3539	0	1611	
Flt Permitted					0.950		
Satd. Flow (perm)	3493	0	0	3539	0	1611	
Link Speed (mph)	30			30	30		
Link Distance (ft)	404			491	211		
Travel Time (s)	9.2			11.2	4.8		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	233	22	0	390	76	38	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	255	0	0	390	76	38	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Right	
Median Width(ft)	12			24	0		
Link Offset(ft)	0			6	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion Err%			IC	CU Level	of Service	Η
Analysis Period (min) 15							

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Lane Group	SBL	SBR	NEL	NET	SWT	SWR	
Lane Configurations		77	ካካ	•			
Traffic Volume (vph)	0	827	205	0	0	0	
Future Volume (vph)	0	827	205	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	0.88	0.97	1.00	1.00	1.00	
Frt		0.850					
Flt Protected			0.950				
Satd. Flow (prot)	0	2787	3433	1863	0	0	
Flt Permitted			0.950				
Satd. Flow (perm)	0	2787	3433	1863	0	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	227			1186	549		
Travel Time (s)	5.2			27.0	12.5		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	899	223	0	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	899	223	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	12			36	36		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15	9	15			9	
Sign Control	Free			Free	Free		
Intersection Summary							
51	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	ion 32.3%			IC	U Level o	of Service A	А
Apolycic Doriod (min) 15							

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Lane Group	EBL	EBR	NEL	NET	SWT	SWR
Lane Configurations				^	^	1
Traffic Volume (vph)	0	0	0	205	342	485
Future Volume (vph)	0	0	0	205	342	485
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	0.91	1.00
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	0	0	3539	5085	1583
Flt Permitted						
Satd. Flow (perm)	0	0	0	3539	5085	1583
Link Speed (mph)	30			30	30	
Link Distance (ft)	1042			233	359	
Travel Time (s)	23.7			5.3	8.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	223	372	527
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	223	372	527
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	8			0	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Free			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 33.4%			IC	CU Level	of Service A
Analysis Dariad (min) 15						

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Lane Group	SBL	SBR	NWL	NWR	NEL	NER
Lane Configurations	<u> </u>			10001	ሻሻ	<u>NLR</u>
Traffic Volume (vph)	- 1 0	6 6 827	0	.	205	.
Future Volume (vph)	0	827	0	0	205	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1900	0.88	1.00	1.00	0.97	1.00
	1.00	0.850	1.00	1.00	0.97	1.00
Frt Elt Protoctod		0.800			0.950	
Flt Protected	10/0	7707	10/0	10/0		10/0
Satd. Flow (prot)	1863	2787	1863	1863	3433	1863
Flt Permitted	10/0	2207	10/0	10/0	0.950	10/0
Satd. Flow (perm)	1863	2787	1863	1863	3433	1863
Right Turn on Red		Yes		Yes		Yes
Satd. Flow (RTOR)		1920				
Link Speed (mph)	30		30		30	
Link Distance (ft)	763		723		430	
Travel Time (s)	17.3		16.4		9.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	899	0	0	223	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	899	0	0	223	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	30	3	32		32	.9
Link Offset(ft)	30		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane	10		10		10	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	9	1.00	9	1.00	9
Number of Detectors	0	9	0	9	0	9
Detector Template						Thru
	Thru	Thru	Thru	Thru	Thru	
Leading Detector (ft)	0	0	0	0	0	0
Trailing Detector (ft)	0 Dret	0	0	0	0 Drat	0
Turn Type	Prot	pt+ov	Prot	Perm	Prot	Perm
Protected Phases	5	56	4		6	
Permitted Phases				4		6
Detector Phase	5	56	4	4	6	6
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (s)	15.5		15.5	15.5	19.0	19.0
Total Split (%)	31.0%		31.0%	31.0%	38.0%	38.0%
Maximum Green (s)	11.0		11.0	11.0	14.5	14.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
			4.0	4.0		
Lead/Lag	Lead				Lag	Lag
Lead-Lag Optimize?	Yes				Yes	Yes
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	None		None	None	C-Max	C-Max

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Lane Group SBL SBR NWL NWR NEL NER Walk Time (s) 7.0 7.0 7.0 7.0 7.0 7.0 Flash Dont Walk (s) 11.0 11.0 11.0 11.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 0 0 0 Act Effet Green (s) 50.0 34.7 Actuated g/C Ratio 0.32 0.09 Control Delay 0.7 2.7 Queue Delay 0.0 0.0 0		L.	Ł		*	•	~	
Flash Dont Walk (s) 11.0	Lane Group	SBL	SBR	NWL	NWR	NEL	NER	
Pedestrian Calls (#/hr) 0								
Act Effct Green (s) 50.0 34.7 Actuated g/C Ratio 1.00 0.69 \forall /c Ratio 0.32 0.09 Control Delay 0.7 2.7 Queue Delay 0.0 0.0 Total Delay 0.7 2.7 LOSAAApproach Delay 0.7 2.7 Approach Delay 0.7 2.7 Approach LOSAAQueue Length 50th (ft) 0 8 Queue Length 95th (ft) 1 16 Internal Link Dist (ft) 683 643 Base Capacity (vph) 2783 2382 Starvation Cap Reductn 0 0 Spillback Cap Reductn 0 0 Reduced v/c Ratio 0.32 0.09 Intersection Summary A A Area Type:Other $Cycle Length: 50$ Offset: 0 0 Offset: 0 0 Natural Cycle: 70 $Control Type: Actuated-CoordinatedMaximum v/c Ratio:0.321.1Intersection Signal Delay:1.1Intersection Capacity Utilization 32.7\%ICU Level of Service AAnalysis Period (min) 15Spilts and Phases:102:$								
Actuated g/C Ratio 1.00 0.69 v/c Ratio 0.32 0.09 Control Delay 0.7 2.7 Queue Delay 0.0 0.0 Total Delay 0.7 2.7 LOS A A Approach Delay 0.7 2.7 LOS A A Approach Delay 0.7 2.7 LOS A A Queue Length 50th (ft) 0 8 Queue Length 95th (ft) 1 16 Internal Link Dist (ft) 683 643 350 Turn Bay Length (ft) 1 16 1 Base Capacity (vph) 2783 2382 2382 Starvation Cap Reductn 0 0 0 Spillback Cap Reductn 0 0 0 Reduced v/c Ratio 0.32 0.09 0 Intersection Summary		0		0	0		0	
v/c Ratio 0.32 0.09 Control Delay 0.7 2.7 Queue Delay 0.0 0.0 Total Delay 0.7 2.7 LOS A A Approach Delay 0.7 2.7 LOS A A Approach Delay 0.7 2.7 LOS A A Queue Length 50th (ft) 0 8 Queue Length 95th (ft) 1 16 Internal Link Dist (ft) 683 643 350 Turn Bay Length (ft) 8 2382 3282 Starvation Cap Reductn 0 0 0 Spillback Cap Reductn 0 0 0 Reduced v/c Ratio 0.32 0.09 0 Intersection Summary								
Control Delay 0.7 2.7 Queue Delay 0.0 0.0 Total Delay 0.7 2.7 LOS A A Approach Delay 0.7 2.7 LOS A A Approach Delay 0.7 2.7 Approach LOS A A Approach LOS A A Queue Length Soth (ft) 0 8 Queue Length 95th (ft) 1 16 Interned Link Dist (ft) 683 643 350 Turn Bay Length (ft) Base Capacity (yph) 2783 2382 Starvation Cap Reductn 0 0 0 Staruet Cap Reductn 0 0 0								
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Total Delay 0.7 2.7 LOS A A Approach Delay 0.7 2.7 Approach LOS A A Queue Length 50th (ft) 0 8 Queue Length 95th (ft) 1 16 Internal Link Dist (ft) 683 643 350 Turn Bay Length (ft) 8 2382 3282 Starvation Cap Reductn 0 0 0 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.32 0.09 0 Intersection Summary Area Type: Other Otype: Actuated Cycle Length: 50 Offset: 0 (0%), Referenced to phase 6:NEL, Start of Green Natural Cycle: 70 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.32 Intersection LOS: A Intersection Signal Delay: 1.1 Intersection LOS: A Intersection Signal Delay: 1.1 Intersection LOS: A Analysis Period (min) 15 Splits and Phases: 102: 102: 102:								
LOSAAApproach Delay0.72.7Approach LOSAAQueue Length 50th (ft)08Queue Length 95th (ft)116Internal Link Dist (ft)683643Base Capacity (vph)27832382Starvation Cap Reductn00Spillback Cap Reductn00Storage Cap Reductn00Reduced v/c Ratio0.320.09Intersection SummaryArea Type:OtherCycle Length: 50Control Type: Actuated CoordinatedMaximum v/c Ratio:0.32Intersection LOS: AIntersection Signal Delay:1.1Intersection LOS: AIntersection Capacity Utilization 32.7%ICU Level of Service AAnalysis Period (min) 15Spilts and Phases:102:								
Approach Delay 0.7 2.7 Approach LOS A A Queue Length 50th (ft) 0 8 Queue Length 95th (ft) 1 16 Internal Link Dist (ft) 683 643 350 Turn Bay Length (ft) Base Capacity (vph) 2783 2382 Starvation Cap Reductn 0 0 0 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.32 0.09 0 Intersection Summary Area Type: Other Cycle Length: 50 Offset: 0 (0%), Referenced to phase 6:NEL, Start of Green Natural Cycle: 70 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.32 Intersection LOS: A Intersection LOS: A Intersection Capacity Utilization 32.7% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 102: 102: 103:								
Approach LOS A A Queue Length 50th (ft) 0 8 Queue Length 95th (ft) 1 16 Internal Link Dist (ft) 683 643 350 Turn Bay Length (ft) 8 2382 350 Base Capacity (vph) 2783 2382 Starvation Cap Reductn 0 0 Spillback Cap Reductn 0 0 Storage Cap Reductn 0 0 Reduced v/c Ratio 0.32 0.09 Intersection Summary 4rea Type: Other Cycle Length: 50 Other Cycle Length: 50 Offset: 0 (0%), Referenced to phase 6:NEL, Start of Green Natural Cycle: 70 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.32 Intersection Signal Delay: 1.1 Intersection LOS: A Intersection Capacity Utilization 32.7% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 102:			А					
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Oueue Length 95th (ft) 1 16 Internal Link Dist (ft) 683 643 350 Turn Bay Length (ft) Base Capacity (vph) 2783 2382 Starvation Cap Reductn 0 0 0 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.32 0.09 0 Intersection Summary		А						
Internal Link Dist (ft)683643350Turn Bay Length (ft)Base Capacity (vph)27832382Starvation Cap Reductn00Spillback Cap Reductn00Storage Cap Reductn00Reduced v/c Ratio0.320.09Intersection SummaryArea Type:OtherCycle Length: 50Actuated Cycle Length: 50Actuated Cycle Length: 50Offset: 0 (0%), Referenced to phase 6:NEL, Start of GreenNatural Cycle: 70Control Type: Actuated-CoordinatedMaximum v/c Ratio: 0.321.1Intersection Signal Delay: 1.1Intersection LOS: AIntersection Capacity Utilization 32.7%ICU Level of Service AAnalysis Period (min) 15Splits and Phases: 102:			0					
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Spillback Cap Reductn00Storage Cap Reductn00Reduced v/c Ratio0.320.09Intersection SummaryArea Type:OtherCycle Length: 500Actuated Cycle Length: 500Offset: 0 (0%), Referenced to phase 6:NEL, Start of GreenNatural Cycle: 70Control Type: Actuated-CoordinatedMaximum v/c Ratio: 0.32Intersection Signal Delay: 1.1Intersection LOS: AIntersection Capacity Utilization 32.7%ICU Level of Service AAnalysis Period (min) 15Splits and Phases:102:			2783			2382		
Storage Cap Reductn00Reduced v/c Ratio0.320.09Intersection SummaryArea Type:OtherCycle Length: 500Actuated Cycle Length: 500Offset: 0 (0%), Referenced to phase 6:NEL, Start of GreenNatural Cycle: 70Control Type: Actuated-CoordinatedMaximum v/c Ratio: 0.32Intersection Signal Delay: 1.1Intersection LOS: AIntersection Capacity Utilization 32.7%ICU Level of Service AAnalysis Period (min) 15Splits and Phases:102:			-					
Reduced v/c Ratio 0.32 0.09 Intersection Summary								
Intersection Summary Area Type: Other Cycle Length: 50 Offset: 50 Actuated Cycle Length: 50 Offset: 0 (0%), Referenced to phase 6:NEL, Start of Green Natural Cycle: 70 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.32 Intersection LOS: A Intersection Signal Delay: 1.1 Intersection LOS: A Intersection Capacity Utilization 32.7% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 102:								
Area Type: Other Cycle Length: 50 Offset: 50 Actuated Cycle Length: 50 Offset: 0 (0%), Referenced to phase 6:NEL, Start of Green Natural Cycle: 70 Ocontrol Type: Actuated-Coordinated Maximum v/c Ratio: 0.32 Intersection LOS: A Intersection Capacity Utilization 32.7% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 102:	Reduced v/c Ratio		0.32			0.09		
Cycle Length: 50 Actuated Cycle Length: 50 Offset: 0 (0%), Referenced to phase 6:NEL, Start of Green Natural Cycle: 70 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.32 Intersection Signal Delay: 1.1 Intersection Capacity Utilization 32.7% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 102:	Intersection Summary							
Actuated Cycle Length: 50 Offset: 0 (0%), Referenced to phase 6:NEL, Start of Green Natural Cycle: 70 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.32 Intersection Signal Delay: 1.1 Intersection Capacity Utilization 32.7% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 102:		Other						
Offset: 0 (0%), Referenced to phase 6:NEL, Start of Green Natural Cycle: 70 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.32 Intersection Signal Delay: 1.1 Intersection Capacity Utilization 32.7% Intersection Capacity Utilization 32.7% ICU Level of Service A Analysis Period (min) 15 Splits and Phases:								
Natural Cycle: 70 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.32 Intersection Signal Delay: 1.1 Intersection Capacity Utilization 32.7% Intersection Capacity Utilization 32.7% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 102:								
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Maximum v/c Ratio: 0.32 Intersection Signal Delay: 1.1 Intersection Capacity Utilization 32.7% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 102:								
Intersection Signal Delay: 1.1 Intersection LOS: A Intersection Capacity Utilization 32.7% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 102:		ordinated						
Intersection Capacity Utilization 32.7% ICU Level of Service A Analysis Period (min) 15 Splits and Phases: 102:								
Analysis Period (min) 15 Splits and Phases: 102:								
Splits and Phases: 102:		ation 32.7%			IC	U Level o	f Service A	
	Analysis Period (min) 15							

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15.5 s	19 s	15.5 s

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	101	LDI		^	ኘ	
Traffic Volume (vph)	205	0	0	TT 429	399	29
Future Volume (vph)	205	0	0	429	399	29
Ideal Flow (vphpl)	1900	1900	1900	429	1900	1900
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	1.00
Frt	0.95	1.00	1.00	0.95	0.97	0.850
Fit Protected					0.950	0.650
Satd. Flow (prot)	3539	0	0	3539	3433	1583
Flt Permitted	2028	0	0	2028	0.950	1000
	2520	0	0	2520		100
Satd. Flow (perm)	3539	0	0	3539	3433	1583
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						32
Link Speed (mph)	30			30	30	
Link Distance (ft)	923			533	500	
Travel Time (s)	21.0			12.1	11.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	223	0	0	466	434	32
Shared Lane Traffic (%)						
Lane Group Flow (vph)	223	0	0	466	434	32
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12	. ugu	2011	12	24	. ugi u
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	10			10	10	
	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	1.00		1.00	1.00		
Turning Speed (mph)	2	9	15	2	15	9
Number of Detectors	2			2	1	1
Detector Template	Thru			Thru	Left	Right
Leading Detector (ft)	100			100	20	20
Trailing Detector (ft)	0			0	0	0
Detector 1 Position(ft)	0			0	0	0
Detector 1 Size(ft)	6			6	20	20
Detector 1 Type	CI+Ex			CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0			0.0	0.0	0.0
Detector 1 Queue (s)	0.0			0.0	0.0	0.0
Detector 1 Delay (s)	0.0			0.0	0.0	0.0
Detector 2 Position(ft)	94			94	0.0	0.0
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel	CITLX			CITLX		
Detector 2 Extend (s)	0.0			0.0		
					Drot	Dorm
Turn Type	NA			NA	Prot	Perm
Protected Phases	6			2	4	
Permitted Phases				_		4
Detector Phase	6			2	4	4
Switch Phase						
Minimum Initial (s)	5.0			5.0	5.0	5.0

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	22.5	231		22.5	22.5	22.5
Total Split (s)	26.0			26.0	24.0	24.0
Total Split (%)	52.0%			52.0%	48.0%	48.0%
Maximum Green (s)	21.5			21.5	19.5	19.5
Yellow Time (s)	3.5			3.5	3.5	3.5
All-Red Time (s)	1.0			1.0	1.0	1.0
Lost Time Adjust (s)	0.0			0.0	0.0	0.0
Total Lost Time (s)	4.5			4.5	4.5	4.5
Lead/Lag	4.0			4.5	4.5	4.0
Lead-Lag Optimize?						
	2.0			2.0	2.0	2.0
Vehicle Extension (s)	3.0			3.0	3.0	3.0
Recall Mode	C-Max			C-Max	None	None
Walk Time (s)	7.0			7.0	7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	29.3			29.3	11.7	11.7
Actuated g/C Ratio	0.59			0.59	0.23	0.23
v/c Ratio	0.11			0.22	0.54	0.08
Control Delay	4.4			5.8	18.9	6.3
Queue Delay	0.0			0.0	0.0	0.0
Total Delay	4.4			5.8	18.9	6.3
LOS	А			А	В	А
Approach Delay	4.4			5.8	18.0	
Approach LOS	А			А	В	
Queue Length 50th (ft)	11			28	57	0
Queue Length 95th (ft)	19			58	81	14
Internal Link Dist (ft)	843			453	420	
Turn Bay Length (ft)	010			100	120	
Base Capacity (vph)	2072			2072	1338	636
Starvation Cap Reductn	0			0	0	0.00
Spillback Cap Reductn	0			0	0	0
Storage Cap Reductn	0			0	0	0
Reduced v/c Ratio						
KEUULEU VIL KALIO	0.11			0.22	0.32	0.05
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50	1					
Offset: 0 (0%), Referenced	d to phase 2:\	NBT and	6:EBT, 3	Start of G	reen	
Natural Cycle: 45						
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.54						
Intersection Signal Delay:	10.5			lr	ntersectio	n LOS: B
Intersection Capacity Utiliz						of Service
Analysis Period (min) 15						

Splits and Phases: 103:

← Ø2 (R)	↑ _{Ø4}
26 s	24 s
, →Ø6 (R)	
26 s	

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	† ‡		<u> </u>	<u></u>	<u> </u>	1
Traffic Volume (vph)	234	0	0	429	0	0
Future Volume (vph)	234	0	0	429	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	0.95	1.00	0.95	1.00	1.00
Frt	0.70	0.70	1.00	0.70	1.00	1.00
Flt Protected						
Satd. Flow (prot)	3539	0	1863	3539	1863	1863
Flt Permitted	5557	0	1005	5557	1005	1005
Satd. Flow (perm)	3539	0	1863	3539	1863	1863
Right Turn on Red	JJJ7	Yes	1005	3337	1005	Yes
Satd. Flow (RTOR)		103				103
Link Speed (mph)	30			30	30	
Link Distance (ft)	533			404	428	
Travel Time (s)	12.1	0.00	0.00	9.2	9.7	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	254	0	0	466	0	0
Shared Lane Traffic (%)	a= :	_				
Lane Group Flow (vph)	254	0	0	466	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	24			24	20	
Link Offset(ft)	-12			8	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Turn Type	NA		Perm	NA	Prot	Perm
Protected Phases	4			8	2	
Permitted Phases			8	-		2
Minimum Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (%)	50.0%		50.0%	50.0%	50.0%	50.0%
Maximum Green (s)	18.0		18.0	18.0	18.0	18.0
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag						
Lead-Lag Optimize?						
Walk Time (s)	7.0		7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0		0	0	0	0
Act Effct Green (s)	18.0			18.0		
Actuated g/C Ratio	0.40			0.40		
v/c Ratio	0.18			0.33		
Control Delay	9.2			10.2		
Queue Delay	0.0			0.0		
Total Delay	9.2			10.2		
	7.2			10.2		

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
LOS	А			В		
Approach Delay	9.2			10.2		
Approach LOS	А			В		
Queue Length 50th (ft)	21			41		
Queue Length 95th (ft)	38			67		
Internal Link Dist (ft)	453			324	348	
Turn Bay Length (ft)						
Base Capacity (vph)	1415			1415		
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.18			0.33		
Intersection Summary						
Area Type:	Other					
Cycle Length: 45						
Actuated Cycle Length: 45						
Offset: 0 (0%), Referenced	d to phase 2:N	IBL and	6:, Start o	of Green		
Natural Cycle: 45						
Control Type: Pretimed						
Maximum v/c Ratio: 0.33						
Intersection Signal Delay:					tersection	
Intersection Capacity Utiliz	zation 15.6%			IC	U Level o	of Service A
Analysis Period (min) 15						

Splits and Phases: 104:

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22.5 s	22.5 s	
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	♥ Ø8	
	22.5 s	

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>	1	<u> </u>	^	٦Y	
Traffic Volume (vph)	237	12	57	92	267	670
Future Volume (vph)	237	12	57	92	267	670
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	0.95
Frt	0.75	0.850	1.00	0.75	0.893	0.75
Flt Protected		0.000	0.950		0.986	
Satd. Flow (prot)	3539	1583	1770	3539	3182	0
Flt Permitted	0007	1303	0.950	3337	0.986	0
Satd. Flow (perm)	3539	1583	1770	3539	3182	0
Right Turn on Red	3337	Yes	1770	5557	5102	Yes
Satd. Flow (RTOR)		13			571	162
Link Speed (mph)	30	13		30	30	
	30 491			30 971	30 1149	
Link Distance (ft)						
Travel Time (s)	11.2	0.00	0.00	22.1	26.1	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	258	13	62	100	290	728
Shared Lane Traffic (%)					46.10	
Lane Group Flow (vph)	258	13	62	100	1018	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	24			24	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Number of Detectors	2	1	1	2	1	
Detector Template	Thru	Right	Left	Thru	Left	
Leading Detector (ft)	100	20	20	100	20	
Trailing Detector (ft)	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	
Detector 1 Size(ft)	6	20	20	6	20	
Detector 1 Type	CI+Ex				CI+Ex	
Detector 1 Channel		0. · LA		01. ZA		
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)	94	0.0	0.0	94	0.0	
	94			94		
Detector 2 Size(ft)						
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel	0.0			0.0		
Detector 2 Extend (s)	0.0	Demo	Durt	0.0	Dert	
Turn Type	NA	Perm	Prot	NA	Prot	
Protected Phases	6		5	2	4	
Permitted Phases		6			_	
Detector Phase	6	6	5	2	4	
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	

04 Alt 01 PM Existing Adjusted adjusted; assume 60% reporting 2:43 pm 04/21/2021 1

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	20.0	20.0	9.5	22.5	15.0	
Total Split (s)	20.0	20.0	10.0	31.0	19.0	
Total Split (%)	42.0%	42.0%	20.0%	62.0%	38.0%	
Maximum Green (s)	16.5	16.5	5.5	26.5	14.5	
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	
Lead/Lag	Lag	Lag	Lead	1.0	1.0	
Lead-Lag Optimize?	Luy	Lug	Loud			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max	None	C-Max	None	
Walk Time (s)	7.0	7.0	NOTIC		NUTIC	
Flash Dont Walk (s)	11.0	11.0				
Pedestrian Calls (#/hr)	0	0				
Act Effct Green (s)	22.4	22.4	5.6	28.4	12.6	
Actuated g/C Ratio	0.45	0.45	0.11	0.57	0.25	
v/c Ratio	0.45	0.43	0.11	0.07	0.23 0.88dr	
Control Delay	10.9	6.8	27.9	4.4	14.2	
Queue Delay	0.0	0.0	0.0	4.4 0.0	0.0	
Total Delay	10.9	6.8	27.9	4.4	14.2	
LOS	10.9 B	0.0 A	27.9 C	4.4 A	14.Z B	
Approach Delay	ы 10.7	A	C	13.4	р 14.2	
Approach LOS	10.7 B			13.4 B	14.2 B	
Queue Length 50th (ft)	Б 27	0	18	Б 5	55	
Queue Length 95th (ft)	50	9	50	с 11	55 113	
Internal Link Dist (ft)	411	9	50	891	1069	
	411			071	1009	
Turn Bay Length (ft)	1586	717	199	2011	1328	
Base Capacity (vph)						
Starvation Cap Reductn	0	0	0	0	0 0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0 16	-	0 21	0.05		
Reduced v/c Ratio	0.16	0.02	0.31	0.05	0.77	
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50						
Offset: 0 (0%), Referenced	to phase 2	:WBT and	6:EBT, S	Start of G	reen	
Natural Cycle: 50						
Control Type: Actuated-Coc	ordinated					
Maximum v/c Ratio: 0.83						
Intersection Signal Delay: 1	3.5			Ir	ntersectior	LOS: B
Intersection Capacity Utiliza)				of Service A
Analysis Period (min) 15						2
dr Defacto Right Lane. R	ecode with	1 though	lane as a	a right lan	le.	
				<u>.</u>		

Splits and Phases: 105:

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31 s		19 s	
√ Ø5	- → = Ø6 (R)		
10 s	21s		

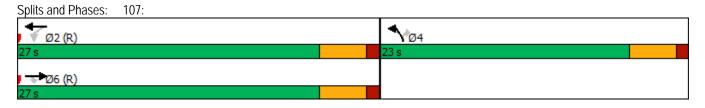
	≯	-	\mathbf{F}	•	+	*	•	1	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<u></u>			<u></u>			•			•	
Traffic Volume (vph)	0	907	0	2	119	2	27	0	17	0	0	4
Future Volume (vph)	0	907	0	2	119	2	27	0	17	0	0	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.998			0.948			0.865	
Flt Protected					0.999			0.970				
Satd. Flow (prot)	0	3539	0	0	3529	0	0	1713	0	0	1611	0
Flt Permitted					0.999			0.970				
Satd. Flow (perm)	0	3539	0	0	3529	0	0	1713	0	0	1611	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		971			260			262			305	
Travel Time (s)		22.1			5.9			6.0			6.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	986	0	2	129	2	29	0	18	0	0	4
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	986	0	0	133	0	0	47	0	0	4	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		16			16			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
J 1)ther											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 40.9%			IC	CU Level of	of Service	A					

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>			₩B1		
Traffic Volume (vph)	TT 567	357	4	H T 122	0	19
Future Volume (vph)	567	357	4	122	0	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	1.00	0.95	0.95	1.00	1.00
	0.90		0.90	0.90	1.00	
Frt Fit Drotoctod		0.850		0.999		0.850
Flt Protected	3539	1689	0	3536	1863	1583
Satd. Flow (prot)	3539	1089	0		1803	1583
Flt Permitted	2520	1/00	0	0.945	10/0	1500
Satd. Flow (perm)	3539	1689	0	3345	1863	1583
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	~~~	388				181
Link Speed (mph)	30			30	30	
Link Distance (ft)	777			738	307	
Travel Time (s)	17.7			16.8	7.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	616	388	4	133	0	21
Shared Lane Traffic (%)						
Lane Group Flow (vph)	616	388	0	137	0	21
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	Left	Right
Median Width(ft)	16			16	36	Ŭ
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	. 3					
Headway Factor	1.00	0.92	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	15	15		15	9
Number of Detectors	2	13	1	2	13	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (ft)	100	20	20	100	20	20
Trailing Detector (ft)	0	20	20	0	20	20
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	6 CL Ex	20 CL Ex	20 CL Ex	6	20 CL Ex	20 CL Ex
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel				0.6		
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	6			2	4	
Permitted Phases	v	6	2	-		4
Detector Phase	6	6	2	2	4	4
Switch Phase	U	0	2	2	4	4

04 Alt 01 PM Existing Adjusted adjusted; assume 60% reporting 2:43 pm 04/21/2021 1

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	27.0	27.0	27.0	27.0	23.0	23.0
Total Split (%)	54.0%	54.0%	54.0%	54.0%	46.0%	46.0%
Maximum Green (s)	22.5	22.5	22.5	22.5	18.5	18.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	1.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5
Lead/Lag	4.0	4.0		4.0	4.0	4.0
0						
Lead-Lag Optimize?	2.0	2.0	2.0	2.0	2.0	2.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0
Act Effct Green (s)	47.1	47.1		47.1		5.5
Actuated g/C Ratio	0.94	0.94		0.94		0.11
v/c Ratio	0.18	0.24		0.04		0.06
Control Delay	1.2	0.9		0.8		0.4
Queue Delay	0.0	0.0		0.0		0.0
Total Delay	1.2	0.9		0.8		0.4
LOS	А	А		А		А
Approach Delay	1.1			0.8	0.4	
Approach LOS	A			A	A	
Queue Length 50th (ft)	0	0		0		0
Queue Length 95th (ft)	m49	m15		9		0
Internal Link Dist (ft)	697	mij		658	227	U
Turn Bay Length (ft)	077			000	221	
Base Capacity (vph)	3334	1613		3151		699
1 3 1 1 2	3334 0			3151		099
Starvation Cap Reductn		0				
Spillback Cap Reductn	0	0		0		0
Storage Cap Reductn	0	0		0		0
Reduced v/c Ratio	0.18	0.24		0.04		0.03
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50						
Offset: 0 (0%), Referenced to phase 2:WBTL and 6:EBT, Start of Green						
Natural Cycle: 45						
Control Type: Actuated-Coordinated						
Maximum v/c Ratio: 0.24						
Intersection Signal Delay: 1.1 Intersection LOS: A						
					CU Level	
Analysis Period (min) 15						
m Volume for 95th percentile queue is metered by upstream signal.						

Lanes, Volumes, Timings 107:



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	<u>۲</u> ۲	LDI				
Traffic Volume (vph)	111 470	115	1 40	TT 1170	TT 922	r 85
Future Volume (vph)	470	115	40	1170	922	85
Ideal Flow (vphpl)	1900	1900	1900	1900	922 1900	1900
Lane Util. Factor	0.97	0.95	1.00	0.95	0.95	1.00
Frt	0.97	0.90	1.00	0.90	0.90	0.850
FIt Protected	0.971		0.950			0.000
	3372	0	0.950	3539	3539	1583
Satd. Flow (prot)		U	0.200	3039	3039	1083
Flt Permitted	0.961 3372	0		2520	2520	1500
Satd. Flow (perm)	3372		373	3539	3539	1583 Voc
Right Turn on Red	(7	Yes				Yes
Satd. Flow (RTOR)	67			20	20	92
Link Speed (mph)	30			30	30	
Link Distance (ft)	738			727	965	
Travel Time (s)	16.8			16.5	21.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	511	125	43	1272	1002	92
Shared Lane Traffic (%)						
Lane Group Flow (vph)	636	0	43	1272	1002	92
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	36			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Turn Type	Prot		pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	1 01111
Permitted Phases			2	2	0	6
Minimum Split (s)	22.5		9.5	22.5	22.5	22.5
Total Split (s)	22.5		8.5	28.5	22.5	22.5
	43.0%		17.0%	57.0%	40.0%	40.0%
Total Split (%)						
Maximum Green (s)	17.0		4.0	24.0	15.5	15.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag			Lead		Lag	Lag
Lead-Lag Optimize?			Yes		Yes	Yes
Walk Time (s)	7.0			7.0	7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	17.0		24.0	24.0	15.5	15.5
Actuated g/C Ratio	0.34		0.48	0.48	0.31	0.31
v/c Ratio	0.53		0.15	0.75	0.91	0.17
Control Delay	14.0		8.2	14.0	32.0	4.6
Queue Delay	0.0		0.2	0.0	0.0	0.0
Total Delay	14.0		8.2	14.0	32.0	4.6
	14.0		ŏ.Z	14.0	32.0	4.0

04 Alt 01 PM Existing Adjusted adjusted; assume 60% reporting 2:43 pm 04/21/2021 1

Synchro 11 Report Page 16

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
LOS	В		А	В	С	А	
Approach Delay	14.0			13.8	29.7		
Approach LOS	В			В	С		
Queue Length 50th (ft)	69		6	145	145	0	
Queue Length 95th (ft)	116		18	212	#253	24	
Internal Link Dist (ft)	658			647	885		
Turn Bay Length (ft)							
Base Capacity (vph)	1190		290	1698	1097	554	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.53		0.15	0.75	0.91	0.17	
Intersection Summary							
Area Type:	Other						
Cycle Length: 50							
Actuated Cycle Length: 50							
Offset: 0 (0%), Reference	d to phase 2:I	VBTL and	6:SBT,	Start of G	ireen, Mas	ster Inters	section
Natural Cycle: 60							
Control Type: Pretimed							
Maximum v/c Ratio: 0.91							
Intersection Signal Delay:					tersectior		
Intersection Capacity Utiliz	zation 57.8%			IC	CU Level o	of Service	B
Analysis Period (min) 15							
# 95th percentile volume			eue may	be longer	r		
Queue shown is maxim	num after two	cycles.					
Splits and Phases: 108:	:						

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28.5 s		21.5 s	
▲ Ø5	🛡 🕈 Ø6 (R)		
8.5 s	20 s		

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Lane Group	NBT	NBR	SBL	SBT	NWL	NWR	
Lane Configurations	†			†			
Traffic Volume (vph)	19	0	361	0	0	0	
Future Volume (vph)	19	0	361	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected				0.950			
Satd. Flow (prot)	1863	0	0	1770	0	0	
Flt Permitted				0.950			
Satd. Flow (perm)	1863	0	0	1770	0	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	1082			1015	590		
Travel Time (s)	24.6			23.1	13.4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	21	0	392	0	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	21	0	0	392	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	0		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion 30.0%			IC	U Level o	of Service	A
Analysis Doriod (min) 15							

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	•			•	- Y	
Traffic Volume (vph)	0	0	0	0	0	19
Future Volume (vph)	0	0	0	0	0	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.865	
Flt Protected						
Satd. Flow (prot)	1863	0	0	1863	1611	0
Flt Permitted						
Satd. Flow (perm)	1863	0	0	1863	1611	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	839			634	538	
Travel Time (s)	19.1			14.4	12.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	0	0	21
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	21	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization	tion 13.3%			IC	U Level o	of Service
Analysis Period (min) 15						

≯	→	+	•	1	4
EBL	EBT	WBT	WBR	SBL	SBR
	ર્સ	f,		ኘ	1
0	0	0	0	0	0
0	0	0	0	0	0
1900	1900	1900	1900	1900	1900
1.00	1.00	1.00	1.00	1.00	1.00
0	1863	1863	0	1863	1863
0	1863	1863	0	1863	1863
	30	30		30	
	98	839		286	
	2.2	19.1		6.5	
0.92	0.92	0.92	0.92	0.92	0.92
0	0	0	0	0	0
0	0	0	0	0	0
No	No	No	No	No	No
Left	Left	Left	Right	Left	Right
	0	0	Ū	36	Ū.
	0	0		0	
	16	16		16	
1.00	1.00	1.00	1.00	1.00	1.00
15			9	15	9
	Free	Stop		Stop	
ther					
on 13.3%			IC	U Level o	of Service A
	0 0 1900 1.00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 1900 1900 1900 1.00 1.00 0 1863 0 1863 0 1863 0 1863 0 1863 0 1863 0 1863 0 0 0 0.92 0 0 0 0 0 0 0 0 0 0 0 0 1.00 1.00 15 Free	Image: style	Image: constraint of the state intermediate inttermediate intermediate intermediate intermediate intermediate in	0 1900 1900

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	∱ î,			<u></u>		1
Traffic Volume (vph)	144	0	0	60	0	0
Future Volume (vph)	404	0	0	60	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	0.95	1.00	0.95	1.00	1.00
Frt						
Flt Protected						
Satd. Flow (prot)	3539	0	0	3539	0	1863
Flt Permitted						
Satd. Flow (perm)	3539	0	0	3539	0	1863
Link Speed (mph)	30			30	30	
Link Distance (ft)	404			491	211	
Travel Time (s)	9.2			11.2	4.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	439	0	0	65	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	439	0	0	65	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	12			24	0	
Link Offset(ft)	0			6	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 7.3%			IC	U Level o	of Service
Analysis Period (min) 15						

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Lane Group	SBL	SBR	NEL	NET	SWT	SWR		
Lane Configurations		77	ሻሻ	•				
Traffic Volume (vph)	0	40	192	592	0	0		
Future Volume (vph)	0	40	452	592	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Util. Factor	1.00	0.88	0.97	1.00	1.00	1.00		
Frt		0.850						
Flt Protected			0.950					
Satd. Flow (prot)	0	2787	3433	1863	0	0		
Flt Permitted			0.950					
Satd. Flow (perm)	0	2787	3433	1863	0	0		
Link Speed (mph)	30			30	30			
Link Distance (ft)	227			1186	549			
Travel Time (s)	5.2			27.0	12.5			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0	43	491	643	0	0		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	43	491	643	0	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Left	Left	Right		
Median Width(ft)	12			36	36			
Link Offset(ft)	0			0	0			
Crosswalk Width(ft)	16			16	16			
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15	9	15			9		
Sign Control	Free			Free	Free			
Intersection Summary								
Area Type:	Other							
Control Type: Unsignalized								
Intersection Capacity Utilization	tion 34.5%			IC	U Level o	of Service A	А	
Analysis Doriod (min) 15								

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Lane Group	EBL	EBR	NEL	NET	SWT	SWR
Lane Configurations				^	^	1
Traffic Volume (vph)	0	0	0	470	14	10
Future Volume (vph)	0	0	0	730	14	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	0.91	1.00
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	0	0	3539	5085	1583
Flt Permitted						
Satd. Flow (perm)	0	0	0	3539	5085	1583
Link Speed (mph)	30			30	30	
Link Distance (ft)	1042			233	359	
Travel Time (s)	23.7			5.3	8.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	793	15	11
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	793	15	11
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	8			0	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Free			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 16.3%			IC	U Level	of Service
Analysis Dariad (min) 15						

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Lane Group	SBL	SBR	NWL	NWR	NEL	NER
Lane Configurations	<u> </u>	<u>, 11</u>		10001	ሻሻ	
Traffic Volume (vph)	1 0	6 1 1 40	1 0	•	71 784	r 0
Future Volume (vph)	0	40	0	0	1044	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.88	1.00	1.00	0.97	1.00
Frt Flt Destastad		0.850			0.050	
Flt Protected	10/0	0707	10/0	10/0	0.950	10/0
Satd. Flow (prot)	1863	2787	1863	1863	3433	1863
Flt Permitted	1010				0.950	
Satd. Flow (perm)	1863	2787	1863	1863	3433	1863
Right Turn on Red		Yes		Yes		Yes
Satd. Flow (RTOR)		1920				
Link Speed (mph)	30		30		30	
Link Distance (ft)	763		723		430	
Travel Time (s)	17.3		16.4		9.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	43	0	0	1135	0
Shared Lane Traffic (%)	-					
Lane Group Flow (vph)	0	43	0	0	1135	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	30	i digitt	32	i tigrit	32	itigitt
Link Offset(ft)	30		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane	10		10		10	
	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor						
Turning Speed (mph)	15	9	15	9	15	9
Number of Detectors	0	0	0	0	0	0
Detector Template	Thru	Thru	Thru	Thru	Thru	Thru
Leading Detector (ft)	0	0	0	0	0	0
Trailing Detector (ft)	0	0	0	0	0	0
Turn Type	Prot	pt+ov	Prot	Perm	Prot	Perm
Protected Phases	5	56	4		6	
Permitted Phases				4		6
Detector Phase	5	56	4	4	6	6
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (s)	15.5		15.5	15.5	19.0	19.0
Total Split (%)	31.0%		31.0%	31.0%	38.0%	38.0%
Maximum Green (s)	11.0		11.0	11.0	14.5	14.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	3.5 1.0		3.5 1.0	3.5 1.0	3.5 1.0	3.5 1.0
.,						
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag	Lead				Lag	Lag
Lead-Lag Optimize?	Yes				Yes	Yes
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	None		None	None	C-Max	C-Max

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Lane Group	SBL	SBR	NWL	NWR	NEL	NER	
Walk Time (s)	7.0		7.0	7.0	7.0	7.0	
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	11.0	
Pedestrian Calls (#/hr)	0		0	0	0	0	
Act Effct Green (s)		50.0			44.2		
Actuated g/C Ratio		1.00			0.88		
v/c Ratio		0.02			0.37		
Control Delay		0.0			2.1		
Queue Delay		0.0			0.0		
Total Delay		0.0			2.1		
LOS		А			А		
Approach Delay					2.1		
Approach LOS					А		
Queue Length 50th (ft)		0			0		
Queue Length 95th (ft)		0			76		
Internal Link Dist (ft)	683		643		350		
Turn Bay Length (ft)							
Base Capacity (vph)		2787			3035		
Starvation Cap Reductn		0			0		
Spillback Cap Reductn		0			0		
Storage Cap Reductn		0			0		
Reduced v/c Ratio		0.02			0.37		
Intersection Summary							
21	Other						
Cycle Length: 50							
Actuated Cycle Length: 50							
Offset: 0 (0%), Referenced	to phase 6:1	VEL, Star	t of Gree	n			
Natural Cycle: 75							
Control Type: Actuated-Coc	ordinated						
Maximum v/c Ratio: 0.37							
Intersection Signal Delay: 2	.0			In	tersection	LOS: A	
Intersection Capacity Utiliza	ation 26.1%			IC	U Level c	f Service /	4
Analysis Period (min) 15							
Splits and Phases: 102:							
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A 05	• 😾 Ø6 (R)	▶ [≜] Ø4	
15.5 s	19 s	15.5 s	

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>+</u>	LDI	VVDL	•••••	אטב ז'ז	
Traffic Volume (vph)	TT 115	0	0	TT 40	0	0
Future Volume (vph)	375	0	0	40	0	0
Ideal Flow (vphpl)	375 1900	1900	1900	40	1900	1900
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	1.00
Frt	0.95	1.00	1.00	0.95	0.97	1.00
Fit Protected						
Satd. Flow (prot)	3539	0	0	3539	3614	1863
Flt Permitted	2028	0	0	2028	3014	1003
	2520	0	0	2520	2/1/	10/0
Satd. Flow (perm)	3539	0	0	3539	3614	1863
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						
Link Speed (mph)	30			30	30	
Link Distance (ft)	923			533	500	
Travel Time (s)	21.0			12.1	11.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	408	0	0	43	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	408	0	0	43	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12	J -		12	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	10			10	10	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	9	1.00	1.00	1.00	9
Number of Detectors	2	7	15	2	1	1
	∠ Thru			Z	Left	
Detector Template					Leit 20	Right
Leading Detector (ft)	100			100		20
Trailing Detector (ft)	0			0	0	0
Detector 1 Position(ft)	0			0	0	0
Detector 1 Size(ft)	6			6	20	20
Detector 1 Type	CI+Ex			CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0			0.0	0.0	0.0
Detector 1 Queue (s)	0.0			0.0	0.0	0.0
Detector 1 Delay (s)	0.0			0.0	0.0	0.0
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA			NA	Prot	Perm
Protected Phases				2		
	6			Z	4	
Permitted Phases	/			0	Λ	4
Detector Phase	6			2	4	4
Switch Phase	F 0			F 0	F 0	F 0
Minimum Initial (s)	5.0			5.0	5.0	5.0

05/26/2021

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	22.5	251		22.5	22.5	22.5
Total Split (s)	22.5			22.5	22.5	22.5
Total Split (%)	50.0%			50.0%	50.0%	50.0%
Maximum Green (s)	18.0			18.0	18.0	18.0
Yellow Time (s)	3.5			3.5	3.5	3.5
All-Red Time (s)	1.0			1.0	1.0	1.0
Lost Time Adjust (s)	0.0			0.0	0.0	0.0
Total Lost Time (s)	4.5			4.5	4.5	4.5
Lead/Lag	7.0			4.0	7.0	7.0
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0			3.0	3.0	3.0
Recall Mode	C-Max			C-Max	None	None
Walk Time (s)	7.0			C-IVIAX 7.0	7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	45.0			45.0		
Actuated g/C Ratio	1.00			1.00		
v/c Ratio	0.12			0.01		
Control Delay	0.1			0.0		
Queue Delay	0.0			0.0		
Total Delay	0.1			0.0		
LOS	А			A		
Approach Delay	0.1					
Approach LOS	А					
Queue Length 50th (ft)	0			0		
Queue Length 95th (ft)	0			0		
Internal Link Dist (ft)	843			453	420	
Turn Bay Length (ft)						
Base Capacity (vph)	3539			3539		
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.12			0.01		
Intersection Summary						
Area Type:	Other					
Cycle Length: 45						
Actuated Cycle Length: 45						
Offset: 0 (0%), Referenced		WBT and	6:FBT.	Start of G	reen	
Natural Cycle: 45			0.2017		10011	
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.12	orunatou					
Intersection Signal Delay:	0 1			Ir	ntersectio	n I OS· A
Intersection Capacity Utiliz					CU Level	
1 2	.ation 7.970			10	SO Level	
Analysis Period (min) 15						

Splits and Phases: 103:

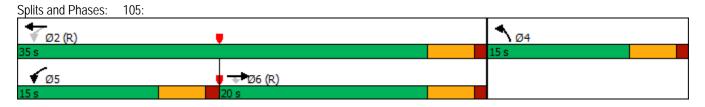
← Ø2 (R)	▲ Ø4	
22.5 s	22.5 s	
₩ 22.5 s		
22.5 s		

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EBT	EBR	WBL	WBT	NBL	NBR
A1⊅		ľ	<u></u>	ľ	1
144	49	20	40	0	0
404	49	20	40	0	0
1900	1900	1900	1900	1900	1900
0.95	0.95	1.00	0.95	1.00	1.00
3483	0	1770	3539	1863	1863
3483	0	1770	3539	1863	1863
30			30	30	
			404	428	
				0.92	0.92
439	53	22	43	0	0
	0	22		0	0
					No
	Right	Left			Right
			24	20	
			8	0	
16			16	16	
1.00	1.00	1.00	1.00	1.00	1.00
	9	15		15	9
Stop			Stop	Stop	
Other					
tion 15.5%			IC	U Level o	of Service A
	 ▲1 144 404 1900 0.95 3483 3483 3483 30 533 12.1 0.92 439 492 No Left 24 -12 16 1.00 Stop 	144 49 144 49 1900 1900 0.95 0.95 3483 0 3483 0 30 533 12.1 0.92 0.92 0.92 439 53 492 0 No No Left Right 24 -12 16 1.00 9 Stop Dther Dther	144 49 20 144 49 20 1900 1900 1900 1900 1900 1900 0.95 0.95 1.00 3483 0 1770 3483 0 1770 3483 0 1770 30 533 12.1 0.92 0.92 0.92 439 53 22 492 0 22 No No No Left Right Left 24 -12 16 1.00 1.00 1.00 9 15 Stop Dther Dther Dther	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	h h h h 14449204004044920400190019001900190019000.950.951.000.951.0034830177035391863348301770353918633030303053340442812.19.29.70.920.920.920.924395322430492022430NoNoNoNoNoLeftRightLeftLeftLeft242420-1280161.001.001.001.009151515stopStopStopStopStopStop

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>	1	<u></u>	† †	٦Y	
Traffic Volume (vph)	87	57	544	39	22	19
Future Volume (vph)	87	317	740	39	22	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	0.95
Frt	0.75	0.850	1.00	0.75	0.97	0.75
Flt Protected		0.000	0.950		0.930	
Satd. Flow (prot)	3539	1583	1770	3539	3273	0
Flt Permitted	3337	1000	0.547	3337	0.974	0
Satd. Flow (perm)	3539	1583	1019	3539	3273	0
	2028		1019	2028	3273	
Right Turn on Red		Yes			.01	Yes
Satd. Flow (RTOR)	25	345		25	21 25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	491			971	1149	
Travel Time (s)	9.6			18.9	31.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	95	345	804	42	24	21
Shared Lane Traffic (%)						
Lane Group Flow (vph)	95	345	804	42	45	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	24			24	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Number of Detectors	2	1	1	2	1	
Detector Template	Thru	Right	Left	Thru	Left	
Leading Detector (ft)	100	20	20	100	20	
Trailing Detector (ft)	0	20	20	0	0	
Detector 1 Position(ft)	0	0	0	0	0	
Detector 1 Size(ft)	6	20	20	6	20	
					20 Cl+Ex	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+EX	
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	Cl+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	pm+pt	NA	Prot	
Protected Phases	6		5	2	4	
Permitted Phases		6	2			
Detector Phase	6	6	5	2	4	
Switch Phase	-	-	-	_		
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	
	5.0	5.0	5.0	5.0	5.0	

	-	\mathbf{r}	4	-	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	20.0	20.0	9.5	22.5	15.0	
Total Split (s)	20.0	20.0	15.0	35.0	15.0	
Total Split (%)	40.0%	40.0%	30.0%	70.0%	30.0%	
Maximum Green (s)	15.5	15.5	10.5	30.5	10.5	
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max	None	C-Max	None	
Walk Time (s)	7.0	7.0				
Flash Dont Walk (s)	11.0	11.0				
Pedestrian Calls (#/hr)	0	0				
Act Effct Green (s)	19.7	19.7	41.1	43.8	6.0	
Actuated g/C Ratio	0.39	0.39	0.82	0.88	0.12	
v/c Ratio	0.07	0.41	0.74	0.01	0.11	
Control Delay	10.7	3.8	10.6	1.7	13.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	10.7	3.8	10.6	1.7	13.9	
LOS	В	А	В	А	В	
Approach Delay	5.3			10.2	13.9	
Approach LOS	А			В	В	
Queue Length 50th (ft)	6	0	0	0	3	
Queue Length 95th (ft)	23	45	#426	m3	14	
Internal Link Dist (ft)	411			891	1069	
Turn Bay Length (ft)						
Base Capacity (vph)	1394	832	1090	3099	703	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.07	0.41	0.74	0.01	0.06	
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50			==-		_	
Offset: 0 (0%), Referenced	to phase 2	:WBTL ar	nd 6:EBT,	Start of (Green	
Natural Cycle: 60						
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.74						
Intersection Signal Delay: 8					ntersection	
Intersection Capacity Utiliza	ation 48.5%)		10	CU Level	of Service A
Analysis Period (min) 15						
# 95th percentile volume			leue may	be longe	er.	
Queue shown is maximi						
m Volume for 95th percer	ntile queue	is metere	d by upst	ream sigr	nal.	

Lanes, Volumes, Timings 105:

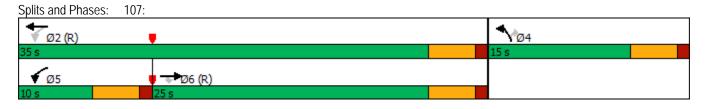


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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		††			† †			•			•	
Traffic Volume (vph)	0	99	7	150	582	15	0	0	4	0	0	0
Future Volume (vph)	0	99	7	150	778	15	0	0	4	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.990			0.998			0.865				
Flt Protected					0.992							
Satd. Flow (prot)	0	3504	0	0	3504	0	0	1611	0	0	1863	0
Flt Permitted					0.992							
Satd. Flow (perm)	0	3504	0	0	3504	0	0	1611	0	0	1863	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		971			260			262			305	
Travel Time (s)		22.1			5.9			6.0			6.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	108	8	163	846	16	0	0	4	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	116	0	0	1025	0	0	4	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		16			16			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
51	other											
Control Type: Unsignalized												
Intersection Capacity Utilization	on 34.2%			IC	CU Level	of Service	A					

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>			 ↑Ъ		
Traffic Volume (vph)	92	10	9	540	207	4
Future Volume (vph)	92	10	75	703	207	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	1700	14	1900	1300	1300	1700
Lane Util. Factor	0.95	1.00	0.95	0.95	1.00	1.00
Frt	0.75	0.850	0.75	0.75	1.00	0.850
Flt Protected		0.050		0.995	0.950	0.050
Satd. Flow (prot)	3539	1689	0	3522	1770	1583
Flt Permitted	3337	1009	0	0.916	0.950	1505
Satd. Flow (perm)	3539	1689	0	3242	1770	1583
Right Turn on Red	2024	Yes	U	JZ4Z	1770	Yes
		res 11				
Satd. Flow (RTOR)	25	11		25	25	4
Link Speed (mph)	25 777			25	25	
Link Distance (ft)	777			738	307	
Travel Time (s)	21.2	0.00	0.00	20.1	8.4	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	100	11	82	764	261	4
Shared Lane Traffic (%)				<u></u>		
Lane Group Flow (vph)	100	11	0	846	261	4
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	Left	Right
Median Width(ft)	16			16	36	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	0.92	1.00	1.00	1.00	1.00
Turning Speed (mph)		15	15		15	9
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (ft)	100	20	20	100	20	20
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	6	20	20	6	20	20
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)	94	0.0	0.0	94	0.0	0.0
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel	CITLA					
Detector 2 Extend (s)	0.0			0.0		
	0.0 NA	Perm	nmint	0.0 NA	Prot	Perm
Turn Type		Pelill	pm+pt	NA 2		Pelill
Protected Phases	6	L	5	2	4	Λ
Permitted Phases		6	2	0	1	4
Detector Phase	6	6	5	2	4	4
Switch Phase						

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	9.5	22.5	22.5	22.5
Total Split (s)	25.0	25.0	10.0	35.0	15.0	15.0
Total Split (%)	50.0%	50.0%	20.0%	70.0%	30.0%	30.0%
Maximum Green (s)	20.5	20.5	5.5	30.5	10.5	10.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None
Walk Time (s)	7.0	7.0		7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0		11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0		0	0	0
Act Effct Green (s)	31.0	31.0		31.0	10.0	10.0
Actuated g/C Ratio	0.62	0.62		0.62	0.20	0.20
v/c Ratio	0.02	0.02		0.42	0.20	0.20
Control Delay	0.03	0.01		6.1	33.7	11.5
Queue Delay	0.0	0.0		0.0	0.0	0.0
Total Delay	0.0	0.0		6.1	33.7	11.5
LOS	0.7 A	A		A	55.7 C	B
Approach Delay	0.7			6.1	33.3	U
Approach LOS	0.7 A			A	55.5 C	
Queue Length 50th (ft)	1	0		51	71	0
Queue Length 95th (ft)	1	0		94	#161	6
Internal Link Dist (ft)	697	0		658	227	0
Turn Bay Length (ft)	077			000	221	
Base Capacity (vph)	2191	1049		2007	371	335
Starvation Cap Reductn	2191			2007		
		0			0	0
Spillback Cap Reductn	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0
Reduced v/c Ratio	0.05	0.01		0.42	0.70	0.01
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50						
Offset: 0 (0%), Referenced		:WBTL ar	nd 6:FBT	Start of (Green	
Natural Cycle: 55				510.101		
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.74	2. 4					
Intersection Signal Delay:	11.5			Ir	ntersectio	n I OS B
Intersection Capacity Utiliz)				of Service
Analysis Period (min) 15						5. COI 1100
# 95th percentile volume	exceeds ca	nacity o	lelle mav	be longe	er.	
Queue shown is maxim			acue may	be longe		
		- ogolo3.				

Lanes, Volumes, Timings 107:



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	<u>۲</u> ۲	LDI				
Traffic Volume (vph)	"ו "ו 72	24	1 317	TT 999	TT 357	r 232
Future Volume (vph)	72	24	383	999	357	232 395
Ideal Flow (vphpl)	1900	1900	303 1900	1900	1900	1900
Lane Util. Factor	0.97	0.95	1.00	0.95	0.95	1,00
Frt	0.97	0.90	1.00	0.90	0.90	0.850
FIt Protected	0.962		0.950			0.000
	0.964 3351	0	0.950	3539	3539	1583
Satd. Flow (prot)	0.964	U		3039	3039	1003
Flt Permitted		0	0.405 754	2520	2520	1500
Satd. Flow (perm)	3351	0	/54	3539	3539	1583 Voc
Right Turn on Red	24	Yes				Yes
Satd. Flow (RTOR)	26			00	00	429
Link Speed (mph)	30			30	30	
Link Distance (ft)	738			727	965	
Travel Time (s)	16.8			16.5	21.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	78	26	416	1086	388	429
Shared Lane Traffic (%)						
Lane Group Flow (vph)	104	0	416	1086	388	429
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	36			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Turn Type	Prot		pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	
Permitted Phases	•		2	-	Ű	6
Minimum Split (s)	22.5		9.5	22.5	22.5	22.5
Total Split (s)	15.0		15.0	35.0	22.5	22.5
Total Split (%)	30.0%		30.0%	70.0%	40.0%	40.0%
Maximum Green (s)	10.5		10.5	30.5	40.0%	40.0%
	3.5					
Yellow Time (s)			3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag			Lead		Lag	Lag
Lead-Lag Optimize?			Yes		Yes	Yes
Walk Time (s)	7.0			7.0	7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	10.5		30.5	30.5	15.5	15.5
Actuated g/C Ratio	0.21		0.61	0.61	0.31	0.31
v/c Ratio	0.14		0.62	0.50	0.35	0.55
Control Delay	7.1		9.5	6.5	14.5	4.8
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	7.1		9.5	6.5	14.5	4.8
	1.1		7.5	0.0	1 4 .J	4.0

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
LOS	А		А	А	В	А
Approach Delay	7.1			7.3	9.4	
Approach LOS	А			А	А	
Queue Length 50th (ft)	9		51	77	45	0
Queue Length 95th (ft)	0		92	113	74	50
Internal Link Dist (ft)	658			647	885	
Turn Bay Length (ft)						
Base Capacity (vph)	724		673	2158	1097	786
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.14		0.62	0.50	0.35	0.55
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50						
Offset: 0 (0%), Referenced	to phase 2:	VBTL and	6:SBT,	Start of G	reen, Mas	ster Inters
Natural Cycle: 60						
Control Type: Pretimed						
Maximum v/c Ratio: 0.62						
Intersection Signal Delay: 8	3.0			In	tersection	LOS: A
Intersection Capacity Utiliz	ation 42.8%			IC	U Level c	of Service
Analysis Period (min) 15						
Splits and Phases: 108:						

		▶ _{Ø4}	
35 s		15 s	
▲ Ø5	📕 🕈 Ø6 (R)		
15 s	20 s		

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Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations	¢Î			†		
Traffic Volume (vph)	211	0	0	19	0	0
Future Volume (vph)	244	0	0	85	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						
Flt Protected						
Satd. Flow (prot)	1863	0	0	1863	0	0
Flt Permitted						
Satd. Flow (perm)	1863	0	0	1863	0	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	1082			1015	590	
Travel Time (s)	24.6			23.1	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	265	0	0	92	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	265	0	0	92	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	ation 14.4%			IC	CU Level o	of Service
Analysis Period (min) 15				10	2 20:010	50

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	•			•	Y		
Traffic Volume (vph)	28	0	0	19	0	183	
Future Volume (vph)	28	0	0	85	130	216	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt					0.916		
Flt Protected					0.982		
Satd. Flow (prot)	1863	0	0	1863	1676	0	
Flt Permitted					0.982		
Satd. Flow (perm)	1863	0	0	1863	1676	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	839			634	538		
Travel Time (s)	19.1			14.4	12.2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	30	0	0	92	141	235	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	30	0	0	92	376	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	12		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
51	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion 21.3%			IC	CU Level of	of Service	А
Analycic Dariad (min) 15							

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्भ	et 🗧		٦	1
Traffic Volume (vph)	0	0	0	19	28	0
Future Volume (vph)	0	0	0	215	28	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.865			
Flt Protected					0.950	
Satd. Flow (prot)	0	1863	1611	0	1770	1863
Flt Permitted					0.950	
Satd. Flow (perm)	0	1863	1611	0	1770	1863
Link Speed (mph)		30	30		30	
Link Distance (ft)		98	839		286	
Travel Time (s)		2.2	19.1		6.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	234	30	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	234	0	30	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		36	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Stop		Stop	
Intersection Summary						
	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	tion 13.3%			IC	CU Level o	of Service

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	∱1 ≱			<u></u>		1	
Traffic Volume (vph)	214	20	0	359	70	35	
Future Volume (vph)	214	20	0	619	70	35	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	0.95	0.95	1.00	0.95	1.00	1.00	
Frt	0.987					0.865	
Flt Protected					0.950		
Satd. Flow (prot)	3493	0	0	3539	0	1611	
Flt Permitted					0.950		
Satd. Flow (perm)	3493	0	0	3539	0	1611	
Link Speed (mph)	30			30	30		
Link Distance (ft)	404			491	211		
Travel Time (s)	9.2			11.2	4.8		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	233	22	0	673	76	38	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	255	0	0	673	76	38	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Right	
Median Width(ft)	12			24	0		
Link Offset(ft)	0			6	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion Err%			IC	CU Level	of Service	Η
Analysis Period (min) 15							

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Lane Group	SBL	SBR	NEL	NET	SWT	SWR
Lane Configurations		77	ሻሻ	•		
Traffic Volume (vph)	0	827	205	0	0	0
Future Volume (vph)	0	1087	205	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.88	0.97	1.00	1.00	1.00
Frt		0.850				
Flt Protected			0.950			
Satd. Flow (prot)	0	2787	3433	1863	0	0
Flt Permitted			0.950			
Satd. Flow (perm)	0	2787	3433	1863	0	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	227			1186	549	
Travel Time (s)	5.2			27.0	12.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	1182	223	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	1182	223	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			36	36	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Free			Free	Free	
Intersection Summary						
Area Type: (Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	tion 32.3%			IC	U Level o	of Service A
Analysis Dariad (min) 15						

	_#	\mathbf{F}	•	×	*	~
Lane Group	EBL	EBR	NEL	NET	SWT	SWR
Lane Configurations				<u></u>	^	1
Traffic Volume (vph)	0	0	0	205	342	485
Future Volume (vph)	0	0	0	205	602	485
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	0.91	1.00
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	0	0	3539	5085	1583
Flt Permitted						
Satd. Flow (perm)	0	0	0	3539	5085	1583
Link Speed (mph)	30			30	30	
Link Distance (ft)	1042			233	359	
Travel Time (s)	23.7			5.3	8.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	223	654	527
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	223	654	527
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	8			0	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Sign Control	Free			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 33.4%			IC	CU Level	of Service
Analysis Poriod (min) 15						

	L.	لر	~	•	•	~
Lane Group	SBL	SBR	NWL	NWR	NEL	NER
Lane Configurations		<u>, 11</u>		11001	ሻሻ	
Traffic Volume (vph)	1 0	827	1 0	r 0	11 205	r 0
Future Volume (vph)	0	1087	0	0	205	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1900	0.88	1.00	1.00	0.97	1.00
Frt	1.00	0.88	1.00	1.00	0.97	1.00
FIt Protected		0.850			0.950	
	1040	7707	1040	1040	3433	1863
Satd. Flow (prot)	1863	2787	1863	1863		1003
Flt Permitted	10/0	2707	10/0	10/0	0.950	10/0
Satd. Flow (perm)	1863	2787	1863	1863 Voc	3433	1863 Voc
Right Turn on Red		Yes		Yes		Yes
Satd. Flow (RTOR)	20	1920	20		20	
Link Speed (mph)	30		30		30	
Link Distance (ft)	763		723		430	
Travel Time (s)	17.3		16.4	0.00	9.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	1182	0	0	223	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	1182	0	0	223	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	30		32		32	
Link Offset(ft)	30		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15	9	15	9
Number of Detectors	0	0	0	0	0	0
Detector Template	Thru	Thru	Thru	Thru	Thru	Thru
Leading Detector (ft)	0	0	0	0	0	0
Trailing Detector (ft)	0	0	0	0	0	0
Turn Type	Prot	pt+ov	Prot	Perm	Prot	Perm
Protected Phases	5	5 6	4		6	
Permitted Phases	-			4		6
Detector Phase	5	56	4	4	6	6
Switch Phase	Ŭ	00			5	5
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (s)	15.5		15.5	15.5	19.0	19.0
Total Split (%)	31.0%		31.0%	31.0%	38.0%	38.0%
Maximum Green (s)	11.0		11.0	11.0	30.0 <i>%</i> 14.5	30.0 <i>%</i> 14.5
	3.5		3.5	3.5	14.5 3.5	
Yellow Time (s) All-Red Time (s)						3.5 1.0
	1.0		1.0	1.0	1.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag	Lead				Lag	Lag
Lead-Lag Optimize?	Yes				Yes	Yes
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	None		None	None	C-Max	C-Max

	L.	J.		*	•	~	
Lane Group	SBL	SBR	NWL	NWR	NEL	NER	
Walk Time (s)	7.0		7.0	7.0	7.0	7.0	
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	11.0	
Pedestrian Calls (#/hr)	0		0	0	0	0	
Act Effct Green (s)		50.0			33.9		
Actuated g/C Ratio		1.00			0.68		
v/c Ratio		0.42			0.10		
Control Delay		1.0			3.1		
Queue Delay		0.0			0.0		
Total Delay		1.0			3.1		
LOS		А			А		
Approach Delay	1.0				3.1		
Approach LOS	А				А		
Queue Length 50th (ft)		0			8		
Queue Length 95th (ft)		0			18		
Internal Link Dist (ft)	683		643		350		
Turn Bay Length (ft)							
Base Capacity (vph)		2773			2327		
Starvation Cap Reductn		0			0		
Spillback Cap Reductn		0			0		
Storage Cap Reductn		0			0		
Reduced v/c Ratio		0.43			0.10		
Intersection Summary							
J 1	Other						
Cycle Length: 50							
Actuated Cycle Length: 50							
Offset: 0 (0%), Referenced to	o phase 6:1	VEL, Star	t of Gree	n			
Natural Cycle: 70							
Control Type: Actuated-Coor	rdinated						
Maximum v/c Ratio: 0.42							
Intersection Signal Delay: 1.3	3			In	tersection	LOS: A	
Intersection Capacity Utilizat				IC	U Level o	f Service A	
Analysis Period (min) 15							
Splits and Phases: 102:							
		≜ ⊾					

A Ø5	🚽 🧏 Ø6 (R)	★ Ø4
15.5 s	19 s	15.5 s

Lane ConfigurationsTraffic Volume (vph)2Future Volume (vph)2	BT 205 205 205 900	EBR 0	WBL	WBT ↑↑	NBL	NBR
Lane ConfigurationsTraffic Volume (vph)2Future Volume (vph)2Ideal Flow (vphpl)19Lane Util. Factor0Frt0	↑↑ 205 205 900	0				
Traffic Volume (vph)2Future Volume (vph)2Ideal Flow (vphpl)19Lane Util. Factor0.Frt0.0	205 205 900			TT	ካካ	1
Future Volume (vph)2Ideal Flow (vphpl)19Lane Util. Factor0.Frt0.	205 900		0	429	399	29
Ideal Flow (vphpl)19Lane Util. Factor0.Frt0.	900	0	0	689	399	29
Lane Util. Factor 0. Frt		1900	1900	1900	1900	1900
Frt	95	1.00	1.00	0.95	0.97	1.00
		1.00	1.00	0.75	0.77	0.850
					0.950	0.000
	539	0	0	3539	3433	1583
Flt Permitted	007	0	0	3337	0.950	1303
	539	0	0	3539	3433	1583
Right Turn on Red	007	Yes	0	3337	3433	Yes
Satd. Flow (RTOR)		162				32
	30			30	30	32
. ,	923			533	500	
	21.0	0.00	0.00	12.1	11.4	0.00
).92	0.92	0.92	0.92	0.92	0.92
	223	0	0	749	434	32
Shared Lane Traffic (%)	0.00					
1 1 1	223	0	0	749	434	32
	No	No	No	No	No	No
5	Left	Right	Left	Left	Left	Right
. ,	12			12	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor 1.	.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Number of Detectors	2			2	1	1
Detector Template TI	hru			Thru	Left	Right
	100			100	20	20
Trailing Detector (ft)	0			0	0	0
Detector 1 Position(ft)	0			0	0	0
Detector 1 Size(ft)	6			6	20	20
Detector 1 Type CI+				CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel				OFLA	OFFER	OFFER
	0.0			0.0	0.0	0.0
	0.0					0.0
()				0.0	0.0	
J ()	0.0			0.0	0.0	0.0
, ,	94			94		
Detector 2 Size(ft)	6			6		
	+Ex			CI+Ex		
Detector 2 Channel						
	0.0			0.0		
51	NA			NA	Prot	Perm
Protected Phases	6			2	4	
Permitted Phases						4
Detector Phase	6			2	4	4
Switch Phase						
	5.0			5.0	5.0	5.0

Synchro 11 Report Page 6

	-	\mathbf{r}	4	+	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	22.5	2311		22.5	22.5	22.5
Total Split (s)	26.0			26.0	24.0	24.0
Total Split (%)	52.0%			52.0%	48.0%	48.0%
Maximum Green (s)	21.5			21.5	19.5	19.5
Yellow Time (s)	3.5			3.5	3.5	3.5
All-Red Time (s)	1.0			1.0	1.0	1.0
Lost Time Adjust (s)	0.0			0.0	0.0	0.0
Total Lost Time (s)	4.5			4.5	4.5	4.5
Lead/Lag	4.0			4.5	4.5	4.0
Lead-Lag Optimize?						
	3.0			2.0	2.0	3.0
Vehicle Extension (s)				3.0	3.0	
Recall Mode	C-Max			C-Max	None	None
Walk Time (s)	7.0			7.0	7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	29.3			29.3	11.7	11.7
Actuated g/C Ratio	0.59			0.59	0.23	0.23
v/c Ratio	0.11			0.36	0.54	0.08
Control Delay	4.4			6.6	18.9	6.3
Queue Delay	0.0			0.0	0.0	0.0
Total Delay	4.4			6.6	18.9	6.3
LOS	А			А	В	А
Approach Delay	4.4			6.6	18.0	
Approach LOS	А			А	В	
Queue Length 50th (ft)	11			51	57	0
Queue Length 95th (ft)	18			97	81	14
Internal Link Dist (ft)	843			453	420	
Turn Bay Length (ft)	010			100	120	
Base Capacity (vph)	2072			2072	1338	636
Starvation Cap Reductn	0			0	0	0.00
Spillback Cap Reductn	0			0	0	0
Storage Cap Reductn	0			0	0	0
Reduced v/c Ratio						
KEUULEU VIL KALIO	0.11			0.36	0.32	0.05
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50	1					
Offset: 0 (0%), Referenced	d to phase 2:\	NBT and	6:EBT, 3	Start of G	reen	
Natural Cycle: 45						
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.54						
Intersection Signal Delay:	10.0			lr	ntersectio	n LOS: A
Intersection Capacity Utiliz						of Service
Analysis Period (min) 15						

Splits and Phases: 103:

← Ø2 (R)	▲ Ø4
26 s	24 s
, → Ø6 (R)	
26 s	

-	\mathbf{F}	1	-	1	1
EBT	EBR	WBL	WBT	NBL	NBR
A ₽		ľ	<u></u>	ľ	1
234	0	0	429	0	0
234	0	0	689	0	0
1900	1900	1900	1900	1900	1900
0.95	0.95	1.00	0.95	1.00	1.00
3539	0	1863	3539	1863	1863
3539	0	1863	3539	1863	1863
30			30	30	
533			404	428	
12.1			9.2	9.7	
0.92	0.92	0.92	0.92	0.92	0.92
254	0	0	749	0	0
	0	0	749	0	0
	No	No	No	No	No
	Right	Left			Right
			24	20	
			8	0	
16			16	16	
1.00	1.00	1.00	1.00	1.00	1.00
	9	15		15	9
Stop			Stop	Stop	
Other					
tion 15.2%			IC	U Level o	of Service A
	 ▲1> 234 234 234 1900 0.95 3539 3539 30 533 12.1 0.92 254 254 No Left 24 -12 16 1.00 Stop 	1900 1900 234 0 1900 1900 0.95 0.95 3539 0 3539 0 30 533 12.1 0.92 0.92 0.92 254 0 No No Left Right 24 -12 16 1.00 9 Stop Dther Dther	10 1 234 0 0 234 0 0 1900 1900 1900 1900 1900 1900 0.95 0.95 1.00 3539 0 1863 3539 0 1863 30 533 12.1 0.92 0.92 0.92 254 0 0 254 0 0 254 0 0 254 0 0 254 0 0 254 0 1.00 1.00 No No 1.00 1.00 1.00 9 15 Stop Dther	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$

	-	$\mathbf{\hat{v}}$	4	-	1	۲
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>	1	<u>אשר</u>	1	٦Y	HDR
Traffic Volume (vph)	237	12	57	92	267	670
Future Volume (vph)	237	12	57	92	527	865
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	0.95
Frt	0.75	0.850	1.00	0.75	0.97	0.75
Flt Protected		0.000	0.950		0.907	
Satd. Flow (prot)	3539	1583	1770	3539	3215	0
Flt Permitted	2029	1000	0.441	3039	0.981	0
	3539	1583	821	3539	3215	0
Satd. Flow (perm)	3039		0Z I	3037	3215	
Right Turn on Red		Yes			F00	Yes
Satd. Flow (RTOR)	05	13		05	503	
Link Speed (mph)	35			35	25	
Link Distance (ft)	491			971	1149	
Travel Time (s)	9.6			18.9	31.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	258	13	62	100	573	940
Shared Lane Traffic (%)						
Lane Group Flow (vph)	258	13	62	100	1513	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	24	5		24	24	5
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane				10	10	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	9	1.00	1.00	1.00	9
Number of Detectors	2	9	15	2	15	7
Detector Template	∠ Thru		Left	Z Thru	Left	
		Right				
Leading Detector (ft)	100	20	20	100	20	
Trailing Detector (ft)	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	
Detector 1 Size(ft)	6	20	20	6	20	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	Cl+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	pm+pt	NA	Prot	
Protected Phases	6	1 GIIII	рш+рі 5	2	4	
Permitted Phases	U	6	2	2	4	
Detector Phase	6		5	2	4	
	6	6	C	Z	4	
Switch Phase	ГО	ГО	ГО	ГО	ΓΛ	
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	

06 Alt 01 PM Existing Adjusted 650 adjusted with 650 added trips 2:58 pm 04/21/2021 1

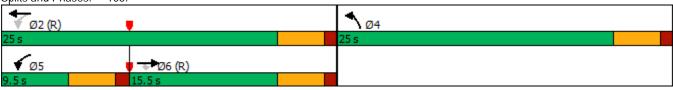
	-	\mathbf{r}	4	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Minimum Split (s)	15.0	15.0	9.5	22.5	15.0		
Total Split (s)	15.5	15.5	9.5	25.0	25.0		
Total Split (%)	31.0%	31.0%	19.0%	50.0%	50.0%		
Maximum Green (s)	11.0	11.0	5.0	20.5	20.5		
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5		
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0		
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5		
Lead/Lag	Lag	Lag	Lead				
Lead-Lag Optimize?	Yes	Yes	Yes				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		
Recall Mode	C-Max	C-Max	None	C-Max	None		
Walk Time (s)	7.0	7.0					
Flash Dont Walk (s)	11.0	11.0					
Pedestrian Calls (#/hr)	0	0					
Act Effct Green (s)	14.9	14.9	20.6	20.6	20.4		
Actuated g/C Ratio	0.30	0.30	0.41	0.41	0.41		
v/c Ratio	0.24	0.03	0.14	0.07	0.98dr		
Control Delay	15.8	9.2	11.0	10.2	23.8		
Queue Delay	0.0	0.0	0.0	0.0	0.0		
Total Delay	15.8	9.2	11.0	10.2	23.8		
LOS	B	А	В	В	С		
Approach Delay	15.5			10.5	23.8		
Approach LOS	В	0		В	C		
Queue Length 50th (ft)	33	0	11	9	140		
Queue Length 95th (ft)	60	10	36	26	#293		
Internal Link Dist (ft)	411			891	1069		
Turn Bay Length (ft)	1054	400	100	1 / [7	1/1/		
Base Capacity (vph)	1054	480	435	1457	1614		
Starvation Cap Reductn	0	0	0	0	0		
Spillback Cap Reductn	0 0		0	0	0		
Storage Cap Reductn Reduced v/c Ratio	0.24	0 0.03	0.14	0 0.07	0.94		
	0.24	0.03	0.14	0.07	0.94		
Intersection Summary	01						
51	Other						
Cycle Length: 50							
Actuated Cycle Length: 50	h				^		
Offset: 0 (0%), Referenced Natural Cycle: 60	to phase 2	:WBIL ar	10 6:EBT,	Start of (Green		
Control Type: Actuated-Coc	ordinated						
Maximum v/c Ratio: 0.94							
Intersection Signal Delay: 2					ntersectior		
Intersection Capacity Utiliza	tion 50.8%)		10	CU Level	of Service A	
Analysis Period (min) 15							
# 95th percentile volume e			leue may	be longe	er.		
Queue shown is maximu							
dr Defacto Right Lane. Re	ecode with	1 though	lane as a	a right lan	le.		

06 Alt 01 PM Existing Adjusted 650 adjusted with 650 added trips 2:58 pm 04/21/2021 1

Synchro 11 Report Page 10

Lanes, Volumes, Timings 105:

Splits and Phases: 105:



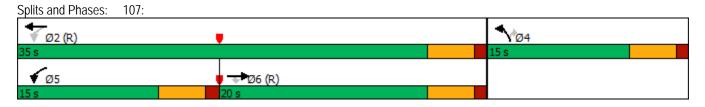
	۶	-	\mathbf{r}	•	+	*	•	Ť	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		^			† †			•			†	
Traffic Volume (vph)	0	907	0	2	119	2	27	0	17	0	0	4
Future Volume (vph)	0	1102	0	2	119	2	27	0	17	0	0	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.998			0.948			0.865	
Flt Protected					0.999			0.970				
Satd. Flow (prot)	0	3539	0	0	3529	0	0	1713	0	0	1611	0
Flt Permitted					0.999			0.970				
Satd. Flow (perm)	0	3539	0	0	3529	0	0	1713	0	0	1611	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		971			260			262			305	
Travel Time (s)		22.1			5.9			6.0			6.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	1198	0	2	129	2	29	0	18	0	0	4
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1198	0	0	133	0	0	47	0	0	4	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		16			16			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
51	ther											
Control Type: Unsignalized												
Intersection Capacity Utilization	on 40.9%			IC	CU Level of	of Service	А					

	-	\mathbf{i}	4	+	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations				<u>↑</u>		
Traffic Volume (vph)	TT 567	357	4	H T 122	0	19
Future Volume (vph)	697	422	4	122	0	85
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	1.00	0.95	0.95	1.00	1.00
Frt	0.95		0.90	0.95	1.00	0.850
Fit Protected		0.850		0.999		0.650
	3539	1689	0	3536	1863	1583
Satd. Flow (prot)	3539	1089	0		1803	1583
Flt Permitted	25.20	1/00	0	0.943	10/0	1500
Satd. Flow (perm)	3539	1689	0	3337	1863	1583
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	0.5	459		0.5	0.5	465
Link Speed (mph)	25			25	25	
Link Distance (ft)	777			738	307	
Travel Time (s)	21.2			20.1	8.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	758	459	4	133	0	92
Shared Lane Traffic (%)						
Lane Group Flow (vph)	758	459	0	137	0	92
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	Left	Right
Median Width(ft)	16			16	36	5
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	.5					
Headway Factor	1.00	0.92	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	15	15	1.00	15	9
Number of Detectors	2	13	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (ft)	100	Right 20	20	100	20	Right 20
Trailing Detector (ft)	0	20	20	0	20	
Detector 1 Position(ft)						0
	0	0	0	0	0	0
Detector 1 Size(ft)	6	20	20	6	20	20
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	6		5	2	4	
Permitted Phases		6	2			4
Detector Phase	6	6	5	2	4	4
Switch Phase	0	0	0	2		•

06 Alt 01 PM Existing Adjusted 650 adjusted with 650 added trips 2:58 pm 04/21/2021 1

m Volume for 95th percentile queue is metered by upstream signal.

Lanes, Volumes, Timings 107:



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	<u>אץ</u>	LDK				
Traffic Volume (vph)	4 70	115	1 40	↑↑ 1170	↑↑ 922	r 85
Future Volume (vph)	568	213	40	1170	922	85
Ideal Flow (vphpl)	1900	1900	1900	1900	922 1900	1900
Lane Util. Factor	0.97	0.95	1.00	0.95	0.95	1,00
Frt	0.97	0.90	1.00	0.90	0.90	0.850
Fit Protected	0.959		0.950			0.000
	0.965 3344	0	0.950	3539	3539	1583
Satd. Flow (prot)	3344 0.965	U	0.156	3039	3039	1003
Flt Permitted		0		2520	2520	1500
Satd. Flow (perm)	3344	0	291	3539	3539	1583 Voc
Right Turn on Red	105	Yes				Yes
Satd. Flow (RTOR)	105			4	45	92
Link Speed (mph)	25			45	45	
Link Distance (ft)	738			727	965	
Travel Time (s)	20.1		_	11.0	14.6	_
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	617	232	43	1272	1002	92
Shared Lane Traffic (%)						
Lane Group Flow (vph)	849	0	43	1272	1002	92
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	36			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Turn Type	Prot		pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	
Permitted Phases	1		2	2	U	6
Minimum Split (s)	22.5		9.5	22.5	22.5	22.5
Total Split (s)	16.0		8.0	34.0	26.0	26.0
	32.0%		16.0%	68.0%	52.0%	52.0%
Total Split (%)						
Maximum Green (s)	11.5		3.5	29.5	21.5	21.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag			Lead		Lag	Lag
Lead-Lag Optimize?			Yes		Yes	Yes
Walk Time (s)	7.0			7.0	7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	11.5		29.5	29.5	21.5	21.5
Actuated g/C Ratio	0.23		0.59	0.59	0.43	0.43
v/c Ratio	1.00		0.16	0.61	0.66	0.13
Control Delay	53.4		5.6	8.1	13.9	3.0
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	53.4		5.6	8.1	13.9	3.0
	JJ.4		0.C	Ŏ. I	13.9	3.0

06 Alt 01 PM Existing Adjusted 650 adjusted with 650 added trips 2:58 pm 04/21/2021 1

Synchro 11 Report Page 16

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Lane Group	EBL	EBR N	IBL NI	BT S	SBT	SBR	
LOS	D		А	А	В	А	
Approach Delay	53.4		8	8.1 1	3.0		
Approach LOS	D			А	В		
Queue Length 50th (ft)	129		5 1	05 [^]	115	0	
Queue Length 95th (ft)	#230		13 1	54 ⁻	169	19	
Internal Link Dist (ft)	658		6	47 8	885		
Turn Bay Length (ft)							
Base Capacity (vph)	849		275 20	88 15	521	733	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	1.00	0	.16 0.	61 0).66	0.13	
Intersection Summary							
Area Type:	Other						
Cycle Length: 50							
Actuated Cycle Length: 5							
Offset: 0 (0%), Reference	ed to phase 2:	NBTL and 6:S	SBT, Start	of Greer	n, Mas	ter Inters	section
Natural Cycle: 60							
Control Type: Pretimed							
Maximum v/c Ratio: 1.00							
Intersection Signal Delay:				Interse	ection	LOS: C	
Intersection Capacity Utili	ization 57.8%			ICU L	evel o	f Service	В
Analysis Period (min) 15							
# 95th percentile volum	e exceeds ca	pacity, queue	may be lo	nger.			
Queue shown is maxir	num after two	cycles.					
Splits and Phases: 108).						
Spiils and Fhases. 100).						

Ø2 (R)	•	▶	
34 s		16 s	
▲ ø5	♥ ♥ Ø6 (R)		
8 s	26 s		

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Lane Group	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations	f,			†		
Traffic Volume (vph)	19	0	361	0	0	0
Future Volume (vph)	85	130	426	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.918					
Flt Protected				0.950		
Satd. Flow (prot)	1710	0	0	1770	0	0
Flt Permitted				0.950		
Satd. Flow (perm)	1710	0	0	1770	0	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	1082			1015	590	
Travel Time (s)	24.6			23.1	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	92	141	463	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	233	0	0	463	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type: (Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	tion 30.0%			IC	U Level o	of Service
Analysis Doriod (min) 15						

	-	\mathbf{r}	4	-	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	•			•	Y	
Traffic Volume (vph)	0	0	0	0	0	19
Future Volume (vph)	196	0	0	0	0	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.865	
Flt Protected						
Satd. Flow (prot)	1863	0	0	1863	1611	0
Flt Permitted						
Satd. Flow (perm)	1863	0	0	1863	1611	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	839			634	538	
Travel Time (s)	19.1			14.4	12.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	213	0	0	0	0	21
Shared Lane Traffic (%)						
Lane Group Flow (vph)	213	0	0	0	21	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 13.3%			IC	U Level o	of Service
Analysis Poriod (min) 15						

	≯	-	-	•	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ę	et		5	1
Traffic Volume (vph)	0	0	0	0	0	0
Future Volume (vph)	0	0	0	0	196	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	1863	1863	0	1770	1863
Flt Permitted					0.950	
Satd. Flow (perm)	0	1863	1863	0	1770	1863
Link Speed (mph)		30	30		30	
Link Distance (ft)		98	839		286	
Travel Time (s)		2.2	19.1		6.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	0	213	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	213	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		36	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Stop		Stop	
Intersection Summary						
J 1	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 13.3%			IC	CU Level o	of Service

	-	\mathbf{i}	4	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	∱ ⊅			<u></u>		1	
Traffic Volume (vph)	144	0	0	60	0	0	
Future Volume (vph)	544	0	0	60	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	0.95	0.95	1.00	0.95	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	3539	0	0	3539	0	1863	
Flt Permitted							
Satd. Flow (perm)	3539	0	0	3539	0	1863	
Link Speed (mph)	30			30	30		
Link Distance (ft)	404			491	211		
Travel Time (s)	9.2			11.2	4.8		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	591	0	0	65	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	591	0	0	65	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Right	
Median Width(ft)	12			24	0		
Link Offset(ft)	0			6	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type: (Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	ion 7.3%			IC	U Level o	of Service A	А
Analysis Period (min) 15							

	G.	¥	•	*	*	ŧ۷	
Lane Group	SBL	SBR	NEL	NET	SWT	SWR	
Lane Configurations		77	ሻሻ	•			
Traffic Volume (vph)	0	40	192	592	0	0	
Future Volume (vph)	0	40	592	592	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	0.88	0.97	1.00	1.00	1.00	
Frt		0.850					
Flt Protected			0.950				
Satd. Flow (prot)	0	2787	3433	1863	0	0	
Flt Permitted			0.950				
Satd. Flow (perm)	0	2787	3433	1863	0	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	227			1186	549		
Travel Time (s)	5.2			27.0	12.5		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	43	643	643	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	43	643	643	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	12			36	36		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15	9	15			9	
Sign Control	Free			Free	Free		
Intersection Summary							
J 1	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	ation 34.5%			IC	U Level o	of Service A	A
Analysis Poriod (min) 15							

	_#	\mathbf{F}	•	×	*	~		
Lane Group	EBL	EBR	NEL	NET	SWT	SWR		
Lane Configurations				^	^	1		
Traffic Volume (vph)	0	0	0	470	14	10		
Future Volume (vph)	0	0	0	870	14	10		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.91	1.00		
Frt						0.850		
Flt Protected								
Satd. Flow (prot)	0	0	0	3539	5085	1583		
Flt Permitted								
Satd. Flow (perm)	0	0	0	3539	5085	1583		
Link Speed (mph)	30			30	30			
Link Distance (ft)	1042			233	359			
Travel Time (s)	23.7			5.3	8.2			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0	0	0	946	15	11		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	0	0	946	15	11		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Left	Left	Right		
Median Width(ft)	8			0	24			
Link Offset(ft)	0			0	0			
Crosswalk Width(ft)	16			16	16			
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15	9	15			9		
Sign Control	Free			Free	Free			
Intersection Summary								
Area Type:	Other							
Control Type: Unsignalized								
Intersection Capacity Utiliza	tion 16.3%			IC	CU Level	of Service	eΑ	
Analysis Doriod (min) 15								

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Lane Group	SBL	SBR	NWL	NWR	NEL	NER
Lane Configurations		<u>, 11</u>		10001	ሻሻ	
Traffic Volume (vph)	1 0	6 1 1 40	1 0	r 0	71 784	r 0
Future Volume (vph)	0	40	0	0	1184	0
Ideal Flow (vphpl)	1900	40 1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.88	1.00	1.00	0.97	1.00
Frt	1.00	0.88	1.00	1.00	0.97	1.00
		0.000				
Flt Protected	10/0	2202	10/0	10/0	0.950	10/0
Satd. Flow (prot)	1863	2787	1863	1863	3433	1863
Flt Permitted	10/0	0707	10/0	10/0	0.950	10/0
Satd. Flow (perm)	1863	2787	1863	1863	3433	1863
Right Turn on Red		Yes		Yes		Yes
Satd. Flow (RTOR)		1920				
Link Speed (mph)	30		30		30	
Link Distance (ft)	763		723		430	
Travel Time (s)	17.3		16.4		9.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	43	0	0	1287	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	43	0	0	1287	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	30		32		32	
Link Offset(ft)	30		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane	10		10		10	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	9	1.00	9	1.00	9
	0	9	0	9	0	9
Number of Detectors						
Detector Template	Thru	Thru	Thru	Thru	Thru	Thru
Leading Detector (ft)	0	0	0	0	0	0
Trailing Detector (ft)	0	0	0	0	0	0
Turn Type	Prot	pt+ov	Prot	Perm	Prot	Perm
Protected Phases	5	56	4		6	
Permitted Phases				4		6
Detector Phase	5	56	4	4	6	6
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (s)	15.5		15.5	15.5	19.0	19.0
Total Split (%)	31.0%		31.0%	31.0%	38.0%	38.0%
Maximum Green (s)	11.0		11.0	11.0	14.5	14.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	3.5 1.0	3.0 1.0	3.5 1.0
.,						
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag	Lead				Lag	Lag
Lead-Lag Optimize?	Yes		_		Yes	Yes
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	None		None	None	C-Max	C-Max

	L.	J.		*	•	~	
Lane Group	SBL	SBR	NWL	NWR	NEL	NER	
Walk Time (s)	7.0		7.0	7.0	7.0	7.0	
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	11.0	
Pedestrian Calls (#/hr)	0		0	0	0	0	
Act Effct Green (s)		50.0			44.2		
Actuated g/C Ratio		1.00			0.88		
v/c Ratio		0.02			0.42		
Control Delay		0.0			2.3		
Queue Delay		0.0			0.0		
Total Delay		0.0			2.3		
LOS		А			А		
Approach Delay					2.3		
Approach LOS					А		
Queue Length 50th (ft)		0			0		
Queue Length 95th (ft)		0			92		
Internal Link Dist (ft)	683		643		350		
Turn Bay Length (ft)							
Base Capacity (vph)		2787			3035		
Starvation Cap Reductn		0			0		
Spillback Cap Reductn		0			0		
Storage Cap Reductn		0			0		
Reduced v/c Ratio		0.02			0.42		
Intersection Summary							
51	Other						
Cycle Length: 50							
Actuated Cycle Length: 50							
Offset: 0 (0%), Referenced t	to phase 6:I	VEL, Stai	t of Gree	n			
Natural Cycle: 80							
Control Type: Actuated-Coo	rdinated						
Maximum v/c Ratio: 0.42							
Intersection Signal Delay: 2.					tersection		
Intersection Capacity Utilization	tion 26.1%			IC	U Level c	of Service A	
Analysis Period (min) 15							
Splits and Phases: 102:							
		- I J 4					

A 05	• 😾 Ø6 (R)	▶ @4
15.5 s	19 s	15.5 s

Lane GroupEBTEBRWBLWBTNBLNBRLane Configurations $\uparrow \uparrow$ $\uparrow \uparrow$ $\uparrow \uparrow$ $\uparrow \uparrow$ Traffic Volume (vph)115004000Future Volume (vph)155004000Ideal Flow (vph)19001900190019001900Lane Util. Factor0.951.001.000.950.971.00FrtFit ProtectedSatal. Flow (prot)353900353936141863Sight Flow (prot)3539003539361418631863Right Turn on RedYesYesYesYesSatd. Flow (RTOR)21.012.111.4Peak Hour Factor0.920.920.920.92Link Speed (mph)3030303000Shared Lane Traffic (%)1.001.001.000Lane Group Flow (vph)56000430000Crosswalk Width(ft)1212241.001.001.001.00Link Offset(ft)00000000Crosswalk Width(ft)161616161616Word State (ft)0000000Link Offset(ft)0000000Crosswalk Width(ft)1616161616
Lane Configurations A Y Y Y Traffic Volume (vph) 115 0 0 40 0 0 Future Volume (vph) 155 0 0 40 0 0 Ideal Flow (vph) 1900 1900 1900 1900 1900 1900 Lane Util. Factor 0.95 1.00 1.00 0.95 0.97 1.00 Fit Protected Satal. Flow (port) 3539 0 0 3539 3614 1863 Right Turn on Red Yes Yes Yes Yes Yes Yes Satd. Flow (prOR) 30 30 30 30 30 10 Link Speed (mph) 30 30 30 30 30 111.1 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 Link Speed (mph) 560 0 0 43 0 0 Lane Alignment Left Right <td< td=""></td<>
Traffic Volume (vph) 115 0 0 40 0 0 Future Volume (vph) 1515 0 0 400 1900 1100 111 14 1863 Right Michici Mich
Future Volume (vph) 515 0 0 400 100 1900 1900 Lane Util. Factor 0.95 1.00 1.00 0.95 0.97 1.00 Fit Frater
ideal Flow (vphpl) 1900 1900 1900 1900 1900 Lane Util. Factor 0.95 1.00 1.00 0.95 0.97 1.00 Frt Fit Fritocted 5 1.00 0.95 0.97 1.00 Satd. Flow (port) 3539 0 0 3539 3614 1863 Right Turn on Red Yes Yes Yes Yes Yes Satd. Flow (RTOR) 100 12.1 11.4 Peak Hour Factor 0.92
Lane Util. Factor 0.95 1.00 1.00 0.95 0.97 1.00 Fit Fit Protected 5 5 5 5 5 1.00 3539 0 0 3539 3614 1863 Fit Permitted 3539 0 0 3539 3614 1863 Right Turn on Red Yes Yes Yes Yes Satd. Flow (RTOR) 11.1 4 1863 Link Speed (mph) 30 30 30 30 Link Distance (ft) 923 533 500 533 500 Travel Time (s) 21.0 12.1 11.4 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 560 0 0 43 0 0 Shared Lane Traffic (%) 1 12 12 24 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Frt Flt Protected Sald, Flow (prot) 3539 0 0 3539 3614 1863 Flt Permitted 3539 0 0 3539 3614 1863 Right Turn on Red Yes Yes Yes Yes Satd, Flow (perm) 30 30 30 1164 Link Distance (ft) 923 533 500 Travel Time (s) 21.0 11.1 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 Adj, Flow (vph) 560 0 0 43 0 0 Shared Lane Traffic (%) 12 12 24 1164 Lane Group Flow (vph) 560 0 0 43 0 0 Enter Blocked Intersection No No No No No No Lane Alignment Left Right Left Left Left Right Median Width(ft) 16 16 16 16 16 Trave Turu Lane Headway Factor 1.00<
Fit Protected Satd. Flow (prot) 3539 0 0 3539 3614 1863 Fit Permitted 3539 0 0 3539 3614 1863 Satd. Flow (perm) 3539 0 0 3539 3614 1863 Right Turn on Red Yes Yes Yes Yes Yes Link Speed (mph) 30 30 30 30 1 Link Speed (mph) 30 20.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 560 0 0 43 0 0 Shared Lane Traffic (%) 1 12 12 24 1 Lane Group Flow (vph) 560 0 0 43 0 0 Eane Alignment Left Right Left Left Right Median Width(ft) 12 12 24 1 1 Median Width(ft) 16 16 16 16 100 Two way Left Turn Lane 1 100 1.00 1.00 1.00
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Fit Permitted Satd. Flow (perm) 3539 0 0 3539 3614 1863 Right Turn on Red Yes Yes Yes Satd. Flow (RTOR) 1 1 1863 Link Speed (mph) 30 30 30 Link Distance (ft) 923 533 500 Travel Time (s) 21.0 12.1 11.4 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 560 0 0 43 0 0 Shared Lane Traffic (%) Lane Group Flow (vph) 560 0 0 43 0 0 Lane Group Flow (vph) 560 0 0 43 0 0 Enter Blocked Intersection No No No No No No Lane Group Flow (vph) 12 12 24 1 1 0 0 0 0 0 0 100 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 <td< td=""></td<>
Satd. Flow (perm) 3539 0 0 3539 3614 1863 Right Turn on Red Yes Yes Yes Yes Satd. Flow (RTOR) 1 1 Yes Yes Link Speed (mph) 30 30 30 30 Link Distance (ft) 923 533 500 Travel Time (s) 21.0 12.1 11.4 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 560 0 0 43 0 0 Shared Lane Traffic (%) 1 Left Left Right Left Right Lane Alignment Left Right Left Left Right No No Link Offset(ft) 0 0 0 0 0 0 Corsswalk Width(ft) 16 16 16 16 16 16 16 16 100 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Right Turn on Red Yes Yes Satd. Flow (RTOR) Uink Speed (mph) 30 30 30 Link Distance (ft) 923 533 500 Travel Time (s) 21.0 12.1 11.4 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 560 0 0 43 0 0 Shared Lane Traffic (%) 0 0 43 0 0 Lane Group Flow (vph) 560 0 0 43 0 0 0 Lane Alignment Left Right Left Left Right Median Width(ft) 12 24 Unroling Speed (mph) 9 15 15 9 9 100 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Satd. Flow (RTOR) Link Speed (mph) 30 30 30 Link Distance (ft) 923 533 500 Travel Time (s) 21.0 12.1 11.4 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 560 0 0 43 0 0 Shared Lane Traffic (%) 0 0 Lane Group Flow (vph) 560 0 0 43 0 0 0 Enter Blocked Intersection No No No No No No No Link Offset(ft) 0 0 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 100 1.00
Link Speed (mph) 30 30 30 Link Distance (ft) 923 533 500 Travel Time (s) 21.0 12.1 11.4 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 560 0 0 43 0 0 Shared Lane Traffic (%) Lane Group Flow (vph) 560 0 0 43 0 0 Lane Group Flow (vph) 560 0 0 43 0 0 Enter Blocked Intersection No No No No No No Link Offset(ft) 0 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 100 1.00 1.00 Turning Speed (mph) 9 15 15 9 Number of Detectors 2 1 1 Detector Template Thru Left Right Leading Detector (ft) 0 0 0
Link Distance (ft)923533500Travel Time (s)21.012.111.4Peak Hour Factor0.920.920.920.920.92Adj. Flow (vph)560004300Shared Lane Traffic (%)0Lane Group Flow (vph)560004300Enter Blocked IntersectionNoNoNoNoNoLane AlignmentLeftRightLeftLeftLeftRightMedian Width(ft)1212241224Link Offset(ft)00000Crosswalk Width(ft)16161616Two way Left Turn Lane1599Headway Factor1.001.001.001.001.00Turning Speed (mph)915159Number of Detectors2211Detector TemplateThruThruLeftRightLeading Detector (ft)100000Detector 1 Size(ft)662020Detector 1 Size(ft)662020Detector 1 Size(ft)66620Detector 1 Size(ft)66620Detector 2 Position(ft)949494Detector 2 ChannelUsenceUsenceUsenceDetector 2 Size(ft)6662 </td
Travel Time (s) 21.0 12.1 11.4 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 Adj. Flow (vph) 560 0 0 43 0 0 Shared Lane Traffic (%) 0 0 Lane Group Flow (vph) 560 0 0 43 0 0 Eane Group Flow (vph) 560 0 0 43 0 0 Lane Group Flow (vph) 560 0 0 43 0 0 Lane Alignment Left Right Left Left Left Right Median Width(ft) 12 12 24 11.00 1.00 1.00 1.00 Crosswalk Width(ft) 16 16 16 16 16 16 16 Two way Left Turn Lane Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 Trailing Detector (ft) 100 100 20 20 20 20 20 20
Peak Hour Factor 0.92 0.93 0
Adj. Flow (vph) 560 0 0 43 0 0 Shared Lane Traffic (%) Lane Group Flow (vph) 560 0 0 43 0 0 Enter Blocked Intersection No No No No No No No No Lane Alignment Left Right Left Left Left Left Right Median Width(ft) 12 12 24 12 24 Link Offset(ft) 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 100 Turoing Speed (mph) 9 15 15 9 9 Number of Detectors 2 2 1 1 1 Detector Template Thru Thru Left Right Leading Detector (ft) 100 20 20 Trailing Detector (ft) 0 0 0 0 0 0 0 Detector 1 Size(ft) 6 6 20 20 20 20 20 20<
Shared Lane Traffic (%) Lane Group Flow (vph) 560 0 0 43 0 0 Enter Blocked Intersection No No No No No No No Lane Alignment Left Right Left Left Left Left Right Median Width(ft) 12 12 24 12 24 Link Offset(ft) 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 Two way Left Turn Lane Headway Factor 1.00 1.00 1.00 1.00 1.00 Mumber of Detectors 2 2 1 1 1 100 100 20 20 Trailing Detector (ft) 100 100 20
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Enter Blocked Intersection No No No No No No Lane Alignment Left Right Left Left Left Left Right Median Width(ft) 12 12 24 12 24 Link Offset(ft) 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 100 Two way Left Turn Lane 100 1.00 1.00 1.00 1.00 1.00 Turning Speed (mph) 9 15 15 9 Number of Detectors 2 2 1 1 Detector Template Thru Thru Left Right Leading Detector (ft) 100 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 0 0 Detector 1 Size(ft) 6 6 20 20 20 20 20 20 20 20
Lane Alignment Left Right Left Left Left Right Median Width(ft) 12 12 24 12 24 Link Offset(ft) 0 0 0 0 0 Crosswalk Width(ft) 16 16 16 16 100 Two way Left Turn Lane
Median Width(ft) 12 12 24 Link Offset(ft) 0 0 0 Crosswalk Width(ft) 16 16 16 Two way Left Turn Lane 100 1.00 1.00 1.00 1.00 Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 Turning Speed (mph) 9 15 15 9 Number of Detectors 2 2 1 1 Detector Template Thru Thru Left Right Leading Detector (ft) 100 100 20 20 Trailing Detector (ft) 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 Detector 1 Size(ft) 6 6 20 20 Detector 1 Channel Use the strend (s) 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 Detector 2 Size(ft) 6 6
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Crosswalk Width(ft) 16 16 16 Two way Left Turn Lane 1.00 <
Two way Left Turn Lane Headway Factor 1.00 1.00 1.00 1.00 1.00 Turning Speed (mph) 9 15 15 9 Number of Detectors 2 2 1 1 Detector Template Thru Thru Left Right Leading Detector (ft) 100 100 20 20 Trailing Detector (ft) 0 0 0 0 Detector 1 Position(ft) 0 0 0 0 Detector 1 Size(ft) 6 6 20 20 Detector 1 Size(ft) 6 6 20 20 Detector 1 Type CI+Ex CI+Ex CI+Ex Detector 1 Channel Detector 1 Channel 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 Detector 2 Size(ft) 6 <
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Turning Speed (mph)915159Number of Detectors2211Detector TemplateThruThruLeftRightLeading Detector (ft)1001002020Trailing Detector (ft)0000Detector 1 Position(ft)0000Detector 1 Size(ft)662020Detector 1 Size(ft)662020Detector 1 Channel00.00.00.0Detector 1 Channel00.00.00.0Detector 1 Delay (s)0.00.00.00.0Detector 2 Position(ft)949494Detector 2 Size(ft)660Detector 2 ChannelUnderstandUnderstandDetector 2 Extend (s)0.00.00.0Turn TypeNANAProtPermitted Phases624
Number of Detectors2211Detector TemplateThruThruLeftRightLeading Detector (ft)1001002020Trailing Detector (ft)0000Detector 1 Position(ft)0000Detector 1 Size(ft)662020Detector 1 Size(ft)662020Detector 1 TypeCl+ExCl+ExCl+ExCl+ExDetector 1 Channel00.00.00.0Detector 1 Queue (s)0.00.00.00.0Detector 2 Position(ft)949494Detector 2 Size(ft)660Detector 2 ChannelUnderstandUnderstandDetector 2 Extend (s)0.00.00.0Turn TypeNANAProtPermitted Phases624
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Leading Detector (ft) 100 100 20 20 Trailing Detector (ft) 0
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Detector 1 Position(ft) 0 0 0 0 Detector 1 Size(ft) 6 6 20 20 Detector 1 Size(ft) 6 6 20 20 Detector 1 Type Cl+Ex Cl+Ex Cl+Ex Cl+Ex Detector 1 Channel Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 Detector 2 Size(ft) 6 6 Detector 2 Type Cl+Ex Cl+Ex Detector 2 Extend (s) 0.0 0.0 </td
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Detector 1 Extend (s) 0.0 0.0 0.0 0.0 Detector 1 Queue (s) 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 Detector 1 Delay (s) 0.0 0.0 0.0 0.0 Detector 2 Position(ft) 94 94 94 Detector 2 Size(ft) 6 6 6 Detector 2 Type Cl+Ex Cl+Ex V Detector 2 Channel U V V Detector 2 Extend (s) 0.0 0.0 0.0 Turn Type NA NA Perm Protected Phases 6 2 4
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Detector 2 Position(ft)9494Detector 2 Size(ft)66Detector 2 TypeCI+ExDetector 2 Channel0.0Detector 2 Extend (s)0.0Turn TypeNAProtected Phases624Permitted Phases4
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Detector 2 TypeCI+ExCI+ExDetector 2 Channel0.00.0Detector 2 Extend (s)0.00.0Turn TypeNANAProtProtected Phases624Permitted Phases4
Detector 2 ChannelDetector 2 Extend (s)0.0Turn TypeNANAProtected Phases62Permitted Phases4
Detector 2 Extend (s)0.00.0Turn TypeNANAProtPermProtected Phases624Permitted Phases4
Turn TypeNANAProtPermProtected Phases624Permitted Phases4
Protected Phases 6 2 4 Permitted Phases 4
Permitted Phases 4
Detector Phase 6 2 4 4
Switch Phase
Minimum Initial (s) 5.0 5.0 5.0 5.0

05/26/2021

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	22.5			22.5	22.5	22.5
Total Split (s)	22.5			22.5	22.5	22.5
Total Split (%)	50.0%			50.0%	50.0%	50.0%
Maximum Green (s)	18.0			18.0	18.0	18.0
Yellow Time (s)	3.5			3.5	3.5	3.5
All-Red Time (s)	1.0			1.0	1.0	1.0
Lost Time Adjust (s)	0.0			0.0	0.0	0.0
Total Lost Time (s)	4.5			4.5	4.5	4.5
Lead/Lag	4.5			4.5	4.5	4.5
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0			3.0	3.0	3.0
Recall Mode	C-Max				None	None
				C-Max		
Walk Time (s)	7.0			7.0	7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	45.0			45.0		
Actuated g/C Ratio	1.00			1.00		
v/c Ratio	0.16			0.01		
Control Delay	0.1			0.0		
Queue Delay	0.0			0.0		
Total Delay	0.1			0.0		
LOS	А			А		
Approach Delay	0.1					
Approach LOS	А					
Queue Length 50th (ft)	0			0		
Queue Length 95th (ft)	0			0		
Internal Link Dist (ft)	843			453	420	
Turn Bay Length (ft)						
Base Capacity (vph)	3539			3539		
Starvation Cap Reductn	0			0		
Spillback Cap Reductn	0			0		
Storage Cap Reductn	0			0		
Reduced v/c Ratio	0.16			0.01		
Intersection Summary						
Area Type:	Other					
Cycle Length: 45	ULICI					
Actuated Cycle Length: 45	to phase 2.1	NDT and		Start of C	roon	
Offset: 0 (0%), Referenced	to phase 2:	WBI and	0:EBT, 3	Start of G	reen	
Natural Cycle: 45						
Control Type: Actuated-Coo	ordinated					
Maximum v/c Ratio: 0.16						
						100
Intersection Signal Delay: 0					ntersectio	
Intersection Signal Delay: 0 Intersection Capacity Utiliza Analysis Period (min) 15						n LOS: A of Service

Splits and Phases: 103:

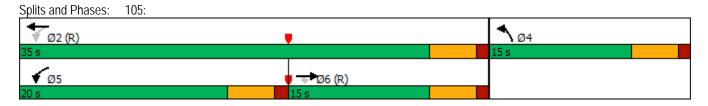
← Ø2 (R)	▲ Ø4	
22.5 s	22.5 s	
₩ 22.5 s		
22.5 s		

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	A1⊅		ľ	<u></u>	ľ	1
Traffic Volume (vph)	144	49	20	40	0	0
Future Volume (vph)	544	49	20	40	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	0.95	1.00	0.95	1.00	1.00
Frt	0.988					
Flt Protected			0.950			
Satd. Flow (prot)	3497	0	1770	3539	1863	1863
Flt Permitted			0.950			
Satd. Flow (perm)	3497	0	1770	3539	1863	1863
Link Speed (mph)	30			30	30	
Link Distance (ft)	533			404	428	
Travel Time (s)	12.1			9.2	9.7	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	591	53	22	43	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	644	0	22	43	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	24			24	20	
Link Offset(ft)	-12			8	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Stop			Stop	Stop	
Intersection Summary						
Area Type: 0	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	tion 15.5%			IC	U Level o	of Service
Analysis Dariad (min) 15						

	-	\mathbf{r}	4	-	1	۲
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>	1	5	1	٦Y	NDR
Traffic Volume (vph)	87	57	544	39	22	19
Future Volume (vph)	87	457	844	39	22	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	0.95
Frt	0.75	0.850	1.00	0.75	0.97	0.75
Flt Protected		0.050	0.950		0.930	
Satd. Flow (prot)	3539	1583	1770	3539	3273	0
Flt Permitted	3337	1505	0.490	3337	0.974	0
Satd. Flow (perm)	3539	1583	0.490 913	3539	3273	0
	2028	Yes	913	2028	3273	Yes
Right Turn on Red					01	res
Satd. Flow (RTOR)	٦Г	487		٦F	21 25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	491			971	1149	
Travel Time (s)	9.6	0.00	0.00	18.9	31.3	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	95	497	917	42	24	21
Shared Lane Traffic (%)						
Lane Group Flow (vph)	95	497	917	42	45	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	24			24	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Number of Detectors	2	1	1	2	1	
Detector Template	Thru	Right	Left	Thru	Left	
Leading Detector (ft)	100	20	20	100	20	
Trailing Detector (ft)	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	
Detector 1 Size(ft)	6	20	20	6	20	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)	94	0.0	0.0	94	0.0	
Detector 2 Size(ft)	6 Ch Ex			6 CL-Ex		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel	0.0			0.0		
Detector 2 Extend (s)	0.0	Dec	pre-st	0.0	Durch	
Turn Type	NA	Perm	pm+pt	NA	Prot	
Protected Phases	6		5	2	4	
Permitted Phases		6	2			
Detector Phase	6	6	5	2	4	
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	

	-	\mathbf{r}	4	-	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	20.0	20.0	9.5	22.5	15.0	
Total Split (s)	15.0	15.0	20.0	35.0	15.0	
Total Split (%)	30.0%	30.0%	40.0%	70.0%	30.0%	
Maximum Green (s)	10.5	10.5	15.5	30.5	10.5	
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max	None	C-Max	None	
Walk Time (s)	7.0	7.0				
Flash Dont Walk (s)	11.0	11.0				
Pedestrian Calls (#/hr)	0	0				
Act Effct Green (s)	13.6	13.6	41.1	43.8	6.0	
Actuated g/C Ratio	0.27	0.27	0.82	0.88	0.12	
v/c Ratio	0.10	0.63	0.80	0.00	0.12	
Control Delay	14.6	7.0	13.3	1.5	13.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	14.6	7.0	13.3	1.5	13.9	
LOS	B	7.0 A	В	A	В	
Approach Delay	8.2	~		12.8	13.9	
Approach LOS	A			12.0 B	В	
Queue Length 50th (ft)	8	2	0	0	3	
Queue Length 95th (ft)	26	#73	#352	m2	14	
Internal Link Dist (ft)	411	"13	" 3 02	891	1069	
Turn Bay Length (ft)				571	1007	
Base Capacity (vph)	964	785	1143	3099	703	
Starvation Cap Reductn	904 0	0	0	0	0	
Spillback Cap Reductin	0	0	0	0	0	
Storage Cap Reductin	0	0	0	0	0	
Reduced v/c Ratio	0.10	0.63	0.80	0.01	0.06	
	0.10	0.05	0.00	0.01	0.00	
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50						
Offset: 0 (0%), Referenced	to phase 2	:WBTL ar	nd 6:EBT,	Start of (Green	
Natural Cycle: 65						
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.80						
Intersection Signal Delay: 1	1.1			lr	ntersectior	n LOS: B
Intersection Capacity Utilization)		[(CU Level (of Service A
Analysis Period (min) 15						
# 95th percentile volume	exceeds ca	ipacity, qu	Leue may	be longe	er.	
Queue shown is maxim			,	J		
m Volume for 95th percer			d by upst	ream sigr	nal.	
				J		

Lanes, Volumes, Timings 105:



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		††			† †			•			•	
Traffic Volume (vph)	0	99	7	150	582	15	0	0	4	0	0	0
Future Volume (vph)	0	99	7	150	882	15	0	0	4	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.990			0.998			0.865				
Flt Protected					0.993							
Satd. Flow (prot)	0	3504	0	0	3507	0	0	1611	0	0	1863	0
Flt Permitted					0.993							
Satd. Flow (perm)	0	3504	0	0	3507	0	0	1611	0	0	1863	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		971			260			262			305	
Travel Time (s)		22.1			5.9			6.0			6.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	108	8	163	959	16	0	0	4	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	116	0	0	1138	0	0	4	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		16			16			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
51)ther											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 34.2%			IC	CU Level	of Service	A					

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>			 ↑Ъ		
Traffic Volume (vph)	92	10	9	540	207	4
Future Volume (vph)	92	10	9 109	790	207	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	1700	14	1300	1300	1300	1700
Lane Util. Factor	0.95	1.00	0.95	0.95	1.00	1.00
Frt	0.95	0.850	0.95	0.95	1.00	0.850
Fit Protected		0.000		0.994	0.950	0.650
Satd. Flow (prot)	3539	1689	0	3518	1770	1583
Flt Permitted	2029	1009	0	0.901	0.950	1000
	25.20	1400	0			1500
Satd. Flow (perm)	3539	1689	0	3189	1770	1583
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	- 00	11		- 0.0		4
Link Speed (mph)	30			30	30	
Link Distance (ft)	777			738	307	
Travel Time (s)	17.7			16.8	7.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	100	11	118	859	279	4
Shared Lane Traffic (%)						
Lane Group Flow (vph)	100	11	0	977	279	4
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	Left	Right
Median Width(ft)	16			16	36	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	0.92	1.00	1.00	1.00	1.00
Turning Speed (mph)		15	15		15	9
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (ft)	100	20	20	100	20	20
Trailing Detector (ft)	0	20	20	0	20	0
Detector 1 Position(ft)	0		0		0	0
()		0		0		
Detector 1 Size(ft)	6	20	20	6	20	20
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	6		5	2	4	
Permitted Phases		6	2			4
Detector Phase	6	6	5	2	4	4
Switch Phase	U	0	0	2		

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	9.5	22.5	22.5	22.5
Total Split (s)	20.0	20.0	8.5	28.5	21.5	21.5
Total Split (%)	40.0%	40.0%	17.0%	57.0%	43.0%	43.0%
Maximum Green (s)	15.5	15.5	4.0	24.0	17.0	17.0
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	1.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5
Lead/Lag	Lag	Lag	Lead	т.5	1.0	1.0
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None
Walk Time (s)	7.0	7.0	NULLE	C-IVIAX 7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0		11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0		0	0	11.0 0
. ,	28.2	28.2		28.2	12.8	12.8
Act Effct Green (s)		28.2 0.56		28.2 0.56		0.26
Actuated g/C Ratio	0.56				0.26	
v/c Ratio	0.05	0.01		0.54	0.62	0.01
Control Delay	0.9	0.1		10.7	21.9	8.2
Queue Delay	0.0	0.0		0.0	0.0	0.0
Total Delay	0.9	0.1		10.7	21.9	8.2
LOS Approach Delay	A	А		B	C	А
Approach Delay	0.8			10.7	21.7	
Approach LOS	A	0		B	C	
Queue Length 50th (ft)	1	0		93	71	0
Queue Length 95th (ft)	1	0		m137	117	5
Internal Link Dist (ft)	697			658	227	
Turn Bay Length (ft)		<u> </u>		4		
Base Capacity (vph)	1994	956		1797	601	540
Starvation Cap Reductn	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0
Reduced v/c Ratio	0.05	0.01		0.54	0.46	0.01
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50						
Offset: 0 (0%), Referenced	to phase 2	:WBTL ar	nd 6:EBT,	Start of (Green	
Natural Cycle: 55						
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.62						
Intersection Signal Delay: 1	12.2			Ir	ntersectio	n LOS: B
Intersection Capacity Utilization)		10	CU Level	of Service
Analysis Period (min) 15						
m Volume for 95th percer	ntile queue	is metere	d by upst	ream sigr	nal.	

Lanes, Volumes, Timings 107:

 Splits and Phases:
 107:

 ✓ Ø2 (R)
 ✓ Ø4

 28.5 s
 21.5 s

 ✓ Ø5
 ✓ Ø6 (R)

 8.5 s
 20 s

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	<u>۲</u> ۲					
Traffic Volume (vph)	"ו "ד 72	24	1 317	TT 999	TT 357	r 232
Future Volume (vph)	72	24	417	999	357	482
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	402
Lane Util. Factor	0.97	0.95	1.00	0.95	0.95	1.00
Frt	0.97	0.90	1.00	0.90	0.70	0.850
Fit Protected	0.962		0.950			0.000
	0.964	0	0.950	3539	3539	1583
Satd. Flow (prot)	0.964	U		3039	3037	1083
Flt Permitted		0	0.405 754	2520	2520	1500
Satd. Flow (perm)	3351	0 Voc	754	3539	3539	1583 Voc
Right Turn on Red	2/	Yes				Yes
Satd. Flow (RTOR)	26			20	20	524
Link Speed (mph)	30			30	30	
Link Distance (ft)	738			727	965	
Travel Time (s)	16.8	0.00	0.00	16.5	21.9	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	78	26	453	1086	388	524
Shared Lane Traffic (%)						
Lane Group Flow (vph)	104	0	453	1086	388	524
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	36			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Turn Type	Prot		pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	
Permitted Phases	·		2	_	-	6
Minimum Split (s)	22.5		9.5	22.5	22.5	22.5
Total Split (s)	21.5		8.5	28.5	20.0	20.0
Total Split (%)	43.0%		17.0%	57.0%	40.0%	40.0%
Maximum Green (s)	43.078		4.0	24.0	15.5	15.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
· · /						
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag			Lead		Lag	Lag
Lead-Lag Optimize?			Yes		Yes	Yes
Walk Time (s)	7.0			7.0	7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	17.0		24.0	24.0	15.5	15.5
Actuated g/C Ratio	0.34		0.48	0.48	0.31	0.31
v/c Ratio	0.09		1.02	0.64	0.35	0.62
Control Delay	6.8		66.4	11.9	14.5	5.3
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	6.8		66.4	11.9	14.5	5.3

Synchro 11 Report Page 16

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
LOS	А		E	В	В	А
Approach Delay	6.8			28.0	9.2	
Approach LOS	А			С	А	
Queue Length 50th (ft)	12		~88	114	45	0
Queue Length 95th (ft)	27		#259	167	74	55
Internal Link Dist (ft)	658			647	885	
Turn Bay Length (ft)						
Base Capacity (vph)	1156		443	1698	1097	852
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.09		1.02	0.64	0.35	0.62
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50						
Offset: 0 (0%), Referenced	I to phase 2:I	NBTL and	d 6:SBT,	Start of G	reen, Mas	ster Inters
Natural Cycle: 60						
Control Type: Pretimed						
Maximum v/c Ratio: 1.02						
Intersection Signal Delay: 2				In	tersectior	LOS: C
Intersection Capacity Utiliz	ation 42.8%			IC	U Level o	of Service
Analysis Period (min) 15						
 Volume exceeds capac 	city, queue is	theoretic	cally infini	te.		
Queue shown is maxim						
# 95th percentile volume	exceeds cap	pacity, qu	ieue may	be longer	r.	
Queue shown is maxim	um after two	cycles.	<u> </u>	Ū.		
Splits and Phases: 108:						

1 Ø2 (R)		<u>∕</u> ≉ _{Ø4}	
28.5 s		21.5 s	
▲ ø5	Ø6 (R)		
8.5 s	20 s		

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Lane Group	NBT	NBR	SBL	SBT	NWL	NWR	
Lane Configurations	¢Î			•			
Traffic Volume (vph)	211	0	0	19	0	0	
Future Volume (vph)	261	0	0	119	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	1863	0	0	1863	0	0	
Flt Permitted							
Satd. Flow (perm)	1863	0	0	1863	0	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	1082			1015	590		
Travel Time (s)	24.6			23.1	13.4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	284	0	0	129	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	284	0	0	129	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	0		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion 14.4%			IC	CU Level (of Service	A
Analysis Period (min) 15						5. 5011100	

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	•			•	¥	
Traffic Volume (vph)	28	0	0	19	0	183
Future Volume (vph)	28	0	0	119	200	233
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.927	
Flt Protected					0.977	
Satd. Flow (prot)	1863	0	0	1863	1687	0
Flt Permitted					0.977	
Satd. Flow (perm)	1863	0	0	1863	1687	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	839			634	538	
Travel Time (s)	19.1			14.4	12.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	30	0	0	129	217	253
Shared Lane Traffic (%)						
Lane Group Flow (vph)	30	0	0	129	470	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 21.3%			IC	CU Level d	of Service A
Analysis Poriod (min) 15						

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्भ	et 🗧		ľ	1
Traffic Volume (vph)	0	0	0	19	28	0
Future Volume (vph)	0	0	0	319	28	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.865			
Flt Protected					0.950	
Satd. Flow (prot)	0	1863	1611	0	1770	1863
Flt Permitted					0.950	
Satd. Flow (perm)	0	1863	1611	0	1770	1863
Link Speed (mph)		30	30		30	
Link Distance (ft)		98	839		286	
Travel Time (s)		2.2	19.1		6.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	347	30	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	347	0	30	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		36	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Stop		Stop	
Intersection Summary						
51	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 13.3%			IC	CU Level o	of Service

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	∱1 ≱			<u></u>		1	
Traffic Volume (vph)	214	20	0	359	70	35	
Future Volume (vph)	214	20	0	759	70	35	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	0.95	0.95	1.00	0.95	1.00	1.00	
Frt	0.987					0.865	
Flt Protected					0.950		
Satd. Flow (prot)	3493	0	0	3539	0	1611	
Flt Permitted					0.950		
Satd. Flow (perm)	3493	0	0	3539	0	1611	
Link Speed (mph)	30			30	30		
Link Distance (ft)	404			491	211		
Travel Time (s)	9.2			11.2	4.8		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	233	22	0	825	76	38	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	255	0	0	825	76	38	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Right	
Median Width(ft)	12			24	0		
Link Offset(ft)	0			6	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion Err%			IC	CU Level	of Service	θH
Analysis Period (min) 15							

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Lane Group	SBL	SBR	NEL	NET	SWT	SWR	
Lane Configurations		77	ሻሻ	•			
Traffic Volume (vph)	0	827	205	0	0	0	
Future Volume (vph)	0	1227	205	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	0.88	0.97	1.00	1.00	1.00	
Frt		0.850					
Flt Protected			0.950				
Satd. Flow (prot)	0	2787	3433	1863	0	0	
Flt Permitted			0.950				
Satd. Flow (perm)	0	2787	3433	1863	0	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	227			1186	549		
Travel Time (s)	5.2			27.0	12.5		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	1334	223	0	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	1334	223	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	12			36	36		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15	9	15			9	
Sign Control	Free			Free	Free		
Intersection Summary							
51	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	tion 32.3%			IC	U Level o	of Service A	А
Analysis Dariad (min) 15							

	_#	7	•	×	*	~		
Lane Group	EBL	EBR	NEL	NET	SWT	SWR		
Lane Configurations				<u></u>	<u></u>	1		
Traffic Volume (vph)	0	0	0	205	342	485		
Future Volume (vph)	0	0	0	205	742	485		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.91	1.00		
Frt						0.850		
Flt Protected								
Satd. Flow (prot)	0	0	0	3539	5085	1583		
Flt Permitted								
Satd. Flow (perm)	0	0	0	3539	5085	1583		
Link Speed (mph)	30			30	30			
Link Distance (ft)	1042			233	359			
Travel Time (s)	23.7			5.3	8.2			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0	0	0	223	807	527		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	0	0	223	807	527		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Left	Left	Right		
Median Width(ft)	8			0	24			
Link Offset(ft)	0			0	0			
Crosswalk Width(ft)	16			16	16			
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15	9	15			9		
Sign Control	Free			Free	Free			
Intersection Summary								
Area Type:	Other							
Control Type: Unsignalized								
Intersection Capacity Utiliza	tion 33.4%			IC	CU Level	of Service	А	
Analysis Doriod (min) 15								

	L.	J.	~	*	•	~
Lane Group	SBL	SBR	NWL	NWR	NEL	NER
Lane Configurations				7	ካካ	<u>NLR</u>
Traffic Volume (vph)	• 1 0	6 6 827	0	r	205	.
Future Volume (vph)	0	1227	0	0	205	0
Ideal Flow (vphpl)	1900	1227	1900	1900	1900	1900
Lane Util. Factor	1.00	0.88	1.00	1.00	0.97	1.00
Frt	1.00	0.88	1.00	1.00	0.97	1.00
		0.800			0.950	
Flt Protected	10/0	7707	10/0	10/0		10/0
Satd. Flow (prot)	1863	2787	1863	1863	3433	1863
Flt Permitted	10/0	0707	10/0	10/0	0.950	10/0
Satd. Flow (perm)	1863	2787	1863	1863	3433	1863
Right Turn on Red		Yes		Yes		Yes
Satd. Flow (RTOR)		1920				
Link Speed (mph)	30		30		30	
Link Distance (ft)	763		723		430	
Travel Time (s)	17.3		16.4		9.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	1334	0	0	223	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	1334	0	0	223	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	30	J	32	3	32	3
Link Offset(ft)	30		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane	10		10		10	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	9	1.00	9	1.00	9
Number of Detectors	0	9	0	9	0	9
Detector Template						Thru
	Thru	Thru	Thru	Thru	Thru	
Leading Detector (ft)	0	0	0	0	0	0
Trailing Detector (ft)	0	0	0	0	0	0
Turn Type	Prot	pt+ov	Prot	Perm	Prot	Perm
Protected Phases	5	56	4		6	
Permitted Phases				4		6
Detector Phase	5	56	4	4	6	6
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	22.5		22.5	22.5	22.5	22.5
Total Split (s)	15.5		15.5	15.5	19.0	19.0
Total Split (%)	31.0%		31.0%	31.0%	38.0%	38.0%
Maximum Green (s)	11.0		11.0	11.0	14.5	14.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag	Lead		4.0	4.0		
					Lag	Lag
Lead-Lag Optimize?	Yes		2.0	2.0	Yes	Yes
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	None		None	None	C-Max	C-Max

	L.	J.	-	*	•	~		
Lane Group	SBL	SBR	NWL	NWR	NEL	NER		
Walk Time (s)	7.0		7.0	7.0	7.0	7.0		
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	11.0		
Pedestrian Calls (#/hr)	0		0	0	0	0		
Act Effct Green (s)		50.0			33.1			
Actuated g/C Ratio		1.00			0.66			
v/c Ratio		0.48			0.10			
Control Delay		1.2			3.4			
Queue Delay		0.0			0.0			
Total Delay		1.2			3.4			
LOS		А			А			
Approach Delay	1.2				3.4			
Approach LOS	А				А			
Queue Length 50th (ft)		4			8			
Queue Length 95th (ft)		0			19			
Internal Link Dist (ft)	683		643		350			
Turn Bay Length (ft)								
Base Capacity (vph)		2762			2272			
Starvation Cap Reductn		0			0			
Spillback Cap Reductn		0			0			
Storage Cap Reductn		0			0			
Reduced v/c Ratio		0.48			0.10			
Intersection Summary								
Area Type:	Other							
Cycle Length: 50								
Actuated Cycle Length: 50								
Offset: 0 (0%), Referenced		VEL, Sta	t of Gree	n				
Natural Cycle: 70	·· [····	1						
Control Type: Actuated-Co	ordinated							
Maximum v/c Ratio: 0.48								
Intersection Signal Delay:	1.5			In	tersection	LOS: A		
Intersection Capacity Utiliz						of Service A	۹	
Analysis Period (min) 15								
Splits and Phases: 102:								
Ĺ								

A Ø5	🚽 🤽 Ø6 (R)	▶ Ø4
15.5 s	19 s	15.5 s

	-	\mathbf{r}	4	-	1	۲
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>			***	<u>ነ</u> ካ	
Traffic Volume (vph)	205	0	0	TT 429	399	29
Future Volume (vph)	205	0	0	429 829	399	29 29
	1900		1900	829 1900		29 1900
Ideal Flow (vphpl)		1900			1900	
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	1.00
Frt					0.050	0.850
Flt Protected					0.950	
Satd. Flow (prot)	3539	0	0	3539	3433	1583
Flt Permitted					0.950	
Satd. Flow (perm)	3539	0	0	3539	3433	1583
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)						32
Link Speed (mph)	30			30	30	
Link Distance (ft)	923			533	500	
Travel Time (s)	21.0			12.1	11.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	223	0.72	0.72	901	434	32
Shared Lane Traffic (%)	225	U	U	701	434	JZ
Lane Group Flow (vph)	223	0	0	001	101	32
		0	0	901	434	
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Number of Detectors	2			2	1	1
Detector Template	Thru			Thru	Left	Right
Leading Detector (ft)	100			100	20	20
Trailing Detector (ft)	0			0	0	0
Detector 1 Position(ft)	0			0	0	0
Detector 1 Size(ft)	6			6	20	20
. ,						
Detector 1 Type	CI+Ex			CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel	0.0			0.0	0.0	0.0
Detector 1 Extend (s)	0.0			0.0	0.0	0.0
Detector 1 Queue (s)	0.0			0.0	0.0	0.0
Detector 1 Delay (s)	0.0			0.0	0.0	0.0
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA			NA	Prot	Perm
Protected Phases	6			2	4	
Permitted Phases	0			2		4
Detector Phase	6			2	4	4
	U			Z	4	4
Switch Phase	F 0			F 0	F 0	F 0
Minimum Initial (s)	5.0			5.0	5.0	5.0

	-	\mathbf{i}	4	-	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	22.5	2311		22.5	22.5	22.5
Total Split (s)	25.0			25.0	25.0	25.0
Total Split (%)	50.0%			50.0%	50.0%	50.0%
Maximum Green (s)	20.5			20.5	20.5	20.5
Yellow Time (s)	3.5			3.5	3.5	3.5
All-Red Time (s)	1.0			1.0	1.0	1.0
Lost Time Adjust (s)	0.0			0.0	0.0	0.0
Total Lost Time (s)	4.5			4.5	4.5	4.5
Lead/Lag	4.5			4.5	4.5	4.5
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0			3.0	3.0	3.0
Recall Mode	C-Max			C-Max	None	None
				C-IVIAX 7.0		
Walk Time (s)	7.0				7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	29.3			29.3	11.7	11.7
Actuated g/C Ratio	0.59			0.59	0.23	0.23
v/c Ratio	0.11			0.43	0.54	0.08
Control Delay	4.6			7.1	18.9	6.3
Queue Delay	0.0			0.0	0.0	0.0
Total Delay	4.6			7.1	18.9	6.3
LOS	А			А	В	А
Approach Delay	4.6			7.1	18.0	
Approach LOS	А			А	В	
Queue Length 50th (ft)	12			64	57	0
Queue Length 95th (ft)	20			121	81	14
Internal Link Dist (ft)	843			453	420	
Turn Bay Length (ft)						
Base Capacity (vph)	2072			2072	1407	667
Starvation Cap Reductn	0			0	0	0
Spillback Cap Reductn	0			0	0	0
Storage Cap Reductn	0			0	0	0
Reduced v/c Ratio	0.11			0.43	0.31	0.05
	0.11			0.43	0.31	0.05
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50)					
Offset: 0 (0%), Referenced		NBT and	6:EBT.	Start of G	reen	
Natural Cycle: 45			012017			
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.54	Joraniatou					
Intersection Signal Delay:	10.0			lr	ntersectio	n I OS· A
Intersection Capacity Utiliz					CU Level	
Analysis Period (min) 15						
Analysis renou (IIIII) 13						

Splits and Phases: 103:

← Ø2 (R)	▲ Ø4
25 s	25 s
I → Ø6 (R)	
25 s	

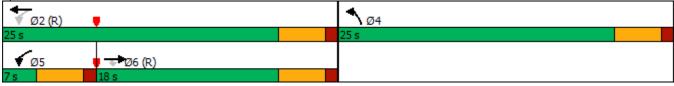
-	\mathbf{r}	1	-	1	1
EBT	EBR	WBL	WBT	NBL	NBR
≜1 ≱		٦	<u></u>	٦	1
234	0	0	429	0	0
234	0	0	829	0	0
1900	1900	1900	1900	1900	1900
0.95	0.95	1.00	0.95	1.00	1.00
3539	0	1863	3539	1863	1863
3539	0	1863	3539	1863	1863
30			30	30	
	0.92	0.92		0.92	0.92
254	0	0	901	0	0
	0	0	901	0	0
					No
	Right	Left			Right
16			16	16	
1.00	1.00		1.00	1.00	1.00
	9	15		15	9
Stop			Stop	Stop	
Other					
ion 15.2%			IC	U Level o	of Service A
	 ↑↑ 234 234 234 234 1900 0.95 3539 3539 30 533 12.1 0.92 254 254 No Left 24 -12 16 1.00 Stop Dther 	1900 1900 234 0 1900 1900 0.95 0.95 3539 0 3539 0 30 533 12.1 0.92 0.92 0.92 254 0 No No Left Right 24 -12 16 1.00 9 Stop Dther Dther	1 1 234 0 0 234 0 0 1900 1900 1900 1900 1900 1900 0.95 0.95 1.00 3539 0 1863 30 533 12.1 0.92 0.92 0.92 254 0 0 254 0 0 254 0 0 254 0 0 254 0 0 100 No No 11.00 1.00 1.00 9 15 Stop Dther 0 0	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	1 1 1 1 234 0 0 429 0 234 0 0 829 0 1900 1900 1900 1900 1900 0.95 0.95 1.00 0.95 1.00 3539 0 1863 3539 1863 30 30 30 30 30 533 404 428 12.1 9.2 9.7 0.92 0.92 0.92 0.92 254 0 0 901 0 254 0 0 901 0 254 0 0 901 0 254 0 0 901 0 254 0 0 901 0 0 100 No No No 16 16 16 16 16 1.00 1.00 1.00 1.00 1.00 9 15 15 15 Stop top </td

	-	\mathbf{F}	4	+	•	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	1	<u> </u>	† †	٦Y	- HDR
Traffic Volume (vph)	11 237	12	57	TT 92	267	670
Future Volume (vph)	237	12	57	92 92	667	970
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	1.00	1.00	0.95	0.97	0.95
Frt	0.75	0.850	1.00	0.75	0.97	0.75
Flt Protected		0.030	0.950		0.980	
Satd. Flow (prot)	3539	1583	1770	3539	3226	0
Flt Permitted	3337	1000	0.452	5557	0.980	0
Satd. Flow (perm)	3539	1583	842	3539	3226	0
Right Turn on Red	3337	Yes	042	3337	5220	Yes
Satd. Flow (RTOR)		13			437	162
	35	13		35	437	
Link Speed (mph)				35 971		
Link Distance (ft)	491				1149 21.2	
Travel Time (s)	9.6	0.00	0.00	18.9	31.3	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	258	13	62	100	725	1054
Shared Lane Traffic (%)	0.7.0			465	4775	-
Lane Group Flow (vph)	258	13	62	100	1779	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	24			24	24	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Number of Detectors	2	1	1	2	1	
Detector Template	Thru	Right	Left	Thru	Left	
Leading Detector (ft)	100	20	20	100	20	
Trailing Detector (ft)	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	
Detector 1 Size(ft)	6	20	20	6	20	
Detector 1 Type					CI+Ex	
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)	94	0.0	0.0	94	0.0	
Detector 2 Size(ft)	6			6		
Detector 2 Type	CI+Ex			CI+Ex		
Detector 2 Channel	ΟITLA					
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	pm+pt	NA	Prot	
Protected Phases		Penn	ртт+рт 5	NA 2		
	6	L		Z	4	
Permitted Phases	1	6	2 5	ſ	4	
Detector Phase	6	6	5	2	4	
Switch Phase		F 0	F 0	F 0	F 0	
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	

	-	\mathbf{r}	4	-	1	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Split (s)	15.0	15.0	9.5	22.5	15.0	
Total Split (s)	18.0	18.0	7.0	25.0	25.0	
Total Split (%)	36.0%	36.0%	14.0%	50.0%	50.0%	
Maximum Green (s)	13.5	13.5	2.5	20.5	20.5	
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	
Lead/Lag	Lag	Lag	Lead	1.0	1.0	
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max	None	C-Max	None	
Walk Time (s)	7.0	7.0	NOTIC		NONC	
Flash Dont Walk (s)	11.0	11.0				
Pedestrian Calls (#/hr)	0	0				
Act Effct Green (s)	16.3	16.3	20.5	20.5	20.5	
Actuated g/C Ratio	0.33	0.33	0.41	0.41	0.41	
v/c Ratio	0.33	0.03	0.41	0.41	1.15dr	
Control Delay	14.1	8.0	6.5	5.7	80.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	14.1	8.0	6.5	5.7	80.9	
LOS	14.1 B	0.0 A	0.5 A	0.7 A	60.9 F	
Approach Delay	13.8	Л	Л	6.0	80.9	
Approach LOS	13.8 B			0.0 A	60.9 F	
Queue Length 50th (ft)	31	0	9	7	~282	
Queue Length 95th (ft)	55	10	18	13	~202 #403	
Internal Link Dist (ft)	411	10	10	891	#403 1069	
Turn Bay Length (ft)	411			071	1009	
Base Capacity (vph)	1153	524	391	1450	1580	
Starvation Cap Reductn	0	524 0	391		1580	
Spillback Cap Reductin	0	0	0	0	0	
				0		
Storage Cap Reductn Reduced v/c Ratio	0 0.22	0 0.02	0 0.16	0.07	0 1.13	
Reduced V/C Rallo	0.22	0.02	0.10	0.07	1.13	
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50						
Offset: 0 (0%), Referenced	to phase 2	:WBTL ar	nd 6:EBT	Start of (Green	
Natural Cycle: 60	F Fridoo L					
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 1.13						
Intersection Signal Delay: 6	67.2			lr	ntersection	n LOS: F
Intersection Capacity Utiliz						of Service A
Analysis Period (min) 15	2					00/1100/1
 Volume exceeds capac 	city, queue i	s theoreti	cally infin	ite.		
Queue shown is maxim						
# 95th percentile volume			IELIE may	he longe	۰r	
Queue shown is maxim			acue may	be longe	/1	
		s cycles.				

dr Defacto Right Lane. Recode with 1 though lane as a right lane.

Splits and Phases: 105:



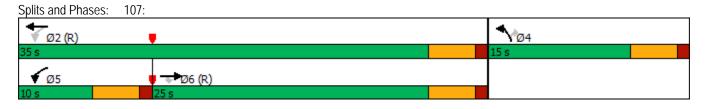
	≯	-	\mathbf{F}	•	+	*	•	Ť	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<u>†</u> †			† †			•			†	
Traffic Volume (vph)	0	907	0	2	119	2	27	0	17	0	0	4
Future Volume (vph)	0	1207	0	2	119	2	27	0	17	0	0	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	0.95	1.00	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.998			0.948			0.865	
Flt Protected					0.999			0.970				
Satd. Flow (prot)	0	3539	0	0	3529	0	0	1713	0	0	1611	0
Flt Permitted					0.999			0.970				
Satd. Flow (perm)	0	3539	0	0	3529	0	0	1713	0	0	1611	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		971			260			262			305	
Travel Time (s)		22.1			5.9			6.0			6.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	1312	0	2	129	2	29	0	18	0	0	4
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1312	0	0	133	0	0	47	0	0	4	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		16			16			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
J 1)ther											
Control Type: Unsignalized												
Intersection Capacity Utilizati	on 40.9%			IC	CU Level of	of Service	A					

	-	\mathbf{i}	4	+	•	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>		VDL	 ↑Ъ		
Traffic Volume (vph)	TT 567	357	4	H T 122	0	r 19
Future Volume (vph)	767	457	4	122	0	119
Ideal Flow (vphpl)	1900	1900	4	1900	1900	1900
Lane Width (ft)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.95	1.00	0.95	0.95	1.00	1.00
Frt	0.75	0.850	0.75	0.75	1.00	0.850
Fit Protected		0.000		0.999		0.000
Satd. Flow (prot)	3539	1689	0	3536	1863	1583
Flt Permitted	2029	1009	0	0.941	1003	1000
	25.20	1400	0		1040	1500
Satd. Flow (perm)	3539	1689	0	3330	1863	1583
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	05	497		05	25	288
Link Speed (mph)	35			35	35	
Link Distance (ft)	777			738	307	
Travel Time (s)	15.1			14.4	6.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	834	497	4	133	0	129
Shared Lane Traffic (%)						
Lane Group Flow (vph)	834	497	0	137	0	129
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	R NA	Left	Left	Left	Right
Median Width(ft)	16			16	36	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	0.92	1.00	1.00	1.00	1.00
Turning Speed (mph)		15	15		15	9
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (ft)	100	20	20	100	20	20
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	6	20	20	6	20	20
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex
Detector 1 Channel	5 EX	5., EA	5., EA	5.7 EA	5 EA	5.7 EA
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)	94	0.0	0.0	94	0.0	0.0
Detector 2 Size(ft)	94			94		
	o CI+Ex			o CI+Ex		
Detector 2 Type	CI+EX			CI+EX		
Detector 2 Channel	0.0			0.0		
Detector 2 Extend (s)	0.0	Dem		0.0	Deet	Dem
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm
Protected Phases	6	,	5	2	4	4
Permitted Phases		6	2			4
Detector Phase	6	6	5	2	4	4
Switch Phase						

	-	\mathbf{r}	4	+	•	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	9.5	22.5	22.5	22.5
Total Split (s)	25.0	25.0	10.0	35.0	15.0	15.0
Total Split (%)	50.0%	50.0%	20.0%	70.0%	30.0%	30.0%
Maximum Green (s)	20.5	20.5	5.5	30.5	10.5	10.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	C-Max	C-Max	None	C-Max	None	None
Walk Time (s)	7.0	7.0		7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0		11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0		0	0	0
Act Effct Green (s)	38.4	38.4		38.4	Ū	5.5
Actuated g/C Ratio	0.77	0.77		0.77		0.11
v/c Ratio	0.31	0.35		0.05		0.30
Control Delay	3.9	1.7		2.1		1.8
Queue Delay	0.0	0.0		0.0		0.0
Total Delay	3.9	1.7		2.1		1.8
LOS	A	A		A		A
Approach Delay	3.1			2.1	1.8	73
Approach LOS	A			A	A	
Queue Length 50th (ft)	56	13		4		0
Queue Length 95th (ft)	m55	m14		9		0
Internal Link Dist (ft)	697			658	227	U
Turn Bay Length (ft)	071			000	~~ 1	
Base Capacity (vph)	2717	1412		2557		559
Starvation Cap Reductn	0	0		0		0
Spillback Cap Reductn	0	0		0		0
Storage Cap Reductn	0	0		0		0
Reduced v/c Ratio	0.31	0.35		0.05		0.23
Intersection Summary	0.51	0.00		0.00		0.23
Area Type:	Other					
Cycle Length: 50	Utilei					
Actuated Cycle Length: 50						
Offset: 0 (0%), Referenced			d 6.EDT	Start of (Groop	
Natural Cycle: 55	no priase z	.vvdil di	UU.EDI,	Start OF	JIEEH	
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.35	orundleu					
Intersection Signal Delay: 2	2.0				ntersection	
Intersection Capacity Utiliz					CU Level	
Analysis Period (min) 15	aliun 33.8%)		Ι	O Level	
m Volume for 95th perce	ntilo auquo	is motoro	d by unct	room ciar	al	

m Volume for 95th percentile queue is metered by upstream signal.

Lanes, Volumes, Timings 107:



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	<u>م</u> لا	LDI				
Traffic Volume (vph)	4 70	115	1 40	↑↑ 1170	TT 922	r 85
Future Volume (vph)	470 620	265	40	1170	922	85
Ideal Flow (vphpl)	1900	1900	1900	1900	922 1900	1900
Lane Util. Factor	0.97	0.95	1.00	0.95	0.95	1,00
Frt	0.97	0.90	1.00	0.90	0.90	0.850
Fit Protected	0.955		0.950			0.000
	3334	0	0.950	3539	3539	1583
Satd. Flow (prot)	3334 0.966	U		3039	3039	1083
Flt Permitted		0	0.200	2520	2520	1500
Satd. Flow (perm)	3334	0	373	3539	3539	1583 Voc
Right Turn on Red	1 / /	Yes				Yes
Satd. Flow (RTOR)	144			4	45	92
Link Speed (mph)	25			45	45	
Link Distance (ft)	738			727	965	
Travel Time (s)	20.1			11.0	14.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	674	288	43	1272	1002	92
Shared Lane Traffic (%)						
Lane Group Flow (vph)	962	0	43	1272	1002	92
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	36			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15			9
Turn Type	Prot		pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	
Permitted Phases	1		2	2	0	6
Minimum Split (s)	22.5		9.5	22.5	22.5	22.5
Total Split (s)	22.5		10.0	30.0	22.5	22.5
	40.0%		20.0%	60.0%	40.0%	40.0%
Total Split (%)						
Maximum Green (s)	15.5		5.5	25.5	15.5	15.5
Yellow Time (s)	3.5		3.5	3.5	3.5	3.5
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Lead/Lag			Lead		Lag	Lag
Lead-Lag Optimize?			Yes		Yes	Yes
Walk Time (s)	7.0			7.0	7.0	7.0
Flash Dont Walk (s)	11.0			11.0	11.0	11.0
Pedestrian Calls (#/hr)	0			0	0	0
Act Effct Green (s)	15.5		25.5	25.5	15.5	15.5
Actuated g/C Ratio	0.31		0.51	0.51	0.31	0.31
v/c Ratio	0.85		0.13	0.71	0.91	0.17
Control Delay	23.4		7.0	12.1	32.0	4.6
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	23.4		7.0	12.1	32.0	4.6
i ulai Delay	Z3.4		7.0	12.1	32.0	4.0

Synchro 11 Report Page 16

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
LOS	С		А	В	С	А	
Approach Delay	23.4			11.9	29.7		
Approach LOS	С			В	С		
Queue Length 50th (ft)	122		6	134	145	0	
Queue Length 95th (ft)	#201		17	195	#253	24	
Internal Link Dist (ft)	658			647	885		
Turn Bay Length (ft)							
Base Capacity (vph)	1132		343	1804	1097	554	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.85		0.13	0.71	0.91	0.17	
Intersection Summary							
JI	Other						
Cycle Length: 50							
Actuated Cycle Length: 50							
Offset: 0 (0%), Referenced	to phase 2:I	NBTL and	d 6:SBT, S	Start of G	reen, Mas	ster Inters	section
Natural Cycle: 60							
Control Type: Pretimed							
Maximum v/c Ratio: 0.91							
Intersection Signal Delay: 2					tersection		
Intersection Capacity Utiliza	ition 57.8%			IC	U Level c	of Service	В
Analysis Period (min) 15							
# 95th percentile volume e			eue may	be longer			
Queue shown is maximu	im after two	cycles.					

Splits and Phases:	108:
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≪¶ø2 (R) 🖡	
30 s	20 s
▲ Ø5 🕴 Ø6 (R)	
10 s 20 s	

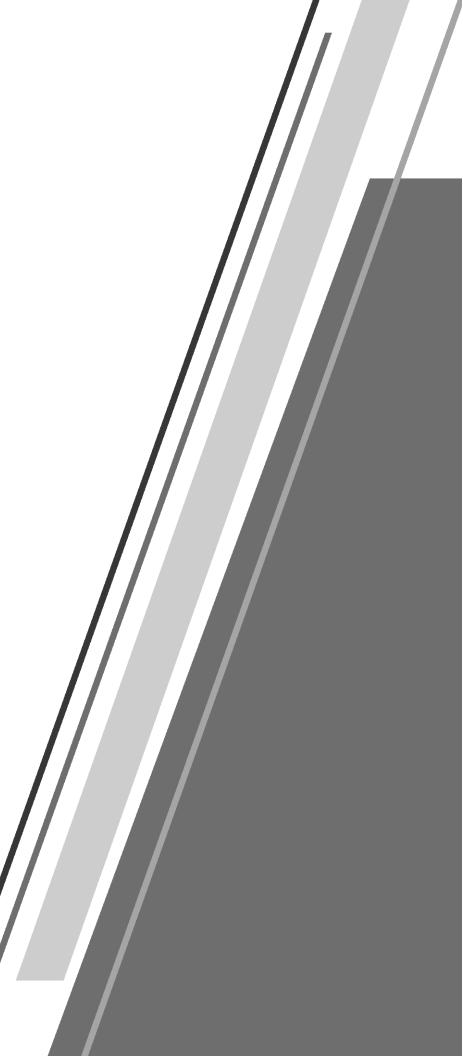
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Lane Group	NBT	NBR	SBL	SBT	NWL	NWR	
Lane Configurations	4			†			
Traffic Volume (vph)	19	0	361	0	0	0	
Future Volume (vph)	119	200	461	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	0.915						
Flt Protected				0.950			
Satd. Flow (prot)	1704	0	0	1770	0	0	
Flt Permitted				0.950			
Satd. Flow (perm)	1704	0	0	1770	0	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	1082			1015	590		
Travel Time (s)	24.6			23.1	13.4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	129	217	501	0	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	346	0	0	501	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	0		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
51	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	tion 30.0%			IC	U Level o	of Service	e A
Analysis Poriod (min) 15							

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	†			†	Y	
Traffic Volume (vph)	0	0	0	0	0	19
Future Volume (vph)	300	0	0	0	0	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.865	
Flt Protected						
Satd. Flow (prot)	1863	0	0	1863	1611	0
Flt Permitted						
Satd. Flow (perm)	1863	0	0	1863	1611	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	839			634	538	
Travel Time (s)	19.1			14.4	12.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	326	0	0	0	0	21
Shared Lane Traffic (%)						
Lane Group Flow (vph)	326	0	0	0	21	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 13.3%			IC	CU Level o	of Service
Analysis Poriod (min) 15						

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ę	et		ľ	*
Traffic Volume (vph)	0	0	0	0	0	0
Future Volume (vph)	0	0	0	0	300	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt						
Flt Protected					0.950	
Satd. Flow (prot)	0	1863	1863	0	1770	1863
Flt Permitted					0.950	
Satd. Flow (perm)	0	1863	1863	0	1770	1863
Link Speed (mph)		30	30		30	
Link Distance (ft)		98	839		286	
Travel Time (s)		2.2	19.1		6.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	0	326	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	326	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		36	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Stop		Stop	
Intersection Summary						
	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 13.3%			IC	CU Level o	of Service

APPENDIX D

Gate SMART Analysis



Existing Gate Needs (Adjusted Volume) – Alternative 1

Barta Road Gate - AM

	Quick Calculation									
	Manual P	rocessing	Handheld	Processing	AIE Pro	cessing				
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms				
Required Lanes	1	1	1	1	1	1				
Traffic Queue (Total # of Vehicles) ?	1	1	1	1	1	1				
Expected Delay per Vehicle (seconds)	19	15	22	17	17	20				
Total Manpower Needed	1	2	1	2	1	1				
Design	Note: 7 Demand Volume: 148		at or below the maxim	um delay per vehicle va	alue set in the "Default	s" tab.				

GEOint Road Gate - AM

Quick Calculation AIE Processing Arms Traffic Arms Handheld Processing gle Tandem Manual Processing Single No Traffic Arms Single Tande **Required Lanes** 1 1 1 1 1 1 Traffic Queue (Total # of Vehicles) ? 1 1 1 1 1 1 Expected Delay per Vehicle (seconds) 19 15 22 17 17 20 1 Total Manpower Needed 2 1 2 1 1 Note: All conditions operate at or below the maxi um delay per vehicle value set in the "Defaults" tab. Design Demand Volume: 150 vph

Heller Road Gate - AM

Quick Calculation

	Manual P	rocessing	Handheld	Processing	AIE Pro	cessing
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms
Required Lanes	1	1	1	1	1	1
Traffic Queue (Total # of Vehicles) ?	0	0	0	0	0	0
Expected Delay per Vehicle (seconds)	16	13	17	15	15	17
Total Manpower Needed	1	2	1	2	1	1
			at or below the maxim	um delay per vehicle va	alue set in the "Default	s" tab.
Desig	n Demand Volume: 19	vph				

Existing Gate Needs (Adjusted Volume) – Alternative 2

Meade Road Gate - AM

	Quick Calculation									
	Manual F	rocessing	Handheld	Processing	AIE Pro					
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms				
Required Lanes	1	1	1	1	1	1				
Traffic Queue (Total # of Vehicles) ?	0	0	0	0	0	0				
Expected Delay per Vehicle (seconds)	16	13	17	15	15	16				
Total Manpower Needed	1	2	1	2	1	1				
	Note:	All conditions operate	at or below the maximu	um delay per vehicle va	alue set in the "Default	s" tab.				
Desi	gn Demand Volume: 1	/ph								

Belvoir Road Gate - AM

	Quick Calculation										
	Manual F	Processing	Handheld	Processing	AIE Processing						
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms					
Required Lanes	2	1	2	1	1	2					
Traffic Queue (Total # of Vehicles) ?	3	5	4	9	9	4					
Expected Delay per Vehicle (seconds)	18	27	21	59	59	20					
Total Manpower Needed	2	2	2	2	1	2					
	Note:	All conditions operate	at or below the maxim	um delay per vehicle v	alue set in the "Default	s" tab.					
Desigr	Demand Volume: 398	8 vph									

Pohick Road Gate - AM

	Quick Calculation										
	Manual Processing Single Tandem		Handheld Processing Single Tandem		AIE Processing No Traffic Arms Traffic Arms						
Required Lanes	1	1	1	1	1	1					
Traffic Queue (Total # of Vehicles) ?	4	2	7	3	3	5					
Expected Delay per Vehicle (seconds)	31	18	54	23	23	39					
Total Manpower Needed	1	2	1	2	1	1					
		All conditions operate	at or below the maximu	um delay per vehicle va	alue set in the "Default	s" tab.					
Design	n Demand Volume: 288	l vph									

Existing Gate Needs (Adjusted Volumes w/ 650 Additional Personnel) - Alternative 1

Barta Road Gate - AM

	Quick Calculation										
	Manual Processing		Handheld	Processing	AIE Processing						
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms					
Required Lanes	1	1	1	1	1	1					
Traffic Queue (Total # of Vehicles) ?	1	1	1	1	1	1					
Expected Delay per Vehicle (seconds)	19	15	22	17	17	20					
Total Manpower Needed	1	2	1	2	1	1					
	Note: All c	onditions operate at	or below the maxim	um delay per vehicle	value set in the "Def	aults" tab.					
Design	Demand Volume: 14	8 vph									

GEOint Road Gate - AM

	Quick Calculation										
	Manual Processing		Handheld Processing		AIE Processing						
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms					
Required Lanes	1	1	1	1	1	1					
Traffic Queue (Total # of Vehicles) ?	3	2	5	2	2	4					
Expected Delay per Vehicle (seconds)	27	17	40	21	21	32					
Total Manpower Needed	1	2	1	2	1	1					
	Note: All o	onditions operate at	or below the maxim	um delay per vehicle	value set in the "Def	aults" tab.					
Design	Demand Volume: 26	4 vph									

Heller Road Gate - AM

Quick Calculation										
	Manual Processing		Handheld	Processing	AIE Processing					
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms				
Required Lanes	1	1	1	1	1	1				
Traffic Queue (Total # of Vehicles) ?	0	0	0	0	0	0				
Expected Delay per Vehicle (seconds)	16	14	18	15	15	17				
Total Manpower Needed	1	2	1	2	1	1				
	Note: All o	onditions operate at	or below the maxim	um delay per vehicle	value set in the "Defa	aults" tab.				
Design	Demand Volume: 51	l vph								

Existing Gate Needs (Adjusted Volumes w/ 650 Additional Personnel) – Alternative 2

Meade Road Gate - AM

	Quick Calculation										
	Manual P	rocessing	Handheld	Processing	AIE Processing						
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms					
Required Lanes	1	1	1	1	1	1					
Traffic Queue (Total # of Vehicles) ?	0	0	0	0	0	0					
Expected Delay per Vehicle (seconds)	16	13	17	15	15	17					
Total Manpower Needed	1	2	1	2	1	1					
	Note: /	All conditions operate	at or below the maxim	um delay per vehicle v	alue set in the "Default	s" tab.					
Desig	n Demand Volume: 13	vph									

Belvoir Road Gate – AM

Quick Calculation										
	Manual F	rocessing	Handheld	Processing	AIE Processing					
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms				
Required Lanes	2	1	2	2	2	2				
Traffic Queue (Total # of Vehicles) ?	5	11	7	4	4	6				
Expected Delay per Vehicle (seconds)	20	66	25	17	17	22				
Total Manpower Needed	2	2	2	4	2	2				
	Note:	All conditions operate	at or below the maxim	um delay per vehicle va	alue set in the "Default	s" tab.				
Design	n Demand Volume: 482	! vph								

Pohick Road Gate - AM

	Quick Calculation										
	Manual P	rocessing	Handheld	Processing	AIE Processing						
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms					
Required Lanes	1	1	2	1	1	1					
Traffic Queue (Total # of Vehicles) ?	9	3	3	5	5	11					
Expected Delay per Vehicle (seconds)	63	21	20	33	33	110					
Total Manpower Needed	1	2	2	2	1	1					
	Note:	All conditions operate	at or below the maxim	um delay per vehicle v	alue set in the "Default	s" tab.					
Design	n Demand Volume: 349	vph									

Existing Gate Needs (Adjusted Volumes w/ 1000 Additional Personnel) – Alternative 1

Barta Road Gate - AM

	Quick Calculation									
	Manual Processing		Handheld Processing		AIE Processing					
Required Lanes	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms				
· · · · · · · · · · · · · · · · · · ·	1	1	1	1	1	1				
Traffic Queue (Total # of Vehicles) ?	1	1	1	1	1	1				
Expected Delay per Vehicle (seconds)	19	15	22	17	17	20				
Total Manpower Needed	1	2	1	2	1	1				
			or below the maxim	um delay per vehicle	value set in the "Def	aults" tab.				
Design	Demand Volume: 14	8 vph								

GEOint Road Gate - AM

Quick Calculation										
	Manual Processing		Handheld Processing		AIE Processing					
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms				
Required Lanes	1	1	2	1	1	1				
Traffic Queue (Total # of Vehicles) ?	9	3	3	5	5	11				
Expected Delay per Vehicle (seconds)	61	21	20	33	33	107				
Total Manpower Needed	1	2	2	2	1	1				
Note: All conditions operate at or below the maximum delay per vehicle value set in the "Defaults" tab. Design Demand Volume: 348 vph										

Heller Road Gate - AM

	Quick Calculation										
	Manual Processing		Handheld Processing		AIE Processing						
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms					
Required Lanes	1	1	1	1	1	1					
Traffic Queue (Total # of Vehicles) ?	0	0	0	0	0	0					
Expected Delay per Vehicle (seconds)	17	14	19	15	15	18					
Total Manpower Needed	1	2	1	2	1	1					
	Note: All	conditions operate at	or below the maxim	um delay per vehicle	value set in the "Defa	aults" tab.					
Design	Demand Volume: 7	8 vph									

Existing Gate Needs (Adjusted Volumes w/ 1000 Additional Personnel) – Alternative 2

Meade Road Gate - AM

	Quick Calculation										
	Manual P	rocessing	Handheld	Processing	AIE Pro	AIE Processing					
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms					
Required Lanes	1	1	1	1	1	1					
Traffic Queue (Total # of Vehicles) ?	0	0	0	0	0	0					
Expected Delay per Vehicle (seconds)	16	13	17	15	15	17					
Total Manpower Needed	1	2	1	2	1	1					
	Note:	All conditions operate	at or below the maxim	um delay per vehicle v	alue set in the "Default	s" tab.					
Desig	n Demand Volume: 13	vph									

Belvoir Road Gate - AM

Quick Calculation										
	Manual Processing		Handheld Processing		AIE Processing					
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms				
Required Lanes	2	2	2	2	2	2				
Traffic Queue (Total # of Vehicles) ?	6	3	10	5	5	8				
Expected Delay per Vehicle (seconds)	21	15	29	18	18	24				
Total Manpower Needed	2	4	2	4	2	2				
Note: All conditions operate at or below the maximum delay per vehicle value set in the "Defaults" tab.										
Design Demand Volume: 528 vph										

Pohick Road Gate - AM

Quick Calculation										
	Manual Processing		Handheld Processing		AIE Processing					
	Single	Tandem	Single	Tandem	No Traffic Arms	Traffic Arms				
Required Lanes	2	1	2	1	1	2				
Traffic Queue (Total # of Vehicles) ?	3	4	4	8	8	4				
Expected Delay per Vehicle (seconds)	18	25	21	51	51	20				
Total Manpower Needed	2	2	2	2	1	2				
Note: All conditions operate at or below the maximum delay per vehicle value set in the "Defaults" tab.										
Design Demand Volume: 388 vph										



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